BRIZON INVESTMENTS SCALE FUTURE LAND USE AMENDMENT FROM ROI TO CG

Project № 22242.3, v1.2 June 2023

TRANSPORTATION IMPACT ANALYSIS PORT ST LUCIE FLORIDA





Traffic & Mobility Consultants

3101 Maguire Boulevard, Suite 265 Orlando, Florida 32803 www.trafficmobility.com (407) 531-5332

Prepared for:

Estacado Interest LLC 2800 Guilder Drive Plano, Texas 75074

EXECUTIVE SUMMARY

Project Information

Name: Brizon Investments Scale Future Land Use Amendment

Location: Northeast corner of SW Port St. Lucie Boulevard and SW Aviation

Avenue

Jurisdiction: Port St Lucie, Florida

Description: Comprehensive Plan Amendment:

Current Future Land Use (FLU) – Residential Office Institutional (ROI)

Proposed FLU – General Commercial (CG)

Propose Development:

Convenience Store/Gas Station – 8 vehicle fueling positions

<u>Findings</u>

Trip Generation: 891 Daily Trips/ 51 AM Peak Hour Trips/ 65 PM Peak Hour Trips

Proposed FLU Amendment will result in the following additional traffic:

1,861 daily trips

107 PM peak hour trips

Roadway Existing – Savona Boulevard from Paar Drive to Gaitlin Boulevard

Capacity: operates below adopted level of service (LOS).

Horizon Year: <u>2030 Base Condition Deficiencies:</u>

Gaitlin Boulevard – Rosser Boulevard to Savona Boulevard Gaitlin Boulevard – Savona Boulevard to Port St Lucie Boulevard Port St Lucie Boulevard – Del Rio Boulevard to Cameo Boulevard Port St Lucie Boulevard – Cameo Boulevard to Florida's Turnpike

Savona Boulevard - Paar Drive to Gaitlin Boulevard

Savona Boulevard - Gaitlin Boulevard to California Boulevard

Tulip Boulevard - Port St Lucie Boulevard to Paar Drive

2030 Projected Condition Deficiencies:

Amendment does not cause any further deficiencies.

Intersection Capacity: SW Port St Lucie Boulevard and SW Aviation Avenue currently

experiences delay and is projected to continue at buildout. Project access driveways are projected to operate at an adequate level of

service (LOS) at project buildout.

Turn Lane Existing turn lanes are sufficient to accommodate projected traffic

Improvements: conditions.

Turn Lanes are not warranted at the project access driveways on SW

Port St. Lucie Boulevard nor SW Aviation Avenue.



Brizon Investments
Transportation Impact Analysis
Project № 22242.3, v1.2
Executive Summary

PROFESSIONAL ENGINEERING CERTIFICATION

I hereby certify that I am a Professional Engineer properly registered in the State of Florida practicing with Traffic & Mobility Consultants LLC, a corporation authorized to operate as an engineering business, CA-30024, by the State of Florida Department of Professional Regulation, Board of Professional Engineers, and that I have prepared or approved the evaluations, findings, opinions, conclusions, or technical advice attached hereto for:

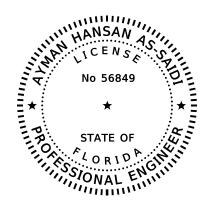
PROJECT: Brizon Investments Scale Future Land Use Amendment

LOCATION: Port St Lucie

CLIENT: Estacado Interest LLC

I hereby acknowledge that the procedures and references used to develop the results contained in these computations are standard to the professional practice of Transportation Engineering as applied through professional judgment and experience.

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY



ON THE DATE ADJACENT TO THE SEAL

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TRAFFIC & MOBILITY CONSULTANTS LLC 3101 MAGUIRE BOULEVARD, SUITE 265 ORLANDO, FLORIDA 32803 CERTIFICATE OF AUTHORIZATION CA-30024 AYMAN H. AS-SAIDI. P.E. NO. 56849

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1.0 INTRODUCTION

This Transportation Impact Analysis (TIA) was conducted to assess the impact of the proposed

convenience store/gas station with 8 vehicle fueling pumps and in support of a Comprehensive

Plan Amendment application to change the Future Land Use (FLU) for the proposed development

of Brizon Investments. The ±1.03-acre site is located on the northeast corner of SW Port St. Lucie

Boulevard and SW Aviation Avenue in Port St. Lucie, Florida. **Figure 1** illustrates the site location

and transportation area network. This TIA was revised in consideration of the comments received

from the City of Port St. Lucie. The property information and City correspondence are provided in

Appendix A.

Access to the proposed site will be provided via two (2) access driveways; one (1) existing right-

in/right-out (RI/RO) access driveway on SW Port St. Lucie Boulevard, which is shared with an

adjacent existing business, and one (1) proposed full access driveway on SW Aviation Avenue.

The analysis evaluates the capacity of the access driveways on SW Port St. Lucie Boulevard and

SW Aviation Avenue to accommodate the projected traffic for buildout traffic conditions.

Additionally, the analysis evaluates the need for ingress turn lanes at the project driveways,

including left and right turn lane warrants.

1.1 Future Land Use Designation

The current FLU designation for the property is Residential Office Institutional (ROI) with a

maximum building coverage of 40%. The proposed FLU designation for the property is General

Commercial (GC) with a maximum building coverage of 40%. Excerpts from the Port St. Lucie

Code of Ordinances are included in **Appendix B**.

Table 1 summarizes the land uses and the maximum building coverage for the site as currently

adopted and with the requested CP Amendment.

TAC

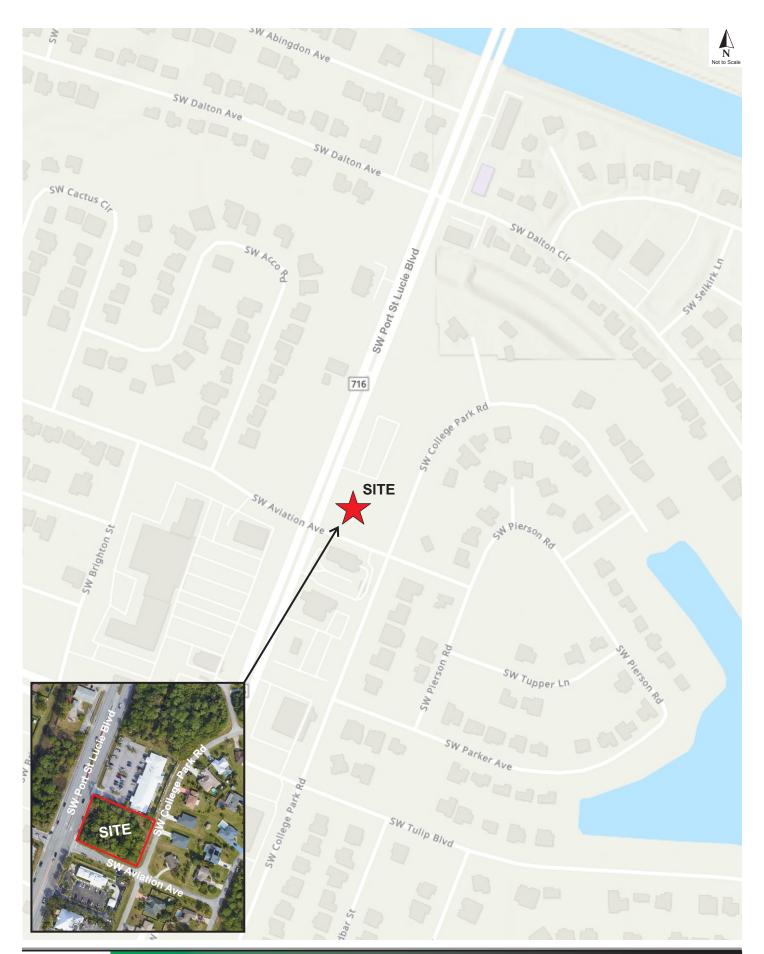


Table 1
Future Land Use Designations

Land I Hara Bardina di San	harda Padian	A	Maximum	D
Land Use Designation	Jurisdiction	Area	Building Coverage	Development
Existing FLU - Professional Zoning	District (P)			
Residential Office Institutional (ROI)	Port St Lucie	1.03 Ac	40%	17.95 KSF
Proposed FLU - General Commercia	al Zoning Dist	rict (CG)		
General Commercial	Port St Lucie	1.03 Ac	40%	17.95 KSF

The analysis was performed using traffic data obtained in the field by Traffic & Mobility Consultants LLC (TMC), information obtained from the project team, data published by the Florida Department of Transportation (FDOT), the Institute of Transportation Engineers, and public data obtained from *St. Lucie Transportation Planning Organization (TPO)*, included in Appendix C.



2.0 EXISTING TRAFFIC CONDITIONS

Existing conditions were analyzed to establish a baseline for the traffic conditions prevailing in the vicinity of the proposed development. This included the area's major roadways, which were analyzed for PM peak hour conditions and a review of intersection capacities, which were analyzed for both AM peak hour and PM peak hour conditions.

2.1 Roadway Segment Capacity

The existing conditions on the roadway network were analyzed by comparing the latest available traffic volumes on each of the roadway segments to the adopted capacity thresholds. The existing conditions analysis was based on information obtained from the *St. Lucie County TPO 2022 Traffic Counts and Level of Service Report* and the City of Port St. Lucie 2020 – 2040 Comprehensive Plan (CP) Table 2-1 Roadway System Classifications. Excerpts from the TPO and CP are provided in **Appendix C**.

Table 2 summarizes the existing conditions capacity analysis in the study area. The analysis reveals that all study roadway segments currently operate at adequate Level of Service (LOS) except Savona Boulevard from Paar Drive to Gaitlin Boulevard, which is currently operating below the adopted level of service (LOS).

2.2 Intersection Capacity Analysis

Existing traffic volumes were obtained at the intersections of SW Port St. Lucie Boulevard and SW Aviation Avenue and SW Port St. Lucie Boulevard and the RI/RO access driveway to establish the prevailing traffic profiles during the peak hours. Traffic counts were obtained between the hours of 7:00am to 9:00am and 4:00pm to 6:00pm. The intersection capacity analysis was performed for AM and PM peak hour periods. The capacity analysis was performed using Synchro software and the methods of the *Highway Capacity Manual (HCM)*. A peak seasonal adjustment factor was not applied to the volumes as they were collected during the peak season. The raw traffic volume data and the *2021 Peak Seasonal Factor Category Report* are included in **Appendix D**. The results of the intersection capacity analysis are summarized in **Table 3** and included in **Appendix E**.



Table 2
Existing Roadway Segment Capacity Analysis

Station			#	Road	Min		Pl	M Peak Ho	our		Meets
ID	Roadway	Segment Limits	Lns	Classification	LOS	AADT	Сар	Volume	V/C	LOS	Std?
638	Cameo Blvd	Port St Lucie Blvd to California Blvd	2	Collector	D	4,900	750	299	0.399	С	Y
659	Darwin Blvd	Tulip Blvd to Port St Lucie Blvd	2	Collector	D	12,900	920	603	0.655	С	Υ
311	Del Rio Blvd	Port St Lucie Blvd to California Blvd	2	Collector	D	10,000	920	704	0.765	С	Υ
945075	Gaitlin Blvd	Rosser Blvd to Savona Blvd	6	Pincipal Arterial	Е	47,867	3,170	2,846	0.898	С	Y
945075	Gaitlin Blvd	Savona Blvd to Port St Lucie Blvd	6	Pincipal Arterial	Е	47,867	3,170	2,846	0.898	С	Y
948519	Port St Lucie Blvd	Tulip Blvd to Darwin Blvd	2	Pincipal Arterial	Е	15,257	920	704	0.765	С	Υ
697	Port St Lucie Blvd	Darwin Blvd to Gaitlin Blvd	4	Pincipal Arterial	Е	33,879	3,020	1,712	0.567	С	Υ
698	Port St Lucie Blvd	Gaitlin Blvd to Del Rio Blvd	6	Pincipal Arterial	Е	40,895	3,170	2,108	0.665	С	Υ
945074	Port St Lucie Blvd	Del Rio Blvd to Cameo Blvd	6	Pincipal Arterial	Е	49,241	3,170	2,989	0.943	С	Υ
945074	Port St Lucie Blvd	Cameo Blvd to Florida's Turnpike	6	Pincipal Arterial	Е	49,241	3,020	2,989	0.990	D	Υ
236	Savona Blvd	Paar Dr to Gaitlin Blvd	2	Minor Arterial	Е	10,556	750	844	1.125	F	N
702	Savona Blvd	Gaitlin Blvd to California Blvd	2	Minor Arterial	Е	14,200	790	695	0.880	D	Υ
714	Tulip Blvd	Port St Lucie Blvd to Paar Dr	2	Collector	D	10,127	790	606	0.767	D	Υ

Source: St Lucie County TPO 2022 Traffic Counts and Level of Service Report



Table 3
Existing Roadway Segment Capacity Analysis

		Time	Е	В	W	В	N	В	SI	В
Intersection	Control	Period	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
SW Port St Lucie Boulevard &	TWSC	AM	73.2	F	20.7	С	18.4	С	18.9	С
SW Aviation Ave	10030	PM	>300	F	26.6	D	32.6	D	35.5	Е
SW Port St Lucie Boulevard &	TWSC	AM			16.8	С	0.0	Α	0.0	Α
RI/RO Access Driveway	10050	PM			17.4	С	0.0	Α	0.0	Α

Average delay expressed in seconds/vehicle

The unsignalized SW Port St Lucie Boulevard and SW Aviation Avenue intersection currently operates with delays on the eastbound approach during both AM and PM peak hour. The access driveway currently operates with an acceptable LOS.



3.0 PLANNED AND PROGRAMMED IMPROVEMENTS

The St Lucie Transportation Improvement Program (TIP) was checked to identify any planned or programmed improvements to the transportation facilities in this area. Pertinent information is included in **Appendix F**.

The *TIP* includes the planned widening Port St. Lucie Boulevard from two (2) to four (4) lanes, from Paar Drive to Darwin Boulevard. The *TIP* also includes traffic signal improvements on Gatlin Boulevard from west of I-95 to Port St. Lucie Boulevard. Both roadway segments are currently being funded for construction.



4.0 PROJECT TRAFFIC

4.1 Trip Generation

A trip generation analysis was conducted for the convenience store/gas station using the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11th Edition.* Pass-by trips were calculated based on the trip percentages from land use code (LUC) 945. The traffic generation of the existing and proposed maximum development scenarios were calculated using the data published in the ITE *Trip Generation Manual, 11th Edition.* The trip generation analysis and trip generation comparative analysis are summarized in **Table 4** and **Table 5**, respectively. Detailed trip generation sheets are provided in **Appendix G**.

Table 4
Trip Generation Analysis

ITE	ITE Da					AM Pea	ak Hour	PM Peak Hour				
Code	Land Use	Size	Rate	Trips	Rate	Total	Enter	Exit	Rate	Total	Enter	Exit
945	Convenience Store/Gas Station	8 VFP	265.12	2,121	16.06	128	64	64	18.42	147	74	73
Pass By Trips (60% AM, 56% PM)						77	38	39		82	41	41
	Net New Trips					51	26	25		65	33	32

Trip generation analysis and pass-by trip percentages were based on ITE Trip Generation Manual, 11th Edition VFP - Vehicle Fueling Positions

The proposed development is projected to generate 891 new trips per day, of which 51 new trips occur during the AM peak hour and 65 new trips occur during the PM peak hour.



Table 5
Trip Generation Comparative Analysis

ITE			Da	aily	PM Peak Hour Trips				
Code	Land Use	Size	Rate Trips		Rate	Total	Enter	Exit	
Existi	ng FLU - Professional Zoning Distric	t (P)							
710	Residential Office Institutional (ROI)	17.95 KSF	14.51	260	2.22	40	7	33	
Propo	sed FLU - General Commercial Zoni	ng District (C	G)						
945	Convenience Store/Gas Station	8 VFP	265.12	2,121	18.42	147	74	73	
	Net Change in Trips w/ I	endment	1,861		107	67	40		

Trip calculations and pass-by trip percentages based on ITE Trip Generation, 11th Edition

Fitted Curve Equation used to calculate rates when $R^2 > 0.75$

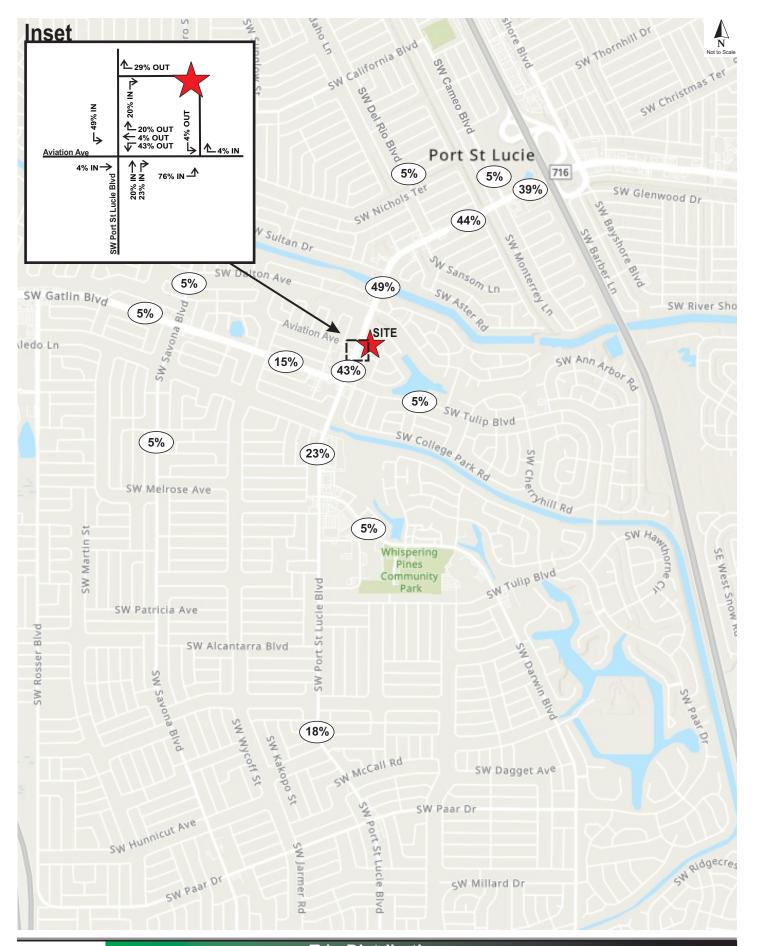
General Office Building ITE Trip Generation rates were used for ROI

The proposed FLU amendment is projected to generate a net increase of 1,861 daily trips and 107 PM peak hour trips.

4.2 Trip Distribution

The project trips were distributed and assigned to the development driveways to generate the projected traffic volumes and turning movements at both proposed driveways. The trip assignment was made in accordance with the existing peak hour traffic patterns of the transportation roadway network. The proposed trip distribution pattern is provided in **Figure 2**.





5.0 PROJECTED TRAFFIC CONDITIONS

An analysis of projected conditions was conducted to evaluate the impact of the proposed amendment on the roadway network, as well as the proposed access connections and intersection to the site. The projected conditions analysis was performed for the Horizon Year (2030). The analyses were conducted for the base condition (without the amendment) and for the proposed condition (with the amendment).

5.1 Background Traffic Volumes and Transportation Network

A review of the historical trends was conducted within the influence area. A minimum 2% annual growth rate was applied to roadway segments for which minimal or no growth was detected. The annual growth rate was used to compute the background traffic volumes for the Horizon Year, which reflects the continuing growth in the general area and resulting additional traffic in these segments. Historical counts and growth rate information are provided in **Appendix H.**

5.2 Horizon Year Conditions

Table 6 summarizes the base conditions analysis (without the amendment) for the 2030 Horizon Year. The analysis reveals that the following segments are projected to exceed the LOS standard in the Horizon Year:

- Gaitlin Boulevard Rosser Boulevard to Savona Boulevard
- Gaitlin Boulevard Savona Boulevard to Port St Lucie Boulevard
- Port St Lucie Boulevard Del Rio Boulevard to Cameo Boulevard
- Port St Lucie Boulevard Cameo Boulevard to Florida's Turnpike
- Savona Boulevard Paar Drive to Gaitlin Boulevard
- Savona Boulevard Gaitlin Boulevard to California Boulevard
- Tulip Boulevard Port St Lucie Boulevard to Paar Drive

Analysis of projected conditions with the proposed amendment, summarized in **Table 7**, reveals that the amendment will not cause any additional segments to become deficient in the Horizon Year.



Table 6
Base Conditions Analysis – Horizon Year (2030)

Station			#	Min		2022 PM Peak Hr	Back	ground	Meets
ID	Roadway	Segment Limits	Lns	LOS	Capacity	Volume	Growth	Volume	Std?
638	Cameo Blvd	Port St Lucie Blvd to California Blvd	2	D	750	299	2%	347	Υ
659	Darwin Blvd	Tulip Blvd to Port St Lucie Blvd	2	D	920	603	5.42%	864	Υ
311	Del Rio Blvd	Port St Lucie Blvd to California Blvd	2	D	920	704	2%	817	Υ
945075	Gaitlin Blvd	Rosser Blvd to Savona Blvd	6	Е	3,170	2,846	6.87%	4,411	N
945075	Gaitlin Blvd	Savona Blvd to Port St Lucie Blvd	6	Е	3,170	2,846	6.87%	4,411	N
948519	Port St Lucie Blvd	Tulip Blvd to Darwin Blvd	2	ш	920	704	2%	817	Υ
697	Port St Lucie Blvd	Darwin Blvd to Gaitlin Blvd	4	Ш	3,020	1,712	2%	1,986	Υ
698	Port St Lucie Blvd	Gaitlin Blvd to Del Rio Blvd	6	Е	3,170	2,108	2%	2,445	Υ
945074	Port St Lucie Blvd	Del Rio Blvd to Cameo Blvd	6	Е	3,170	2,989	4.26%	4,008	N
945074	Port St Lucie Blvd	Cameo Blvd to Florida's Turnpike	6	Е	3,020	2,989	2%	3,467	N
236	Savona Blvd	Paar Dr to Gaitlin Blvd	2	Е	750	844	5.36%	1,206	N
702	Savona Blvd	Gaitlin Blvd to California Blvd	2	Е	750	695	5.36%	993	N
714	Tulip Blvd	Port St Lucie Blvd to Paar Dr	2	D	790	606	6.67%	929	N



Table 7
Projected Conditions Analysis – Horizon Year (2030)

Station			#	Min		Backg'd		F	roject	Total	Meets
ID	Roadway	Segment Limits	Lns	LOS	Capacity	Volume	Dir	Dist	FLU Trips	Volume	Std?
638	Cameo Blvd	Port St Lucie Blvd to California Blvd	2	D	750	347	NB	5%	2	349	Υ
659	Darwin Blvd	Tulip Blvd to Port St Lucie Blvd	2	D	920	864	NB	5%	3	867	Υ
311	Del Rio Blvd	Port St Lucie Blvd to California Blvd	2	D	920	817	NB	5%	2	819	Υ
945075	Gaitlin Blvd	Rosser Blvd to Savona Blvd	6	Е	3,170	4,411	ЕВ	5%	3	4,414	N
945075	Gaitlin Blvd	Savona Blvd to Port St Lucie Blvd	6	Е	3,170	4,411	ЕВ	15%	10	4,421	N
948519	Port St Lucie Blvd	Tulip Blvd to Darwin Blvd	2	Е	920	817	SB	23%	9	826	Υ
697	Port St Lucie Blvd	Darwin Blvd to Gaitlin Blvd	4	Е	3,020	1,986	NB	23%	15	2,001	Υ
698	Port St Lucie Blvd	Gaitlin Blvd to Del Rio Blvd	6	Е	3,170	2,445	NB	49%	33	2,478	Υ
945074	Port St Lucie Blvd	Del Rio Blvd to Cameo Blvd	6	Е	3,170	4,008	NB	44%	18	4,026	N
945074	Port St Lucie Blvd	Cameo Blvd to Florida's Turnpike	6	Е	3,020	3,467	NB	39%	16	3,483	N
236	Savona Blvd	Paar Dr to Gaitlin Blvd	2	Е	750	1,206	NB	5%	3	1,209	N
702	Savona Blvd	Gaitlin Blvd to California Blvd	2	Е	750	993	NB	5%	2	995	N
714	Tulip Blvd	Port St Lucie Blvd to Paar Dr	2	D	790	929	ЕВ	5%	2	931	N



5.3 Intersection Capacity Analysis

The projected volumes for the intersection analysis were calculated based on the trip generation projections and the project's trip distribution pattern. The intersection operations were analyzed using the methods of the *Highway Capacity Manual (HCM)*, as applied in the *Synchro* analytical tool, and the projected traffic volumes at the study driveways. The projected AM and PM peak intersection volume calculations are included in **Appendix I** and illustrated in **Figure 3** and **Figure 4**, respectively. The results of the background and projected analysis of peak hour conditions are summarized in **Table 8** and **Table 9**, respectively. The *Synchro* analysis output sheets are provided as **Appendix J**.

Table 8
Background Intersection Capacity Analysis

			E	EB		WB		NB		SB	
Intersection	Control	Period	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	
SW Port St Lucie Boulevard &	TMSC	AM	69.0	F	20.7	С	18.1	С	18.9	С	
SW Aviation Ave	10030	PM	>300	F	26.6	D	31.8	D	35.5	Е	

Average delay expressed in seconds/vehicle

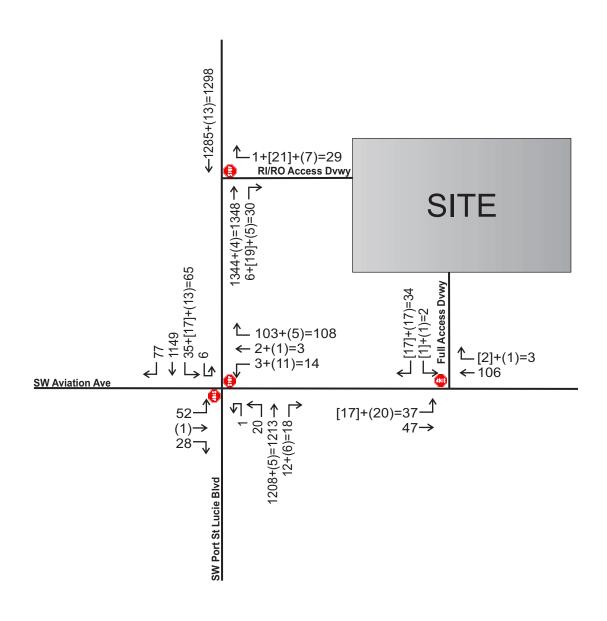
Table 9
Projected Intersection Capacity Analysis

		Time	Е	В	W	В	N	В	SI	В
Intersection	Control	Period	Delay	LOS	Delay	Los	Delay	LOS	Delay	LOS
SW Port St Lucie Boulevard &	TWSC	AM			18.2	С	0.0	Α	0.0	Α
RI/RO Access Driveway	IWSC	PM			18.8	С	0.0	Α	0.0	Α
SW Aviation Ave &	TWSC	AM	7.5	Α	0.0	Α			9.1	Α
Full Access Driveway	10030	PM	7.5	Α	0.0	Α			9.2	Α
SW Port St Lucie Boulevard &	TWOO	AM	89.3	F	25.0	D	18.1	С	21.4	С
SW Aviation Ave	TWSC	PM	>300	F	>300	F	31.8	D	48.7	Е

Average delay expressed in seconds/vehicle



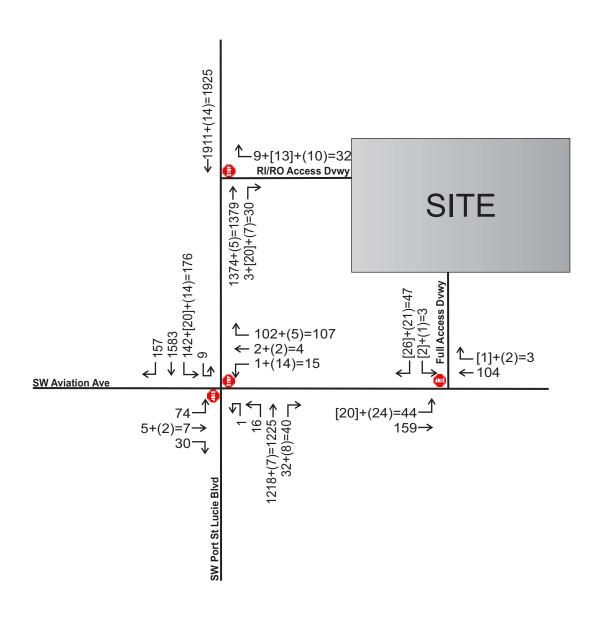




Legend: Existing + [Pass-by] + (Project) = Total







Legend: Existing + [Pass-by] + (Project) = Total



A background intersection capacity analysis was done for the intersection SW Port St Lucie Boulevard and SW Aviation Avenue to compare the maximum background 95th percentile vehicle queue to the buildout 95th percentile queue. The results of the background intersection analysis reveals that the intersection will continue to experience delays on the minor street due to the eastbound and westbound approaches. Project trips contribute less than one (1%) percent of the overall intersection volume. The analysis of driveway operations reveals that the access driveways are projected to operate with an acceptable level of service for buildout traffic conditions.

5.4 Intersection Queue Review

A review of projected queuing conditions at the intersection of SW Port St Lucie Boulevard and SW Aviation Avenue was conducted to determine if the existing turn lanes adequately accommodate the projected queues. The analysis was conducted for existing and buildout using the 95th percentile queue on the affected movements as obtained from the *HCM* worksheets included in **Appendix E** and **Appendix J**, respectively. Deceleration length was determined based on *FDOT Design Manual Exhibit 212-1*, included in **Appendix K**. **Table 10** shows the queue analysis summary, which shows that the existing left turn lane lengths for the southbound and westbound approaches are sufficient for the projected traffic conditions.

Table 10
Queue Analysis Summary

			95 ^t	h %ile Queue	(ft)	Turn Lane	Adequate	Adequate	Adequate
Intersection	Арр	Mvmt	Existing	Background	Buildout	Length (ft)	Queue? Existing	Queue? Background	Queue? Buildout
SW Port St Lucie Blvd	WB	Left	25'	25'	100'	260'	YES	YES	YES
& SW Aviation Ave	SB	Left	75'	75'	125'	250'	YES	YES	YES

Source: HCM Worksheets

Queue Length is the maximum value from AM & PM



6.0 ACCESS EVALUATION

SW Port St. Lucie Boulevard is a six (6) lane divided roadway with a posted speed limit of 45 miles per hour (mph). SW Aviation Avenue is a two (2) lane undivided roadway with a posted speed limit of 25 miles per hour (mph). The site will provide access via an existing right-in/right-out (RI/RO) access driveway on SW Port St. Lucie Boulevard, which is shared with an adjacent existing business. The site will also be provided with access via one (1) full driveway on SW Aviation Avenue.

A review of turn lane warrants on SW Port St. Lucie and SW Aviation Avenue at the proposed access driveways was conducted to determine if auxiliary turn lanes are necessary to maintain the integrity of traffic flow and capacity of the road during peak hours.

Right Turn Warrants

Based on the requirements that warrant a right turn lane from the *City of Port St. Lucie Engineering Standards for Land Development* report (Table 8-5), included in the **Appendix K**, exclusive right turn lanes for driveways are required when the operational aspects of the driveway meet the volume and speed criteria presented in Table 8-5, where a traffic study indicates that the LOS is degraded by the proposed development, or where required for safety reasons. The projected volumes and posted speed limits show that a right turn lane is not warranted. Also, the crash history was reviewed for the past 5 years, using the *Signal4 Analytics*, and it shows that no crashes occurred at the existing driveway on SW Port St. Lucie, and no crashes on the segment of St. Lucie and SW Aviation Avenue adjacent to the proposed driveway. Accordingly, exclusive right turn lanes are not warranted at either project access driveway on SW Port St. Lucie at SW Aviation Avenue.

Left Turn Warrants

Based on requirements that warrant a left turn lane in the *City of Port St. Lucie Engineering Standards for Land Development* report, a left turn lane warrant was analyzed based on the National Cooperative Highway Research Program (NCHRP) method (see **Appendix K**), which shows that a left turn lane is not warranted at the project full access driveway on SW Aviation Avenue.



7.0 STUDY CONCLUSIONS

This TIA was conducted to assess the impact of the proposed convenience store/gas station with

8 vehicle fueling pumps and in support of a request to amend the Comprehensive Plan for the

proposed development of Brizon Investments. The ±1.03-acre site is located on the northeast

corner of SW Port St. Lucie Boulevard and SW Aviation Avenue in Port St. Lucie, Florida. The

proposed amendment is to modify the FLU from Residential Office Institutional (ROI) to General

Commercial (GC) with a total development of 17,424 square feet.

The findings and results of the analysis are summarized as follows:

• The project is estimated to generate 891 new daily trips, of which 51 new trips occur during

the AM peak hour and 65 new trips occur during the PM peak hour.

The proposed FLU amendment will result in a net increase of 1,861 daily trips and 107 PM

peak hour trips.

The site will have two (2) access driveways; one (1) existing right-in/right-out (RI/RO) access

driveway on SW Port St. Lucie Boulevard (shared with adjacent development), and one (1)

proposed full access driveway on SW Aviation Avenue.

An analysis of existing conditions reveals that Savona Boulevard from Paar Drive to Gaitlin

Boulevard is currently operating below the adopted LOS.

Analysis of Horizon Year 2030 base conditions indicates that the segments of Gaitlin

Boulevard from Rosser Boulevard to Port St Lucie Boulevard, Port St Lucie Boulevard from

Del Rio Blvd to Florida's Turnpike, Savona Boulevard from Gaitlin Boulevard to California

Boulevard and Tulip Boulevard from Port St Lucie Boulevard to Paar Drive are projected to

operate above the adopted capacity due to background traffic growth. The proposed FLU

amendment will not cause any further deficiencies on the network.

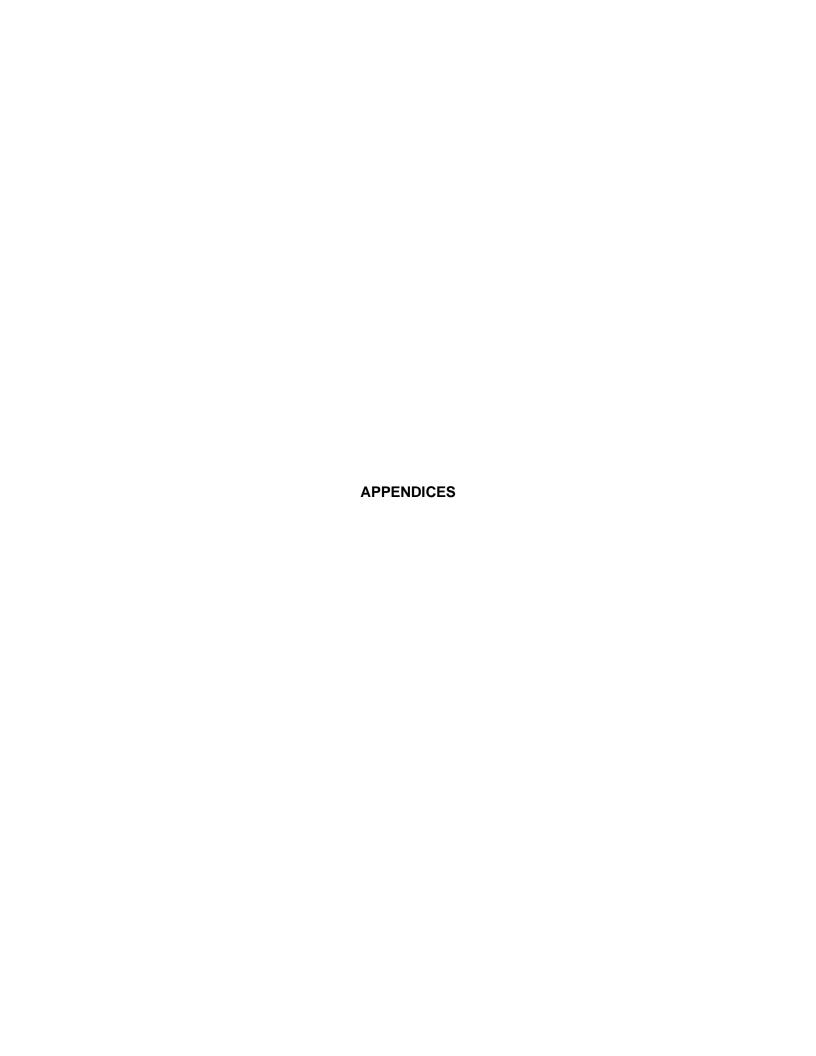
SW Port St Lucie Boulevard and SW Aviation Avenue operates with delays under existing

traffic conditions. Project trips contribute less than one (1%) percent of the overall intersection

volume.

- The project access driveways are projected to operate at an adequate level of service (LOS) at project buildout.
- Existing turn lanes are sufficient to accommodate projected traffic conditions.
- Left and right turn lanes are not warranted on SW Port St. Lucie Boulevard nor SW Aviation Avenue.





Appendix A
Response to Comments and Property Information



June 26, 2023

Ms. Diana Spriggs, PE
Regulatory Division Director
City of Port St. Lucie Planning and Zoning
121 SW Port St Lucie Blvd
Port St. Lucie, FL 34984
DSpriggs@cityofpsl.com

Re: Brizon Investments

Response to Traffic Impact Analysis Comments

TMC Project № 22242, v1.1 City of Port St. Lucie, Florida

Dear Ms. Spriggs,

Please find below our responses to the review comments prepared by the City of Port St. Lucie Planning and Zoning dated May 19, 2023, regarding the above referenced Traffic Impact Analysis version 1.1, dated May 2023. The comments are listed in **bold** typeface and the TMC responses follow in *italic* typeface. Additionally, a revised Traffic Impact Analysis version 1.2 is provided under cover reflecting the changes resulting from these comments.

Page 6, Table 3 – Existing Roadway Segment Capacity Analysis:

1. What causes existing values to change from the last submittal?

TMC Response: The existing delay values changed because there was a miscalculation of the U-turn and left turn volumes in the previous TIA.

Page 8, Table 4:

2. Pass by trips should be 60% AM and 56% PM for 8 VFP locations.

TMC Response: Acknowledged. The pass-by trips have been updated to 60% AM and 56% PM for 8 VFP. Please see attached revised TIA v1.2 report attached.

Page 14, Table 8:

3. Why did values go down?

TMC Response: Please see response to Comment # 1.

Ms. Diana Spriggs, PE
Brizon Investments
Response to Traffic Impact Analysis Comments
TMC Project № 22242, v1.1
June 26, 2023
Page 2 of 3

4. The westbound leg of the intersection of SW Port St Lucie Boulevard and SW Aviation Avenue has been reduced below the acceptable LOS with the impacts of this project.

TMC Response: This transportation analysis was performed as a high-level analysis for the purpose of supporting the Future Land Use amendment for the subject property. A more detailed Traffic Impact Analysis will be performed at a later date during the Site Plan process, in which a mitigation plan will be provided for intersections and/or roadways that are projected to operate below their adopted LOS as a result of the proposed development.

5. A reduction in the LOS of a leg of the intersection based on the impacts of this project must be mitigated.

TMC Response: Please see response to Comment # 4.

Figure 4:

6. Check the queue length of the southbound left turn lane at SW Port St Lucie Boulevard and SW Aviation Avenue.

TMC Response: A review of the projected queuing conditions at the intersection of SW Port St Lucie Boulevard and SW Aviation Avenue was conducted to determine if the existing turn lanes adequately accommodate projected queues (see Section 5.4 of the updated TIA v1.2 attached).

Page 18, Study Conclusions:

7. All reductions to LOS in the existing intersections caused by this development will be required to be mitigated. The reduction of the LOS of the Westbound traffic at PSL Boulevard and Aviation Avenue below the LOS Standards is not acceptable.

TMC Response: Please see response to Comment # 4.

Page 19:

8. Verify existing left turn lane on Port St. Lucie Boulevard westbound has an acceptable que length for the additional left turns that will occur.

TMC Response: A review of the projected queuing conditions at the intersection of SW Port St Lucie Boulevard and SW Aviation Avenue was conducted to determine if the existing turn lanes adequately accommodate projected queues (see Section 5.4 of the updated TIA v1.2 attached).

9. Per City Code Section 160.80, a Site Plan will be evaluated in regard to if the existing surrounding roadways have adequate service volume to support this development at or above the adopted level of service of these roadways.

TMC Response: Noted. Please see response to Comment # 4.

Ms. Diana Spriggs, PE
Brizon Investments
Response to Traffic Impact Analysis Comments
TMC Project № 22242, v1.1
June 26, 2023
Page 3 of 3

END OF COMMENTS

We trust these responses and the revised Traffic Impact Analysis adequately address the review comments. We remain available to discuss this matter further or to answer any questions you may have.

Kind regards,

TRAFFIC & MOBILITY CONSULTANTS LLC

Ayman H. As-Saidi, PE Director of Engineering



June 21, 2023

Ms. Diana Spriggs, PE
Regulatory Division Director
City of Port St. Lucie Planning and Zoning
121 SW Port St Lucie Blvd
Port St. Lucie, FL 34984
DSpriggs@cityofpsl.com

Re: Brizon Investments

Response to Traffic Impact Analysis Comments

TMC Project № 22242, v1.1 City of Port St. Lucie, Florida

Dear Ms. Spriggs,

Please find below our responses to the review comments prepared by the City of Port St. Lucie Planning and Zoning dated May 19, 2023, regarding the above referenced Traffic Impact Analysis version 1.1, dated May 2023. The comments are listed in **bold** typeface and the TMC responses follow in *italic* typeface. Additionally, a revised Traffic Impact Analysis version 1.2 is provided under cover reflecting the changes resulting from these comments.

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Page 14, Table 8:

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TMC Response: Please see response to Comment # 1.

Ms. Diana Spriggs, PE
Brizon Investments
Response to Traffic Impact Analysis Comments
TMC Project № 22242, v1.1
June 21, 2023
Page 2 of 3

4. The westbound leg of the intersection of SW Port St Lucie Boulevard and SW Aviation Avenue has been reduced below the acceptable LOS with the impacts of this project.

TMC Response: This transportation analysis was performed as a high-level analysis for the purpose of supporting the Future Land Use amendment for the subject property. A more detailed Traffic Impact Analysis will be performed at a later date during the Site Plan process, in which a mitigation plan will be provided for intersections and/or roadways that are projected to operate below their adopted LOS as a result of the proposed development.

5. A reduction in the LOS of a leg of the intersection based on the impacts of this project must be mitigated.

TMC Response: Please see response to Comment # 4.

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6. Check the queue length of the southbound left turn lane t SW Port St Lucie Boulevard and SW Aviation Avenue.

TMC Response: A review of the projected queuing conditions at the intersection of SW Port St Lucie Boulevard and SW Aviation Avenue was conducted to determine if the existing turn lanes adequately accommodate projected queues (see Section 5.4 of the updated TIA v1.2 attached).

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TMC Response: A review of the projected queuing conditions at the intersection of SW Port St Lucie Boulevard and SW Aviation Avenue was conducted to determine if the existing turn lanes adequately accommodate projected queues (see Section 5.4 of the updated TIA v1.2 attached).

9. Per City Code Section 160.80, a Site Plan will be evaluated in regard to if the existing surrounding roadways have adequate service volume to support this development at or above the adopted level of service of these roadways.

TMC Response: Noted. Please see response to Comment # 4.

Ms. Diana Spriggs, PE
Brizon Investments
Response to Traffic Impact Analysis Comments
TMC Project № 22242, v1.1
June 21, 2023
Page 3 of 3

END OF COMMENTS

We trust these responses and the revised Traffic Impact Analysis adequately address the review comments. We remain available to discuss this matter further or to answer any questions you may have.

Kind regards,

TRAFFIC & MOBILITY CONSULTANTS LLC

Ayman H. As-Saidi, PE Director of Engineering



May 19, 2023

Ms. Diana Spriggs
City of Port St. Lucie Public Works
121 SW Port St Lucie Blvd
Port St. Lucie, FL 34984
dspriggs@cityofpsl.com

Re: Brizon Investments

Response to Transportation Impact Analysis Comments

TMC Project № 22242 City of Port St. Lucie, Florida

Dear Ms. Spriggs,

Please find below our responses to the review comments prepared by the City of Port St. Lucie Public Works department dated May 11, 2023, regarding the above referenced Transportation Impact Analysis dated May 2023. The comments are listed in **bold** typeface and the TMC responses follow in *italic* typeface. Additionally, a revised Transportation Impact Analysis is provided under cover reflecting the changes resulting from these comments.

1. Introduction 1.0 (page 1):

TMC Response: The site acreage has been updated to "1.03-acre site" in the revised (v1.1) report.

2. Future Land Use Designation 1.1 (page 3)

TMC Response: The area and development in Table 1 has been updated to reflect correct 1.03 acreage. Language in the text has also been revised to reflect St. Lucie TPO. Please see revised report v1.1.

3. Trip Generation 4.1 (page 8): Table 4, can you provide an example of a 12-fuel pump station site that fits on 1 acre of land? Please utilize a maximum of 8 VFP.

TMC Response: Noted. The TIA trip generation analysis table has been updated to 8 vehicle fueling pumps (VFP). Please see revised report v1.1.

4. Table 5: Revise to utilize worst case land use within General Commercial Zoning.

TMC Response: Acknowledged. The Convenience Store/ Gas Station (ITE code 945) land use code was utilized to reflect the worst-case scenario within General Commercial Zoning. Please see revised report v1.1.

5. Table 5: Update the development size from previous comment.

TMC Response: Acknowledged. The development size has been updated, in the revised report v1.1.

Ms. Diana Spriggs
Brizon Investments
Response to Transportation Impact Analysis Comments
TMC Project № 22242
May 19, 2023
Page 2 of 2

6. 7.0 Study Conclusions: Update to incorporate all comments marked on previous pages.

TMC Response: Acknowledged. The Study conclusions section of the report has been updated to reflect all changes that were made throughout the report. Please see revised report v1.1.

END OF COMMENTS

We trust these responses and the revised Transportation Impact Analysis adequately address the review comments. We remain available to discuss this matter further or to answer any questions you may have.

Kind regards,

TRAFFIC & MOBILITY CONSULTANTS LLC

Ayman H. As-Saidi, PE Director of Engineering



May 3, 2023

Ms. Bridget Kean, AICP Senior Planner City of Port St. Lucie Planning and Zoning 121 SW Port St Lucie Blvd Port St. Lucie, FL 34984 BKean@cityofpsl.com

Re: PSL-Starbucks

Response to Traffic Impact Analysis Comments

TMC Project № 22242 City of Port St. Lucie, Florida

Dear Ms. Bridget,

Please find below our responses to the review comments prepared by the City of Port St. Lucie Planning and Zoning dated April 13, 2023, regarding the above referenced Traffic Impact Analysis dated March 2023. The comments are listed in **bold** typeface and the TMC responses follow in *italic* typeface. Additionally, a revised Traffic Impact Analysis is provided undercover reflecting the changes resulting from these comments.

1. The report is labeled PSL-Starbucks. I would remove that and identify the project as Brizon Investments Scale Future Land Use Amendment from ROI to CG Traffic Impact Analysis.

TMC Response: Acknowledged. The report name has been changed to "Brizon Investments Scale Future Land Use Amendment from ROI to CG".

2. I would also include in the report an additional traffic analysis page(s) for a convenience store with gas pumps based on the maximum size that could fit there if you include fuel pumps with the retail.

TMC Response: The report has been updated to analyze the impact of a convenience store with gas pumps based on the maximum size.

3. I looked at the previous study you submitted (attached as traffic.pdf) and I noticed that some of the info. included in that study is not included in the revised study. For the revised study add the Intersection analysis for Aviation and PSL Blvd. and the access evaluation to determine if turn lanes are warranted.

TMC Response: Acknowledged. The intersection analysis for SW Aviation Avenue and SW Port St Lucie Boulevard have been included in the study. Additionally, an access evaluation section to determine if turn lanes are warranted has been included in the report.

Ms. Bridget Kean, ACP
PSL-Starbucks
Response to Traffic Impact Analysis Comments
TMC Project № 22242
May 3, 2023
Page 2 of 2

END OF COMMENTS

We trust these responses and the revised Traffic Impact Analysis adequately address the review comments. We remain available to discuss this matter further or to answer any questions you may have.

Kind regards,

TRAFFIC & MOBILITY CONSULTANTS LLC

Ayman H. As-Saidi, PE Director of Engineering

Property Identification

Site Address: 2773 SW PORT ST LUCIE BLVD

Sec/Town/Range: 18/37S/40E Parcel ID: 3420-705-0299-000-5 Jurisdiction: Port Saint Lucie

Use Type: 1000 Account #: 101259 Map ID: 44/18N Zoning:

Ownership

Brizon Investment LLC % Diaz 1891 Stratford Dr Westbury, NY 11590

Legal Description

PORT ST LUCIE-SECTION 41- BLK 2894 LOTS 10,11,12 AND 13 (MAP 44/18N) (OR 1759-71)

Current Values

Just/Market Value:	\$404,000
Assessed Value:	\$207,900
Exemptions:	\$0
Taxable Value:	\$207,900

Property taxes are subject to change upon change of ownership.

- Past taxes are not a reliable projection of future taxes. The sale of a property will prompt the removal of all exemptions, assessment caps, and special classifications.

Taxes for this parcel: SLC Tax Collector's Office Download TRIM for this parcel: Download PDF



Total Areas

Finished/Under Air (SF):	0
Gross Sketched Area (SF):	0
Land Size (acres):	1.03
Land Size (SF):	45,000

Building Design Wind Speed

Occupancy Category	I	II	Ш
Speed	140	150	160
Sources/links:			

Sale History

Date	Book/Page	Sale Code	Deed	Grantor	Price
Jul 17, 2003	1759 / 0071	XX02	WD	Tesoriero Susan	\$295,000
Apr 2, 2003	1688 / 0685	99	WD	Casai Joy H	\$225,000
Mar 22, 1996	1007 / 2637	XX01	PR	Andrew Casai	\$100
Sep 26, 1989	0656 / 2502	XX00	WD	Wai S Hom	\$75,000
Sep 1, 1983	0414 / 0517	XX01	CV		\$2,800

Building Information (1 of 1)

Finished Area: 0 SF

Gross Sketched Area: 0 SF Exterior Data

View: Roof Cover: Roof Structure: Building Type: Year Built: N/A Frame: Grade: Effective Year: N/A Primary Wall:

Story Height: No. Units: 0 Secondary Wall:

Interior Data

Bedrooms: 0Electric:Primary Int Wall:Full Baths: 0Heat Type:Avg Hgt/Floor: 0Half Baths: 0Heat Fuel:Primary Floors:A/C %: 0%Heated %: N/A%Sprinkled %: 0%



lmage or Sketch unavailable for display

Sketch Area Legend

Sub Area Description Area Fin. Area Perimeter

Special Features and Yard Items

Type Qty Units Year Blt

Current Year Values

Current Values Breakdown

Current Year Exemption Value Breakdown

 Building:
 \$0

 Land:
 \$404,000

 Just/Market:
 \$404,000

Ag Credit: \$0

Save Our Homes or

\$196,100

10% Cap:

Assessed: \$207,900 Exemption(s): \$0

Taxable: \$207,900

Current Year Special Assessment Breakdown

Start Year AssessCode Units Description Amount 1999 0061 3 Port St. Lucie Stormwater \$504.00

This does not necessarily represent the total Special Assessments that could be charged against this property. The total amount charged for special assessments is reflected on the most current tax statement and information is available with the SLC Tax Collector's Office 2.

Historical Values

Year	Just/Market	Assessed	Exemptions	Taxable
2022	\$404,000	\$207,900	\$0	\$207,900
2021	\$189,000	\$189,000	\$0	\$189,000
2020	\$189,000	\$189,000	\$0	\$189,000
2019	\$189,000	\$189,000	\$0	\$189,000

Permits

Number Issue Date Description Amount Fee

Notice: This does not necessarily represent all the permits for this property. Click the following link to check for additional permit data in Port Saint Lucie

All information is believed to be correct at this time, but is subject to change and is provided without any warranty. © Copyright 2022 Saint Lucie County Property Appraiser. All rights reserved.

Appendix B
Port St. Lucie Code of Ordinances

- (A) **Purpose.** The purpose of the professional zoning district (P) shall be to locate and establish areas within the City which are deemed to be uniquely suited for the development and maintenance of professional office facilities; to designate those uses and services deemed appropriate and proper for location and development within said zoning district; and to establish such development standards and provisions as are appropriate to ensure proper development and functioning of uses within the district. This district includes those uses formerly designated professional commercial.
- (B) **Permitted Principal Uses and Structures.** The following principal uses and structures are permitted:
 - (1) Office for administrative, business, or professional use, barber or beauty shop, but not including the sale or storage of merchandise except where clearly incidental to and an accessory component of the rendering of professional services.
 - (2) Studio for professional work of any form of fine arts or performing arts, but not including the sale or storage of merchandise except where clearly incidental to, and an accessory component of, the rendering of professional services.
 - (3) Enclosed assembly area 3,000 square feet or less, with or without an alcoholic beverage license for on premises consumption of alcoholic beverages, in accordance with Chapter 110.
 - (4) One dwelling unit contained within the development which is incidental to and designed as an integral part of the principal structure.
- (C) **Special Exception Uses.** The following uses may be permitted only following the review and specific approval thereof by the City Council:
 - (1) Any building exceeding thirty-five (35) feet in height.
 - (2) Model home centers.
 - (3) Enclosed assembly area over 3,000 square feet, with or without an alcoholic beverage license for on premises consumption of alcoholic beverages, in accordance with <u>Chapter 110</u>.
 - (4) Any use set forth in subsection (B): "Permitted Principal Uses and Structures" that include drive-through service.
- (D) Accessory Uses. As set forth within section 158.217.
- (E) **Minimum Lot Requirements.** Twenty thousand (20,000) square feet and a minimum width of one hundred (100) feet. More than one (1) permitted or special exception use may be located upon the lot as part of a totally-designed development to be maintained under single ownership. Properties located within conversion areas as defined by this chapter shall meet the requirements contained within the City of Port St. Lucie Land Use Conversion Manual.

Maximum Building Coverage. Forty (40) percent, provided that the combined area coverage of all impervious surfaces shall not exceed eighty (80) percent.

- (G) Maximum Building Height. Thirty-five (35) feet, except for the ROI (Residential, office and institutional) conversion area as identified in the City of Port St. Lucie Land Use Conversion Manual, lying between Airoso Boulevard and U.S. #1 where the maximum building height shall be one (1) story. (See subsection 158.174(E) for height variations allowed through PUD zoning.)
- (H) **Minimum Building Size and Minimum Living Area.** Commercial and office buildings shall have a minimum total gross floor area of one thousand two hundred (1,200) square feet. Apartment-type unit six hundred (600) square feet.
- (I) Setback Requirements and Landscaping.
 - (1) **Front Yard.** Each lot shall have a front yard with a building setback line of twenty-five (25) feet.
 - (2) **Side Yards.** Each lot shall have two (2) side yards, each of which shall have a building setback line of ten (10) feet. A building setback line of twenty-five (25) feet shall be required when the yard adjoins a residential future land use category or a public right-of-way.
 - (3) **Rear Yard.** Each lot shall have a rear yard with a building setback line of ten (10) feet. A building setback line of twenty-five (25) feet shall be required when the yard adjoins a residential future land use category or a public right-of-way.
 - (4) Landscaping Requirements. Landscaping and buffering requirements are subject to <u>Chapter 154</u>. All mechanical equipment shall be screened from property zoned residential. This screening shall be designed as both a visual barrier and a noise barrier.
- () Off-Street Parking and Service Requirements. As set forth in section 158.221.
- (K) **Site Plan Review.** All permitted and special exception uses shall be subject to the provisions of sections <u>158.235</u> through <u>158.245</u>.

(Ord. No. 98-84, § 1, 3-22-99; Ord. No. 02-124, § 1, 11-12-02; Ord. No. 06-81, § 1, 8-14-06; Ord. No. 11-79, § 1(Exh. A), 11-14-11; Ord. No. 12-09, § 1(Exh. A), 3-12-12; Ord. No. 15-85, § 1, 12-7-15; Ord. No. 20-25, § 2, 5-11-20)

Sec. 158.124. - General Commercial Zoning District (CG).

(A) Purpose. The purpose of the general commercial zoning district (CG) shall be to locate and establish areas within the City which are deemed to be uniquely suited for the development and maintenance of general commercial facilities. Said areas to be primarily along established highways where a mixed pattern of commercial usage is substantially established; to designate those uses and services deemed appropriate and proper for location and development standards

and provisions as are appropriate to ensure proper development and functioning of uses within the district. This district incorporates most of those uses formerly designated shopping center commercial (CSC) and resort commercial (CR).

- (B) Permitted Principal Uses and Structures. The following principal uses and structures are permitted.
 - (1) Any retail, business, or personal service use (including repair of personal articles, furniture, and household appliances) conducted wholly within an enclosed building, where repair, processing, or fabrication of products is clearly incidental to and restricted to on-premises sales.
 - (2) Horticultural nursery, garden supply sales, or produce stand.
 - (3) Office for administrative, business, or professional use.
 - (4) Public facility or use.
 - (5) Restaurants with or without an alcoholic beverage license for on premises consumption of alcoholic beverages in accordance with <u>Chapter 110</u>.
 - (6) Retail sales of alcoholic beverages for incidental on and off premises consumption in accordance with <u>Chapter 110</u>.
 - (7) Park or playground or other public recreation.
 - (8) Motel, hotel, or motor lodge.
 - (9) Enclosed assembly area 3,000 square feet or less, with or without an alcoholic beverage license for on premises consumption of alcoholic beverages, in accordance with <u>Chapter 110</u>.
 - (10) Brewpub. provided no more than 10,000 kegs (5,000 barrels) of beer are made per year, in accordance with <u>Chapter 110</u>.
 - (11) One dwelling unit contained within the development which is incidental to and designed as an integral part of the principal structure.
 - (12) Kennel, enclosed.
 - (13) Medical Marijuana Dispensing Facilities as set forth in <u>Chapter 120</u>.
 - (14) Pharmacy.
- (C) Special Exception Uses. The following uses may be permitted only following the review and specific approval thereof by the City Council:
 - (1) Enclosed assembly area over 3,000 square feet, with or without an alcoholic beverage license for on premises consumption of alcoholic beverages, in accordance with <u>Chapter 110</u>.
 - (2) Public utility facility, including water pumping plant, reservoir, and electrical substation, and sewage treatment plant.
 - (3) Semi-public facility or use.

- (4) Car wash (full or self-service).
- (5) Kennel, enclosed with outdoor runs.
- (6) Bars, lounges, and night clubs.
- (7) Schools (public, private or parochial) or technical or vocational schools.
- (8) Automobile, truck, boat and/or farm equipment sales. No storage or display of vehicles shall be permitted outside an enclosed building unless an area for such use is designated on the approved site plan and does not reduce the required number of parking spaces for the building.
- (9) Automobile fuel sales.
- (10) Repair and maintenance of vehicles. No storage of vehicles shall be permitted outside of an enclosed building unless an area designated for such use is on the approved site plan and does not reduce the required number of parking spaces for the building.
- (11) Retail convenience stores with or without fuel service station.
- (12) Hospitals, free standing emergency department, nursing, or convalescent homes.
- (13) Any use set forth in Subsection B: "Permitted Principal Uses and Structures" that include drive-through service.
- (14) Pain management clinic as set forth in <u>Section 158.231</u>.
- (D) Accessory Uses. As set forth within section 158.217.
- (E) Minimum Lot Requirements. Twenty thousand (20,000) square feet and a minimum width of one hundred (100) feet. More than one (1) permitted or special exception use may be located upon the lot as part of a totally-designed development. Properties located within conversion areas as defined by this chapter shall meet the requirements contained within the City of Port St. Lucie Land Use Conversion Manual.
- (F) (Maximum Building Coverage. Forty (40%) percent, provided that the combined area coverage of all impervious surfaces shall not exceed eighty (80%) percent.
- (G) Maximum Building Height. Thirty-five (35) feet. (See subsection <u>158.174</u>(E) for height variations allowed through PUD zoning.)
- (H) Minimum Building Size and Minimum Living Area. Commercial and office buildings shall have a minimum total gross floor area of one thousand two hundred (1,200) square feet. For automobile service stations and drive-through restaurants: nine hundred (900) square feet.
- (I) Setback Requirements and Landscaping.
 - (1) Front Setback. Each lot shall have a front yard with a building setback line of twenty-five (25) feet.

Appendix C
St. Lucie TPO and CP Data

			2022	Last Physical	Pk Hr	AM I	Pk Hr Pk C	k Hr Pk Dir PM Pk Hr Pk			Dir
Roadway Name	Location	STATION ID	AADT *	Count Year	Service Capacity	Volume	LOS	V/C	Volume	LOS	V/C
CALIFORNIA BLVD	SAVONA BLVD to DEL RIO BLVD	635	12,982	2020	920	796	С	0.865	653	С	0.71
CALIFORNIA BLVD	DEL RIO BLVD to CROSSTOWN PKWY	636	16,500	2022	920	1,005	F	1.092	873	D	0.949
CALIFORNIA BLVD	CROSSTOWN PKWY to HEATHERWOOD BLVD	234	18,000	2022	920	872	D	0.948	984	F	1.07
CALIFORNIA BLVD	HEATHERWOOD BLVD to ST LUCIE WEST BLVD	234	18,000	2022	920	872	D	0.948	984	F	1.07
CALIFORNIA BLVD	ST LUCIE WEST BLVD to COUNTRY CLUB DR	233	9,100	2022	920	488	С	0.53	484	С	0.526
CALIFORNIA BLVD	COUNTRY CLUB DR to UNIVERSITY BLVD	724	7,300	2022	790	497	С	0.629	436	С	0.552
CALIFORNIA BLVD	UNIVERSITY BLVD to PEACOCK BLVD	724	7,300	2022	630	497	С	0.789	436	С	0.692
CALIFORNIA BLVD	PEACOCK BLVD to TORINO PKWY	637	12,953	2020	630	785	F	1.246	776	F	1.232
CAMEO BLVD	PORT ST LUICE BLVD to CALIFORNIA BLVD	638	4,900	2019	750	400	D	0.533	299	C	0.399
CAMEO BLVD	CALIFORNIA BLVD to CROSSTOWN PKWY	639	10,143	2020	790	717	D	0.908	603	D	0.763
CAMPBELL RD	PICOS RD to ORANGE AVE	640	850	2022	540	70	С	0.13	69	С	0.128
CANE SLOUGH RD	US 1 to LENNARD RD	167	9,619	2020	1,710	487	С	0.285	490	С	0.287
CARLTON RD	CARLTON RD (S) to OKEECHOBEE RD	641	750	2022	390	65	В	0.167	60	В	0.154
CASHMERE BLVD	PEACOCK BLVD to TORINO PKWY	676	11,321	2020	630	742	F	1.178	690	F	1.095
CASHMERE BLVD	DEL RIO BLVD to CROSSTOWN PKWY	642	10,621	2020	920	653	С	0.71	640	С	0.696
CASHMERE BLVD	CROSSTOWN PKWY to HEATHERWOOD BLVD	232	12,370	2020	920	690	С	0.75	605	С	0.658
CASHMERE BLVD	HEATHERWOOD BLVD to ST LUCIE WEST BLVD	232	12,370	2020	920	690	С	0.75	605	С	0.658
CASHMERE BLVD	ST LUCIE WEST BLVD to PEACOCK BLVD	231	14,626	2020	920	1,026	F	1.115	993	F	1.079
CITRUS AVE	7TH ST to US 1	643	1,158	2018	750	161	С	0.215	161	С	0.215
CITRUS AVE	US 1 to 2ND ST	940160	4,374	2020	790	219	С	0.277	223	С	0.282
CITRUS AVE	2ND ST to INDIAN RIVER DR	644	4,300	2022	540	262	С	0.485	264	С	0.489
COMMUNITY BLVD	WESTCLIFFE LN to TRADITION PKWY	647	5,886	2020	1,470	325	С	0.221	339	С	0.231
COMMERCE CENTER DR	CROSSTOWN PKWY to ST LUCIE WEST BLVD	645	5,050	2020	1,710	300	С	0.175	362	С	0.212
COMMERCE CENTER DR	ST LUCIE WEST BLVD to GLADES CUT-OFF RD	646	9,900	2022	540	529	D	0.98	607	F	1.124
CORTEZ BLVD	35TH ST to 25TH ST	948500	2,042	2020	750	99	С	0.132	99	С	0.132

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			2022	Last Physical	Pk Hr	AM I	AM Pk Hr Pk Dir PM Pk Hr P			Pk Hr Pk I	Dir
Roadway Name	Location	STATION ID	AADT *	Count Year	Service Capacity	Volume	LOS	V/C	Volume	LOS	V/C
CORTEZ BLVD	25TH ST to SUNRISE BLVD	648	3,100	2018	750	222	С	0.296	203	С	0.271
COUNTRY CLUB DR	ST LUCIE WEST BLVD to CALIFORNIA BLVD	725	8,879	2019	1,710	572	С	0.335	523	С	0.306
CROSSTOWN PKWY	COMMERCE CENTER DR to I-95	650	17,097	2021	3,170	1,062	С	0.335	911	С	0.287
CROSSTOWN PKWY	I-95 to CALIFORNIA BLVD	651	29,056	2020	3,170	1,641	С	0.518	1,845	С	0.582
CROSSTOWN PKWY	CALIFORNIA BLVD to CASHMERE BLVD	652	29,435	2020	3,170	1,659	С	0.523	1,560	С	0.492
CROSSTOWN PKWY	CASHMERE BLVD to CAMEO BLVD	653	32,571	2020	3,170	1,763	С	0.556	1,694	С	0.534
CROSSTOWN PKWY	CAMEO BLVD to BAYSHORE BLVD	654	37,282	2020	3,170	1,854	С	0.585	1,943	С	0.613
CROSSTOWN PKWY	BAYSHORE BLVD to AIROSO BLVD	655	28,807	2020	3,170	1,497	С	0.472	1,470	С	0.464
CROSSTOWN PKWY	AIROSO BLVD to SANDIA DR	656	15,859	2021	3,170	1,023	С	0.323	872	С	0.275
CROSSTOWN PKWY	SANDIA DR to MANTH LN	657	19,123	2021	3,170	1,030	С	0.325	1,075	С	0.339
CROSSTOWN PKWY	FLORESTA DR to US 1	66	34,000	2022	3,170	2,257	С	0.712	2,055	С	0.648
CROSSROADS PKWY	OKEECHOBEE RD to KINGS HWY	649	2,100	2022	790	106	С	0.134	105	С	0.133
DARWIN BLVD	BECKER RD to PAAR DR	235	7,762	2020	630	691	F	1.097	639	F	1.014
DARWIN BLVD	PAAR DR to TULIP BLVD	235	7,762	2020	920	691	С	0.751	639	С	0.695
DARWIN BLVD	TULIP BLVD to PORT ST LUCIE BLVD	659	12,900	2020	920	608	C	0.661	<mark>603</mark>	C	0.655
DEL RIO BLVD	PORT ST LUCIE BLVD to CALIFORNIA BLVD	311	10,000	2022	920	781	C	0.849	704	C	0.765
DEL RIO BLVD	CALIFORNIA BLVD to CASHMERE BLVD	660	2,300	2022	880	140	С	0.159	139	С	0.158
DEL RIO BLVD	CASHMERE BLVD to CALIFORNIA BLVD	661	5,015	2020	880	267	С	0.303	270	С	0.307
DELAWARE AVE	HARTMAN RD to 33RD ST	662	1,900	2022	600	295	С	0.492	237	С	0.395
DELAWARE AVE	33RD ST to 25TH ST	500	3,100	2022	1,710	206	С	0.12	235	С	0.137
DELAWARE AVE	25TH ST to OKEECHOBEE RD	948526	1,428	2020	1,220	65	С	0.053	65	С	0.053
DELAWARE AVE	OKEECHOBEE RD to 13TH ST	663	11,714	2019	790	641	D	0.811	597	D	0.756
DELAWARE AVE	13TH ST to 10TH ST	664	7,505	2020	750	408	D	0.544	388	D	0.517
DELAWARE AVE	10TH ST to 7TH ST	664	7,505	2020	600	408	D	0.68	388	D	0.647
DELAWARE AVE	7TH ST to US 1	665	7,231	2019	750	392	D	0.523	403	D	0.537

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			2022	Last Physical	Pk Hr	AM I	Pk Hr Pk C	Dir	РМ	Pk Hr Pk [Dir
Roadway Name	Location	STATION ID	AADT *	Count Year	Service Capacity	Volume	LOS	V/C	Volume	LOS	V/C
EAST TORINO PKWY	CASHMERE BLVD to TORINO PKWY	710	12,128	2020	830	715	С	0.861	707	С	0.852
EAST TORINO PKWY	TORINO PKWY to MIDWAY RD	237	14,432	2020	880	936	F	1.064	807	С	0.917
EASY ST	US 1 to BUCHANAN DR	106	7,210	2020	750	400	D	0.533	506	D	0.675
EASY ST	BUCHANAN DR to YUCCA DR	106	7,210	2020	540	400	D	0.741	506	D	0.937
EDWARDS RD	JENKINS RD to MCNEIL RD	174	11,057	2021	630	551	С	0.875	571	С	0.906
EDWARDS RD	MCNEIL RD to SELVITZ RD	174	11,057	2021	700	551	С	0.787	571	С	0.816
EDWARDS RD	SELVITZ RD to 25TH ST	110	15,060	2020	880	757	С	0.86	770	С	0.875
EDWARDS RD	25TH ST to SUNRISE BLVD	108	16,851	2020	1,630	819	D	0.502	820	D	0.503
EDWARDS RD	SUNRISE BLVD to OLEANDER AVE	502	15,790	2018	1,630	783	D	0.48	763	D	0.468
EDWARDS RD	OLEANDER AVE to US 1	173	9,687	2018	1,630	533	С	0.327	465	С	0.285
FARMER'S MARKET RD	OLEANDER AVE to US 1	112	1,792	2018	750	124	С	0.165	121	С	0.161
FLORESTA DR	OAKLYN ST to PORT ST LUCIE BLVD	317	15,000	2022	920	1,038	F	1.128	793	С	0.862
FLORESTA DR	THORNHILL DR to CROSSTOWN PKWY	315	10,500	2022	880	680	С	0.773	620	С	0.705
FLORESTA DR	PORT ST LUCIE BLVD to THORNHILL DR	315	10,500	2022	880	680	С	0.773	620	С	0.705
FLORESTA DR	CROSSTOWN PKWY to PRIMA VISTA BLVD	109	12,190	2020	920	675	С	0.734	620	С	0.674
FLORESTA DR	PRIMA VISTA BLVD to AIROSO BLVD	107	10,623	2020	920	587	С	0.638	648	С	0.704
FLORESTA DR	SELVITZ RD to BAYSHORE BLVD	313	4,660	2020	630	334	С	0.53	356	С	0.565
FLORESTA DR	AIROSO BLVD to SELVITZ RD	313	4,660	2020	880	334	С	0.38	356	С	0.405
FT PIERCE BLVD	INDRIO RD to EMERSON AVE	226	3,729	2018	540	280	D	0.519	286	D	0.53
GARDENIA AVE	OLEANDER AVE to US 1	666	2,700	2022	750	180	С	0.24	192	С	0.256
GATLIN BLVD	W OF I-95 to E OF I-95	945075	47,867	2020	3,170	3,492	F	1.102	2,846	С	0.898
GATLIN BLVD	E OF I-95 to SAVAGE BLVD	945075	47,867	2020	3,170	3,492	F	1.102	2,846	С	0.898
GATLIN BLVD	SAVAGE BLVD to ROSSER BLVD	945075	47,867	2020	3,170	3,492	F	1.102	2,846	С	0.898
GATLIN BLVD	ROSSER BLVD to SAVONA BLVD	945075	47,867	2020	3,170	3,492	F	1.102	<mark>2,846</mark>	C	0.898
GATLIN BLVD	SAVONA BLVD to PORT ST LUCIE BLVD	945075	47,867	2020	3,170	3,492	F	1.102	2,846	C	0.898

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			2022	Last Physical	Pk Hr	AM I	Pk Hr Pk C	Dir	PM	Pk Hr Pk	Dir
Roadway Name	Location	STATION ID	AADT *	Count Year	Service Capacity	Volume	LOS	V/C	Volume	LOS	V/C
ORANGE AVE	10TH ST to 7TH ST	940155	8,912	2020	600	428	D	0.713	492	D	0.82
ORANGE AVE	7TH ST to US 1	945134	7,206	2020	600	441	D	0.735	392	D	0.653
ORANGE AVE	US 1 to 2ND ST	945133	3,934	2020	600	208	С	0.347	204	С	0.34
ORANGE AVE	2ND ST to INDIAN RIVER DR	945133	3,934	2020	750	208	С	0.277	204	С	0.272
PARR DR	PORT ST LUCIE BLVD to DARWIN BLVD	209	3,500	2022	700	257	С	0.367	227	С	0.324
PARR DR	DARWIN BLVD to TULIP BLVD	723	1,954	2019	540	171	С	0.317	130	С	0.241
PARR DR	SAVONA BLVD to PORT ST LUCIE BLVD	209	3,500	2022	700	257	С	0.367	227	С	0.324
PARR DR	ROSSER BLVD to SAVONA BLVD	209	3,500	2022	630	257	С	0.408	227	С	0.36
PEACOCK BLVD	CALIFORNIA BLVD to CASHMERE BLVD	693	5,198	2020	630	329	С	0.522	365	С	0.579
PEACOCK BLVD	UNIVERSITY BLVD to CALIFORNIA BLVD	694	10,952	2020	920	752	С	0.817	616	С	0.67
PEACOCK BLVD	ST LUCIE WEST BLVD to UNIVERSITY BLVD	948514	16,197	2020	2,100	747	С	0.356	747	С	0.356
PETERSON RD	BENT CREEK DR to HARTMAN RD	695	2,300	2022	540	184	С	0.341	157	С	0.291
PICOS RD	CAMPBELL RD to KINGS HWY	696	1,333	2017	540	92	С	0.17	86	С	0.159
PORT ST LUCIE BLVD	MARTIN C.L. to BECKER RD	948519	15,257	2020	920	704	С	0.765	704	С	0.765
PORT ST LUCIE BLVD	BECKER RD to PAAR DR	948519	15,257	2020	920	704	С	0.765	704	С	0.765
PORT ST LUCIE BLVD	PAAR DR to TULIP BLVD	948519	15,257	2020	700	704	С	1.006	704	С	1.006
PORT ST LUCIE BLVD	TULIP BLVD to DARWIN BLVD	948519	15,257	2020	920	704	C	0.765	704	C	0.765
PORT ST LUCIE BLVD	DARWIN BLVD to GATLIN BLVD	<u>697</u>	33,879	2020	3,020	1,733	C	0.574	1,712	C	0.567
PORT ST LUCIE BLVD	GATLIN BLVD to DEL RIO BLVD	<u>698</u>	40,895	2020	3,170	2,221	C	0.701	2,108	C	0.665
PORT ST LUCIE BLVD	DEL RIO BLVD to CAMEO BLVD	945074	49,241	2020	3,170	3,293	F	1.039	2,989	C	0.943
PORT ST LUCIE BLVD	CAMEO BLVD to FLORIDA'S TURNPIKE	945074	49,241	2020	3,020	3,293	F	1.09	2,989	D	0.99
PORT ST LUCIE BLVD	FLORIDA'S TURNPIKE to BAYSHORE BLVD	945074	49,241	2020	3,170	3,293	F	1.039	2,989	С	0.943
PORT ST LUCIE BLVD	BAYSHORE BLVD to AIROSO BLVD	945073	49,214	2020	3,020	3,015	D	0.998	2,987	D	0.989
PORT ST LUCIE BLVD	AIROSO BLVD to FLORESTA DR	940780	50,635	2020	3,020	3,150	F	1.043	2,761	С	0.914
PORT ST LUCIE BLVD	FLORESTA DR to VETERANS MEMORIAL PKWY	940778	58,844	2020	3,020	4,354	F	1.442	3,247	F	1.075

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			2022	Last Physical	Pk Hr	AM I	Pk Hr Pk C	Dir	РМ	Pk Hr Pk I	Dir
Roadway Name	Location	STATION ID	AADT *	Count Year	Service Capacity	Volume	LOS	V/C	Volume	LOS	V/C
PORT ST LUCIE BLVD	VETERANS MEMORIAL PKWY to MORNINGSIDE BLVD	940776	38,628	2020	3,020	2,568	С	0.85	2,278	С	0.754
PORT ST LUCIE BLVD	MORNINGSIDE BLVD to US 1	945072	39,257	2020	3,170	3,013	С	0.95	1,879	С	0.593
PRIMA VISTA BLVD	BAYSHORE BLVD to AIROSO BLVD	314	24,567	2020	2,100	1,080	С	0.514	1,185	С	0.564
PRIMA VISTA BLVD	AIROSO BLVD to FLORESTA DR	150	24,142	2021	2,100	1,112	С	0.53	1,042	С	0.496
PRIMA VISTA BLVD	FLORESTA DR to NARANJA AVE	148	34,321	2020	2,100	1,975	С	0.94	1,879	С	0.895
PRIMA VISTA BLVD	NARANJA AVE to RIO MAR DR	148	34,321	2020	2,000	1,975	D	0.988	1,879	С	0.94
PRIMA VISTA BLVD	RIO MAR DR to US 1	146	23,876	2021	2,100	1,160	С	0.552	1,059	С	0.504
PRIMA VISTA BLVD	US 1 to LENNARD RD	699	8,508	2020	1,710	460	С	0.269	438	С	0.256
RANGE LINE RD	MARTIN C.L. to BECKER RD	145	1,741	2018	1,080	117	В	0.108	116	В	0.107
RANGE LINE RD	BECKER RD to 2 MI S OF GLADES CUT-OFF RD	145	1,741	2018	1,080	117	В	0.108	116	В	0.107
RANGE LINE RD	2 MI S OF GLADES CUT-OFF RD to GLADES CUT-OF	145	1,741	2018	1,080	117	В	0.108	116	В	0.107
RIO MAR DR	PRIMA VISTA BLVD to BEACH AVE	147	5,726	2019	750	354	С	0.472	372	D	0.496
RIO MAR DR	BEACH AVE to US 1	147	5,726	2019	790	354	С	0.448	372	С	0.471
ROSSER BLVD	APRICOT RD to GATLIN BLVD	948510	4,020	2020	920	185	С	0.201	185	С	0.201
ROSSER BLVD	PAAR DR to APRICOT RD	948510	4,020	2020	1,070	185	В	0.173	185	В	0.173
SAVONA BLVD	BECKER RD to PAAR DR	236	10,556	2020	790	939	F	1.189	844	F	1.068
SAVONA BLVD	PAAR DR to GATLIN BLVD	236	10,556	2020	750	939	F	1.252	844	F	1.125
SAVONA BLVD	GATLIN BLVD to CALIFORNIA BLVD	702	14,200	2020	790	<mark>659</mark>	D	0.834	<mark>695</mark>	D	0.88
SAVAGE BLVD	GATLIN BLVD to GALIANO RD	168	4,244	2018	920	280	С	0.304	225	С	0.245
SAVANNAH RD	US 1 to INDIAN RIVER DR	514	2,157	2018	540	153	С	0.283	151	С	0.28
SELVITZ RD	BAYSHORE BLVD to ST JAMES BLVD	948501	9,240	2020	750	450	D	0.6	450	D	0.6
SELVITZ RD	ST JAMES BLVD to MIDWAY RD	948501	9,240	2020	750	450	D	0.6	450	D	0.6
SELVITZ RD	MIDWAY RD to GLADES CUT-OFF RD	703	10,073	2020	700	590	С	0.843	594	С	0.849
SELVITZ RD	GLADES CUT-OFF RD to EDWARDS RD	704	14,479	2020	790	767	D	0.971	764	D	0.967
SHINN RD	MIDWAY RD to OKEECHOBEE RD	705	450	2022	580	29	С	0.05	28	С	0.048

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			2022	Last Physical	Pk Hr	AM I	AM Pk Hr Pk Dir PM Pk Hr			Pk Hr Pk I	Dir
Roadway Name	Location	STATION ID	AADT *	Count Year	Service Capacity	Volume	LOS	V/C	Volume	LOS	V/C
ST LUCIE WEST BLVD	COUNTRY CLUB DR to CASHMERE BLVD	318	37,500	2022	2,100	1,770	С	0.843	1,770	С	0.843
ST LUCIE WEST BLVD	CASHMERE BLVD to BAYSHORE BLVD	316	51,315	2020	3,170	2,250	С	0.71	2,397	С	0.756
SUNRISE BLVD	MIDWAY RD to BELL AVE	155	3,593	2020	540	216	С	0.4	229	С	0.424
SUNRISE BLVD	BELL AVE to EDWARDS RD	153	4,400	2022	750	292	С	0.389	330	С	0.44
SUNRISE BLVD	EDWARDS RD to CORTEZ BLVD	511	6,678	2019	600	591	D	0.985	471	D	0.785
SUNRISE BLVD	CORTEZ BLVD to VIRGINIA AVE	511	6,678	2019	750	591	D	0.788	471	D	0.628
SUNRISE BLVD	VIRGINIA AVE to OLEANDER AVE	509	5,172	2019	750	406	D	0.541	401	D	0.535
SUNRISE BLVD	OLEANDER AVE to 7TH ST	708	5,100	2022	1,540	318	С	0.206	369	С	0.24
SUNRISE BLVD	7TH ST to US 1	708	5,100	2022	1,710	318	С	0.186	369	С	0.216
TIFFANY AVE	US 1 to HILLMOOR DR	322	15,000	2022	2,100	855	С	0.407	862	С	0.41
TIFFANY AVE	HILLMOOR DR to VILLAGE GREEN DR	322	15,000	2022	2,100	855	С	0.407	862	С	0.41
TIFFANY AVE	VILLAGE GREEN DR to LENNARD RD	320	4,308	2020	2,100	209	С	0.1	203	С	0.097
TORINO PKWY	CASHMERE BLVD to CALIFORNIA BLVD	709	6,940	2020	630	444	С	0.705	405	С	0.643
TORINO PKWY	CALIFORNIA BLVD to EAST TORINO PKWY	238	4,824	2020	630	318	С	0.505	260	С	0.413
TRADITION PKWY	COMMUNITY BLVD to VILLAGE PKWY	711	7,380	2020	1,710	772	D	0.451	748	С	0.437
TRADITION PKWY	VILLAGE PKWY to W OF I-95	712	40,000	2022	3,170	2,019	С	0.637	2,189	С	0.691
TULIP BLVD	DARWIN BLVD to PORT ST LUCIE BLVD	713	9,300	2022	790	594	D	0.752	518	D	0.656
TULIP BLVD	PORT ST LUCIE BLVD to PAAR DR	714	10,127	2020	790	<u>566</u>	D	0.716	606	D	0.767
TULIP BLVD	PAAR DR to DARWIN BLVD	714	10,127	2020	790	566	D	0.716	606	D	0.767
TURNPIKE FEEDER RD	TURNPIKE FEEDER RD SB RAMP to US 1	940078	4,937	2015	660	469	С	0.711	469	С	0.711
TURNPIKE FEEDER RD	INDIAN PINES BLVD to TURNPIKE FEEDER RD SB R	940269	10,885	2020	870	718	С	0.825	658	С	0.756
TURNPIKE FEEDER RD	INDRIO RD to INDIAN PINES BLVD	940745	12,742	2020	870	704	С	0.809	740	С	0.851
US 1	MARTIN C.L. to LENNARD RD	945071	46,725	2020	4,240	2,246	С	0.53	2,641	С	0.623
US 1	LENNARD RD to PORT ST LUCIE BLVD	945071	46,725	2020	4,040	2,246	С	0.556	2,641	С	0.654
US 1	PORT ST LUCIE BLVD to JENNINGS RD	945070	28,759	2020	3,020	1,426	С	0.472	1,513	С	0.501

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Table 2-1 Roadway System Classifications

			Road Classification	(Urban)
			Federal Highway Administration	City
Road Name	From	То	Adjusted Urban Area 2010	,
AIROSO BLVD	PRIMA VISTA BLVD	ST JAMES DR	Principal Arterial - Other	Principal Arterial
AIROSO BLVD	PORT ST LUCIE BLVD	PRIMA VISTA BLVD	Minor Arterial	Minor Arterial
ALCANTARRA BLVD	SAVONA BLVD	PORT ST LUCIE BLVD	Major Collector	Collector
ALEDO LN	ROSSER BLVD	BRIGINTINE PL	NDA	Collector
BAYSHORE BLVD	PRIMA VISTA BLVD	ST JAMES DR	Minor Arterial	Minor Arterial
BAYSHORE BLVD	OAKRIDGE BLVD	PORT ST LUCIE BLVD	Minor Collector	Collector
BAYSHORE BLVD	PORT ST LUCIE BLVD	PRIMA VISTA BLVD	Principal Arterial - Other	Principal Arterial
BECKER RD	WESTERN END	GILSON RD	Minor Arterial	Minor Arterial
BILTMORE ST	MACEDO BLVD	THORNHILL DR	NDA	Collector
CALIFORNIA BLVD	W TORINO BLVD	DEL RIO BLVD	Minor Arterial	Minor Arterial
CAMEO BLVD	PORT ST LUCIE BLVD	CROSSTOWN PKWY	Minor Collector	Collector
CANE SLOUGH RD	US-1/SR-5	LENNARD RD	Minor Arterial	Minor Arterial
CASHMERE BLVD	RAB @ PEACOCK BLVD	DEL RIO BLVD	Major Collector	Collector
CASHMERE BLVD	E TORINO PKWY	RAB @ PEACOCK BLVD	Minor Collector	Collector
COMMERCE CENTRE DR	CROSSTOWN PKWY	RANGE LINE RD	Major Collector	Collector
COMMUNITY BLVD	DISCOVERY WAY	WESTCLIFFE LN	Minor Collector	Collector
CROSSTOWN PKWY	BAYSHORE DR	US-1	Minor Arterial	Principal Arterial
CROSSTOWN PKWY	VILLAGE PKWY	I-95	Minor Arterial	Principal Arterial
CROSSTOWN PKWY	VILLAGE PKWY	US-1/SR-5	NDA	Principal Arterial
CROSSTOWN PKWY	I-95	BAYSHORE BLVD	Principal Arterial - Other	Principal Arterial
DARWIN BLVD	BECKER RD	PORT ST LUCIE BLVD	Major Collector	Collector
DEL RIO BLVD	PORT ST LUCIE BLVD	MACKENZIE ST	Major Collector	Collector
E TORINO PKWY	CALIFORNIA BLVD	MIDWAY DR	Minor Arterial	Minor Arterial
FLORESTA DR	AIROSO BLVD	BAYSHORE BLVD	Major Collector	Collector
FLORESTA DR	OAKRIDGE BLVD	AIROSO BLVD	Minor Arterial	Minor Arterial
FLORIDA'S TURNPIKE	SOUTH CITY LIMITS	NORTH CITY LIMITS	Major Arterial – F&E	NDA
GATLIN BLVD	(I-95)	PORT ST LUCIE BLVD	Principal Arterial - Other	Principal Arterial

		Road Classification	(Urban)	
			Federal Highway Administration	City
Road Name	From	То	Adjusted Urban Area 2010	
GLADES CUT-OFF ROAD	CARLTON RD	RANGE LINE RD	Major Collector	Collector
GLADES CUT-OFF ROAD	RANGE LINE RD	MIDWAY RD	Minor Arterial	Minor Arterial
GOWIN DR	PORT ST LUCIE BLVD	WESTMORELAND BLVD	NDA	Collector
GRAND DR	LENNARD RD	WALTON RD	Minor Collector	Collector
GREEN RIVER PKWY	MARTIN CO LINE	WALTON RD	Minor Arterial	Minor Arterial
HEATHERWOOD BLVD	CALIFORNIA BLVD	CASHMERE BLVD	Minor Collector	Collector
HILLMOOR DR	LENNARD RD	TIFFANY AVE	Minor Collector	Collector
I-95	SOUTH CITY LIMITS	NORTH CITY LIMITS	Major Arterial - Interstate	NDA
IMPORT DR	SAVAGE BLVD	GATLIN BLVD	Major Collector	Collector
IMPORT DR	GATLIN BLVD	ALEDO LN	NDA	Collector
INDIAN RIVER DR	NORTH CITY LIMIT	SOUTH CITY LIMIT	Major Collector	Collector
JENNINGS RD	US-1/SR-5	LENNARD RD	Major Collector	Collector
LAKEHURST DR	BAYSHORE BLVD	SANDIA DR	Major Collector	Collector
LENNARD RD	US-1/SR-5	WALTON RD	Minor Arterial	Minor Arterial
LENNARD RD	PRIMA VISTA BLVD	KITTERMAN RD	Minor Collector	Collector
LTC PARKWAY	MIDWAY RD	GLADES CUT OFF RD	<u>NDA</u>	Collector
LYNGATE DR	MIDPORT RD	US-1/SR-5	Major Collector	Collector
MANVILLE DR	SELVITZ RD	ST JAMES DR	Major Collector	Collector
MARIPOSA AVE	LENNARD RD	CALAIS ST	Minor Collector	Collector
MELALEUCA BLVD	LENNARD RD	GREEN RIVER PKWY	Minor Collector	Collector
MIDWAY RD	WESTERN CITY LIMITS	EASTERN CITY LIMITS	Principal Arterial - Other	Principal Arterial
MORNINGSIDE BLVD	WESTMORELAND RAB	LYNGATE DR	Major Collector	Collector
MORNINGSIDE BLVD	RIVER VISTA DR	WESTMORELAND RAB	NDA	Collector
N MACEDO BLD	SELVITZ RD	BAYSHORE BLVD	NDA	Collector
N TORINO PKWY	BLANTON BLVD	E TORINO PKWY	Major Collector	Collector
OAKRIDGE BLVD	BAYSHORE BLVD	SOUTHBEND BLVD	Minor Collector	Collector
PAAR DR	BAMBERG ST	DARWIN BLVD	Minor Collector	Collector
PEACHTREE BLVD	SELVITZ RD	ST JAMES BLVD	Major Collector	Collector
PEACOCK BLVD	CALIFORNIA BLVD RAD	ST LUCIE WEST BLVD	Major Collector	Collector

			Road Classification (Urban)					
			Federal Highway Administration	City				
Road Name	From	То	Adjusted Urban Area 2010					
PORT ST LUCIE BLVD	MARTIN COUNTY LINE	BECKER RD	Minor Arterial	Minor Arterial				
PORT ST LUCIE BLVD	BECKER RD	US 1/SR 5	Principal Arterial - Other	Principal Arterial				
PRIMA VISTA BLVD	BAYSHORE BLVD	US 1/SR 5	Principal Arterial - Other	Principal Arterial				
RANGE LINE ROAD	GLADES CUT-OFF ROAD	SOUTHERN CITY LIMITS	Minor Arterial	Minor Arterial				
RESERVE BLVD	COMMERCE CENTRE DR RAB	I-95 SB OFF-RAMP	Major Collector	Collector				
ROSSER BLVD	BAMBERG ST	GATLIN BLVD	Major Collector	Collector				
S MACEDO BLVD	BAYSHORE BLVD	THORNHILL DR	NDA	Collector				
SANDIA DR	THORNHILL DR	PRIMA VISTA BLVD	Major Collector	Collector				
SAVAGE BLVD	SR 9/I-95	GATLIN BLVD	Major Collector	Collector				
SAVONA BLVD	BECKER RD	CALIFORNIA BLVD	Minor Arterial	Minor Arterial				
SELVITZ RD	BAYSHORE BLVD	MIDWAY RD	Minor Arterial	Minor Arterial				
SELVITZ RD	FLORESTA DR	BAYSHORE BLVD	Minor Collector	Collector				
SOUTHBEND BLVD	BECKER RD	OAKRIDGE DR	Minor Arterial	Minor Arterial				
ST JAMES DR	AIROSO BLVD	MIDWAY RD	Principal Arterial - Other	Principal Arterial				
ST LUCIE WEST BLVD	I-95 SB OFF-RAMP	BAYSHORE BLVD	Principal Arterial - Other	Principal Arterial				
THORNHILL DR	BAYSHORE BLVD	FLORESTA DR	Major Collector	Collector				
TIFFANY AVE	US-1/SR-5	LENNARD RD	Major Collector	Collector				
TRADITION PKWY	ABINGDON RAB	I-95 SB OFF-RAMP	Minor Arterial	Minor Arterial				
TULIP BLVD	PORT ST LUCIE BLVD	PORT ST LUCIE BLVD	Major Collector	Collector				
UNIVERSITY BLVD	PEACOCK BLVD	CALIFORNIA BLVD	Minor Collector	Collector				
US-1/SR-5	SOUTH CITY LIMITS	NORTH CITY LIMITS	Principal Arterial - Other	Principal Arterial				
VETERANS MEM PKWY	PORT ST LUCIE BLVD	US-1/SR-5	Minor Arterial	Minor Arterial				
VILLAGE GREEN DR	TIFFANY AVE	US-1/SR-5	Major Collector	Collector				
VILLAGE PKWY	BECKER RD	CROSSTOWN PKWY	Minor Arterial	Minor Arterial				
W TORINO PKWY	CALIFORNIA BLVD	BLANTON BLVD	Major Collector	Collector				
WALTON RD	US-1/SR-5	INDIAN RIVER DR	Minor Arterial	Minor Arterial				
WESTCLIFFE LN	VILLAGE PARKWAY	COMMUNITY BLVD	Minor Collector	Collector				
WESTMORELAND BLVD	US-1/SR-5	PORT ST LUCIE BLVD	Major Collector	Collector				
WHITMORE DR	BAYSHORE BLVD	CUL-DE-SAC	Major Collector	Collector				

Appendix D
Turning Movement Counts and 2021 Peak Seasonal Factor Report

15 MINUTE TURNING MOVEMENT COUNTS

(Cars and Trucks)

DATE: December 15, 2022 (Thursday)

LOCATION: Port St Lucie Bv & Aviation Av

CITY: Port St Lucie

LATITUDE: 0

COUNTY: St Lucie County **LONGITUDE:** 0

		Port	t St Luc	cie Rv			Port	t St Luc	cie Rv				Δ	viation	ιΔv		1	Δι	viation	ιΔν		-	ļ
						<u>i</u>						т					<u></u>					 1 FAM	Tenane
TIME BEGIN	⊢—	NO T	ORTHBO	UND U-turn	TOTAL	 	SO	UTHBOU	UND U-turn	TOTAL	N/S TOTAL	L	EA	ASTBOU R	U-turn	TOTAL	L	T T	/ESTBOU R	JND U-turn	TOTAL	E/W TOTAL	GRAND TOTAL
	<u>-</u>				1.4					1.4.11.1						1.4							
07:00 AM	3	237	0	0	240	6	228	7	0	241	481	9	2	4	0	15	0	0	24	0	24	39	520
07:15 AM	4	318	1	0	323	11	307	17	2	337	660	12	0	9	0	21	1	0	21	0	22	43	703
07:30 AM	2	281	1	0	284	3	217	10	1	231	515	9	1	5	0	15	1	0	12	0	13	28	543
07:45 AM	0	354	2	1	357	8	257	14	3	282	639	18	0	4	0	22	2	1	34	0	37	59	698
TOTAL	9	1,190	4	1	1,204	28	1,009	48	6	1,091	2,295	48	3	22	0	73	4	1	91	0	96	169	2,464
08:00 AM	0	338	1	0	339	7	279	16	2	304	643	13	0	8	0	21	0	0	31	0	31	52	695
08:15 AM	7	302	4	0	313	12	315	19	0	346	659	12	0	2	0	14	0	0	25	0	25	39	698
08:30 AM	13	214	5	0	232	8	316	28	1	353	585	9	0	14	0	23	1	1	13	0	15	38	623
08:45 AM	6	284	1	0	291	12	263	25	2	302	593	11	0	7	0	18	0	0	27	0	27	45	638
TOTAL	26	1,138	11	0	1,175	39	1,173	88	5	1,305	2,480	45	0	31	0	76	1	1	96	0	98	174	2,654
04:00 PM	5	295	3	0	303	28	436	19	4	487	790	13	1	5	0	19	0	0	16	0	16	35	825
04:15 PM	4	303	2	0	309	27	339	28	1	395	704	21	1	13	0	35	0	0	16	0	16	51	755
04:30 PM	5	300	4	0	309	27	410	27	3	467	776	22	1	5	0	28	0	1	24	0	25	53	829
04:45 PM	5	323	7	0	335	36	428	37	2	503	838	17	2	8	0	27	1	0	21	0	22	49	887
TOTAL	19	1,221	16	0	1,256	118	1,613	111	10	1,852	3,108	73	5	31	0	109	1	1	77	0	79	188	3,296
05:00 PM	2	301	4	0	307	34	331	33	4	402	709	19	T 1	3	0	23	0	Π1	22	1 0	23	46	755
05:15 PM	5	312	12	1	330	35	428	48	1	512	842	22	1	8	0	31	0	0	31	0	31	62	904
05:30 PM	4	282	9	0	295	37	416	39	2	494	789	16	1	11	0	28	0	1	28	0	29	57	846
05:45 PM	1	231	2	0	234	48	381	28	5	462	696	10	1	5	0	16	1	1 1	38	0	40	56	752
TOTAL	12	1,126	27	1	1,166	154	1,556	148	12	1,870	3,036	67	4	27	0	98	1	3	119	0	123	221	3,257
AM Peak																					Paak Ho	ur Factor:	r: 0.972
07:45 AM to 08:45 AM	20	1,208	12	1	1,241	35	1,167	77	6	1,285	2,526	52	0	28	0	80	3	2	103	0	108	188	2,714
PM Peak	<u></u>	<u>—</u>	Щ	<u></u>		<u></u>	Щ	Ь—	—	Ь		<u> </u>	Щ	Ь—	<u>-</u>		<u>—</u>	Щ	Щ	<u>-</u>	Paak Ho	ur Factor:	: 0.938
04:45 PM to	$\overline{}$	$\overline{}$	$\overline{}$	$\overline{}$	$\overline{}$		$\overline{}$	$\overline{}$	$\overline{}$	$\overline{}$	$\overline{}$	$\overline{}$	Г	T	$\overline{}$		T	$\overline{}$	$\overline{}$				1
05:45 PM	1 16	1,218	32	1	1,267	142	1,603	157	9	1,911	3,178	74	5	30	0	109	1	2	102	0	105	214	3,392
											South										TNC	orui	
							PM	157	1,603	142	9	Lucie			:	į					_	Δ	
1							FIVI	!			,	t≍				į					7		
1							AM i	j 77	1,167	35	6 ♂	Port				į	АМ		PM		L		J
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1		Eastbou				F	Aviation Av	1			7					<u> </u>			<u>1</u>		 Westbound		
		bou		0		0	5	4				<u></u>				Aviation A					est/		
		ind		74 5		52 0	<i>7</i> 1 →	1			Ī										Š		
1				30 PM		28 AM	<u> </u>	Щ.				<u> </u>				1							
1				PM		AM					t St BV		ا 20	个 1,208	71 12	T AM		· -			•		
1								Í			ort ie i	1 ' '	20	1,208	12	AM							

15 MINUTE TURNING MOVEMENT COUNTS

(Trucks Only)

DATE: December 15, 2022 (Thursday)

CITY: Port St Lucie

LOCATION: Port St Lucie Bv & Aviation Av

05:45 PM

COUNTY: St Lucie County

LATITUDE: 0

LONGITUDE: 0

Port St Lucie Bv Port St Lucie Bv **Aviation Av Aviation Av** NORTHBOUND SOUTHBOUND N/S EASTBOUND WESTBOUND GRAND U-turn | TOTAL U-turn TOTAL TOTAL U-turn TOTAL U-turn TOTAL TOTAL BEGIN R TOTAL L R L L 07:00 AM 07:15 AM 07:30 AM 07:45 AM TOTAL 08:00 AM 08:15 AM 08:30 AM 08:45 AM TOTAL 04:00 PM 04:15 PM Ω 04:30 PM 04:45 PM TOTAL 05:00 PM 05:15 PM 05:30 PM 05:45 PM TOTAL AM Peak 7:45 AM to 08:45 AN PM Peak 4:45 PM to

15 MINUTE TURNING MOVEMENT COUNTS

(Cars and Trucks)

DATE: December 15, 2022 (Thursday)

CITY: Port St Lucie

LATITUDE: 0

LOCATION: Port St Lucie Bv & Driveway

COUNTY: St Lucie County LONGITUDE: 0

Port St Lucie Bv Driveway **Port St Lucie Bv** NORTHBOUND SOUTHBOUND EASTBOUND WESTBOUND GRAND U-turn TOTAL U-turn TOTAL BEGIN TOTAL U-turn TOTAL U-turn TOTAL TOTAL TOTAL L R 07:00 AM 07:15 AM 07:30 AM 07:45 AM TOTAL 1,329 1,332 1,091 1,091 2,423 2,428 MA 00:80 08:15 AM 08:30 AM 08:45 AM TOTAL 1,279 1,287 1,305 1,305 2,592 2,592 04:00 PM 04:15 PM Ω 04:30 PM 04:45 PM 1,371 1,852 TOTAL 1,377 1,852 3,229 3,237 05:00 PM 05:15 PM 05:30 PM 05:45 PM 1.870 1,870 3.184 3.191 TOTAL 1.312 1,314 AM Peak 0.963 Peak Hour Factor: 7:45 AM to 1,363 1,369 1,285 1,285 2,654 2,655 08:45 A PM Peak Peak Hour Factor: 0.942 1,394 1,397 1,911 1,911 3,308 3,317 05:45 PN Southbound 1,911 PM ΑМ 1,285 Port St Lucie Bv

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1,363

1,394

ΑМ

РМ

2021 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL

CATEGORY: 9401 CEN.-W OF US1 TO 195

CATEG	ORY: 9401 CENW OF US1 TO	I95	MOGEL 0 05
WEEK	DATES	SF	MOCF: 0.97 PSCF
1	01/01/2021 - 01/02/2021	1.00	1.03
2	01/03/2021 - 01/09/2021	1.01	1.04
3	01/10/2021 - 01/16/2021	1.01	1.04
4	01/17/2021 - 01/23/2021	1.00	1.03
5	01/24/2021 - 01/30/2021	1.00	1.03
6	01/31/2021 - 02/06/2021	0.99	1.02
7 * 8	02/07/2021 - 02/13/2021	0.99	1.02 1.01
* 9	02/14/2021 - 02/20/2021 02/21/2021 - 02/27/2021	0.98 0.97	1.00
*10	02/21/2021 - 02/21/2021	0.97	1.00
*11	03/07/2021 - 03/13/2021	0.96	0.99
*12	03/14/2021 - 03/20/2021	0.95	0.98
*13	03/21/2021 - 03/27/2021	0.96	0.99
*14	03/28/2021 - 04/03/2021	0.96	0.99
*15	04/04/2021 - 04/10/2021	0.97	1.00
*16	04/11/2021 - 04/17/2021	0.97	1.00
*17	04/18/2021 - 04/24/2021	0.98	1.01
*18	04/25/2021 - 05/01/2021	0.98	1.01
*19 *20	05/02/2021 - 05/08/2021 05/09/2021 - 05/15/2021	0.99 0.99	1.02 1.02
21	05/16/2021 - 05/13/2021	0.99	1.02
22	05/23/2021 - 05/29/2021	1.00	1.03
23	05/30/2021 - 06/05/2021	1.00	1.03
24	06/06/2021 - 06/12/2021	1.01	1.04
25	06/13/2021 - 06/19/2021	1.01	1.04
26	06/20/2021 - 06/26/2021	1.02	1.05
27	06/27/2021 - 07/03/2021	1.02	1.05
28	07/04/2021 - 07/10/2021	1.03	1.06
29	07/11/2021 - 07/17/2021	1.03	1.06
30 31	07/18/2021 - 07/24/2021 07/25/2021 - 07/31/2021	1.03 1.04	1.06 1.07
32	08/01/2021 - 07/31/2021	1.04	1.07
33	08/08/2021 - 08/14/2021	1.05	1.08
34	08/15/2021 - 08/21/2021	1.05	1.08
35	08/22/2021 - 08/28/2021	1.05	1.08
36	08/29/2021 - 09/04/2021	1.05	1.08
37	09/05/2021 - 09/11/2021	1.05	1.08
38	09/12/2021 - 09/18/2021	1.05	1.08
39	09/19/2021 - 09/25/2021	1.04	1.07
40	09/26/2021 - 10/02/2021	1.03	1.06 1.04
41 42	10/03/2021 - 10/09/2021 10/10/2021 - 10/16/2021	1.01	1.03
43	10/10/2021 - 10/10/2021 10/17/2021	1.00	1.03
44	10/24/2021 - 10/30/2021	1.01	1.04
45	10/31/2021 - 11/06/2021	1.01	1.04
46	11/07/2021 - 11/13/2021	1.02	1.05
47	11/14/2021 - 11/20/2021	1.02	1.05
48	11/21/2021 - 11/27/2021	1.02	1.05
49	11/28/2021 - 12/04/2021	1.01	1.04
50 51	12/05/2021 - 12/11/2021 12/12/2021 - 12/18/2021	1.01	1.04 1.03
52	12/12/2021 - 12/18/2021	1.00	1.03
53	12/26/2021 - 12/31/2021	1.01	1.04
	,,	-	

^{*} PEAK SEASON

Appendix E HCM Existing Conditions

Intersection															
Int Delay, s/veh	3.4														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations		ર્ન	7		र्स	7		Ä	ተ ተጉ			Ä	ተተጐ		
Traffic Vol. veh/h	52	0	28	3	2	103	1	20	1208	12	6	35	1167	77	
Future Vol, veh/h	52	0	28	3	2	103	1	20	1208	12	6	35	1167	77	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	
RT Channelized	_	-	None	_	_	None	_	_	-	None	-	_	-	None	
Storage Length	-	-	105	_	-	115	-	350	-	-	-	310	-	-	
Veh in Median Storage,	.# -	1	_	_	1	_	-	_	0	_	_	_	0	_	
Grade, %	_	0	_	-	0	_	-	-	0	_	-	_	0	-	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	97	97	
Heavy Vehicles, %	2	3	2	3	3	8	2	2	11	2	2	2	2	2	
Mvmt Flow	54	0	29	3	2	106	1	21	1245	12	6	36	1203	79	
	0.	•			_	100	•		1210				1200	, 0	
Major/Minor N	/linor2		1	Minor1			Major1			N	Major2				
Conflicting Flow All	1870	2628	641	1860	2661	629	936	1282	0	0	918	1257	0	0	
Stage 1	1327	1327	-	1295	1295	-	-		-	-	-	-	-	-	
Stage 2	543	1301	_	565	1366	_	_	_	_	_	_	_	_	_	
Critical Hdwy	6.44	6.56	7.14	6.46	6.56	7.26	5.64	5.34	_	_	5.64	5.34	_	_	
Critical Hdwy Stg 1	7.34	5.56	-	7.36	5.56	7.20	- 0.01	- 0.01	_	_	-	-	_	_	
Critical Hdwy Stg 2	6.74	5.56	_	6.76	5.56	_	_	_	_	_	_	_	_	_	
Follow-up Hdwy	3.82	4.03	3.92	3.83	4.03	3.98	2.32	3.12	_	_	2.32	3.12	_	_	
Pot Cap-1 Maneuver	76	23	358	77	22	353	477	285	_	_	488	294	_	_	
Stage 1	117	221	-	123	229	-	-	200	_	_	- 00	257	_	_	
Stage 2	449	227	_	433	211	_			_			_	_	_	
Platoon blocked, %	עדד	ZZI		700	211				_	_			_	_	
Mov Cap-1 Maneuver	~ 44	18	358	60	17	353	290	290	_	_	301	301		_	
Mov Cap-1 Maneuver	84	90	-	96	91	333	230	230	_	_	301	301	_	_	
Stage 1	108	190	_	114	212							_			
Stage 2	287	210	_	342	181	-	-	-	-	-	-	-	-	-	
Staye 2	201	210	-	342	101	-	-	-	-	-	-	-	-	-	
Approach	EB			WB			NB				SB				
HCM Control Delay, s	73.2			20.7			0.3				0.6				
HCM LOS	73.Z			20.7 C			0.5				0.0				
I IOIVI LOS	Г			U											
Minor Lane/Major Mvm	t	NBL	NBT	NBR	EBLn1	EBLn2V	VBLn1V	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)		290	_	_	84	358	94	353	301	-	-				
HCM Lane V/C Ratio		0.075	_	_			0.055		0.14	-	_				
HCM Control Delay (s)		18.4	_		104.1	15.9	45.5	19.5	18.9	-	_				
HCM Lane LOS		С	-	_	F	C	E	С	С	_	_				
HCM 95th %tile Q(veh)		0.2	-	-	3	0.3	0.2	1.2	0.5	-	-				
Notes															
	ooit.	¢. Da	lay aya	oodo 3) <u> </u>	r. Com	nutation	Not D	ofined	*. AII	majory	olumo	in plata	on	
: Volume exceeds cap	acity	φ. D6	elay exc	eeus 3	JUS	+: Com	pulation	I NOLD	ennea	. All	major V	olulile	in plato	UII	

Intersection															
Int Delay, s/veh	43														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations		ની	7		र्स	7		Ä	ተ ተጉ			Ä	ተተጐ		
Traffic Vol. veh/h	74	5	30	1	2	102	1	16	1218	32	9	142	1603	157	
Future Vol, veh/h	74	5	30	1	2	102	1	16	1218	32	9	142	1603	157	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	-	None	_	-	_	None	
Storage Length	_	_	105	_	-	115	-	350	_	-	-	310	_	-	
Veh in Median Storage	.# -	1	-	_	1	-	_	-	0	_	_	-	0	_	
Grade, %	-	0	_	_	0	_	_	_	0	_	_	_	0	_	
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94	94	94	
Heavy Vehicles, %	2	2	3	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	79	5	32	1	2	109	1	17	1296	34	10	151	1705	167	
WWW.CT IOW	13	J	UZ			103		17	1230	UT	10	101	1700	107	
Major/Minor I	Minor2		1	Minor1			Major1			N	Major2				
Conflicting Flow All	2666	3477	936	2356	3543	665	1367	1872	0	0	971	1330	0	0	
Stage 1	2111	2111	-	1349	1349	-	-		-	-	-	-	-	-	
Stage 2	555	1366	_	1007	2194	_	_	_	_	_	_	_	_	_	
Critical Hdwy	6.44	6.54	7.16	6.44	6.54	7.14	5.64	5.34	_	_	5.64	5.34	_	_	
Critical Hdwy Stg 1	7.34	5.54	-	7.34	5.54		0.04	0.04	_	_	0.0 -	0.0-	_	_	
Critical Hdwy Stg 2	6.74	5.54	_	6.74	5.54	_	_	_	_	_	_	_	_	_	
Follow-up Hdwy	3.82	4.02	3.93	3.82	4.02	3.92	2.32	3.12	_	_	2.32	3.12	_	_	
Pot Cap-1 Maneuver	~ 24	6	227	38	6	345	274	145	_	_	456	270	_	_	
Stage 1	~ 32	91	-	113	217	070	217	170	_	_	-	210	_	_	
Stage 2	441	213	_	233	82	_						_			
Platoon blocked, %	441	213	_	200	02	_	_	_	_	_	_	_			
Mov Cap-1 Maneuver	~ 7	~ 2	227	12	~ 2	345	148	148	_	_	273	273		_	
•	~ 22	15	- 221	36	14	343	140	140			213	213	-	_	
Mov Cap-2 Maneuver	~ 28	37		99	191	-	-	-	-	-	-			-	
Stage 1	263	187	-	70	34		-	-	-	-	-	-		-	
Stage 2	203	101	-	70	34	-	-	-	-	-	-	-	-	-	
Approach	EB			WB			NB				SB				
HCM Control Delay, \$ 1				26.6			0.4				2.8				
HCM LOS	1257.6 F			20.0 D			0.4				2.0				
I IOIVI LOS	Г			U											
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1	EBLn2V	VBLn1V	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)		148	-	-	21	227	18	345	273	_	-				
HCM Lane V/C Ratio		0.122	-	_		0.141				-	-				
HCM Control Delay (s)		32.6	_		1726.5	23.4	244	20.2	35.5	-	_				
HCM Lane LOS		D	_	_	F	C	F	C	E	_	_				
HCM 95th %tile Q(veh)		0.4	-	-	10.8	0.5	0.5	1.3	3.4	-	-				
Notes															
~: Volume exceeds cap	nacity	\$· De	elay exc	eeds 3	າດຣ	+: Com	nutation	Not De	efined	*· ΔII	maior v	oluma	in plato	nn	
. Volume exceeds cap	Jacity	ψ. De	nay ext	ccus 3	JU3	·. Com	pulation	ו ואטנ טו	Sillieu	. All	major v	Olullie I	iii piato	UII	

Intersection						
Int Delay, s/veh	0					
		MDD	NOT	NDD	051	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	ተ ተጉ			^
Traffic Vol, veh/h	0	1	1363	6	0	
Future Vol, veh/h	0	1	1363	6	0	1285
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	_	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	1	1482	7	0	1397
WWW.CT IOW	U	•	1102	•	U	1001
	/linor1		Major1	١	//ajor2	
Conflicting Flow All	-	745	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	_	7.14	-	-	_	-
Critical Hdwy Stg 1	_	-	_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	3.92	_	_	_	_
Pot Cap-1 Maneuver	0	306	_	_	0	_
Stage 1	0	-	_	_	0	_
	0				0	
Stage 2	U	-	-	-	U	-
Platoon blocked, %		000	-			-
Mov Cap-1 Maneuver	-	306	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Annragah	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	16.8		0		0	
HCM LOS	С					
Minor Lane/Major Mvmt	·	NBT	NRRV	VBLn1	SBT	
		1101	-			
Capacity (veh/h) HCM Lane V/C Ratio		-		0.004	-	
HCM Control Delay (s)		-			-	
DUIVI CONTOL DEIAV (S)		-	_	0.01	-	
3 ()						
HCM Lane LOS HCM 95th %tile Q(veh)		-	-	C 0	-	

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL		^	HOIL	ODL	^
Traffic Vol, veh/h	0	9	1394	3	0	1911
Future Vol, veh/h	0	9	1394	3	0	1911
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	None
	_	0	_			-
Storage Length				-	-	
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	10	1515	3	0	2077
Major/Minor N	1inor1	ı	Major1	١	/lajor2	
		759	0	0		
Conflicting Flow All	-				-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.14	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.92	-	-	-	-
Pot Cap-1 Maneuver	0	300	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	_	300	_	_	_	_
Mov Cap-2 Maneuver	_	-	_	_	_	_
Stage 1	_	_	_	_	_	_
		_	-	-		_
Stage 2	-	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	17.4		0		0	
HCM LOS	C		U		U	
TIOIVI LOO	U					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBT	
Capacity (veh/h)		-	-	300	-	
HCM Lane V/C Ratio		-	-	0.033	-	
HCM Control Delay (s)		-	-	17.4	-	
HCM Lane LOS		_	_	С	-	
HCM 95th %tile Q(veh)		_	_	0.1	_	
1.5m 55m 70m Q(VOII)				J. 1		

Appendix F St Lucie TIP

A.2 PROJECT INDEX AND TIP/RLRTP CROSS REFERENCE

PROJECT NAME	PROJECT LIMITS FROM	PROJECT LIMITS TO	DESCRIPTION	PROJECT LRTP NUMBER Page		TIP MAP Page
A1A BIG MUD CREEK AND BLIND CREEK BRIDGES	BIG MUD CREEK BRIDGE	BLIND CREEK BRIDGE	BRIDGE REPLACEMENT	4491791 3-9	C 6-2	A-5
A1A NORTH CAUSEWAY BRIDGE	ENTIRE BRIDGE	ENTIRE BRIDGE	BRIDGE REPLACEMENT	4299362 8-3	C 6-3	A-4
A1A SUNTRAIL	FT PIERCE INLET STATE PARK	SLC/INDIAN RIVER COUNTY LINE	BIKE PATH/TRAIL	4435061 8-2	C 1-2	A-4
BELL AVENUE	SOUTH 25TH ST	SUNRISE BLVD	BIKE LANE/SIDEWALK	4460761 8-2	C 1-3	A-4
EMERSON AVE	INDRIO RD	25TH ST	RESURFACING	4476511 3-9	C 1-4	A-4
FEC OVERPASS	SAVANNAS RECREATION AREA	SOUTH OF SAVANNAH RD.	BIKE PATH/TRAIL	4400321 8-2	C 1-5	A-4
GATLIN BLVD	WEST OF I-95	PORT ST LUCIE BLVD	TRAFFIC CONTROL DEVICES/SYSTEM	4447071 8-3	C 1-6	A-5
HISTORIC HIGHWAYMAN TRAIL GAP	INDIAN HILLS DR	GEORGIA AVE	BIKE PATH/TRAIL	4400342 8-11	C 1-7	A-4
I-95 @ GATLIN BLVD	OFF-RAMPS	OFF-RAMPS	INTERCHANGE - ADD LANES	4397611 8-3	C 1-8	A-5
I-95 @ ORANGE AVE	NB EXIT RAMP TO WB ORANGE AVE	NB EXIT RAMP TO WB ORANGE AVE	SKID HAZARD OVERLAY	4492811 3-9	C 1-10	A-4
I-95 @ ST. LUCIE WEST BLVD	INTERCHANGE	INTERCHANGE	INTERCHANGE - ADD LANES	4353371 8-2	C 1-12	A-5
I-95 FROM GATLIN BLVD TO ST. LUCIE WEST BLVD	GATLIN BLVD	ST. LUCIE WEST BLVD	SKID HAZARD OVERLAY	4438471 3-9	C 1-13	A-5
I-95 FROM GLADES CUT-OFF RD TO FL TPK	N OF GLADES CUT-OFF RD	N OF FLORIDA TURNPIKE	RESURFACING	4491631 3-9	C 1-14	A-4
I-95 FROM SLC/MARTIN TO SR-70	SLC/MARTIN COUNTY LINE	SR-70/OKEECHOBEE RD	PD&E/EMO STUDY	4226816 8-3	C 1-15	A-4, 5
I-95 ST. LUCIE SOUTHBOUND REST AREA	REST AREA	REST AREA	REST AREA	4499611 3-9	C 1-16	A-4
INTERSECTION LIGHTING RETROFIT IMPROVEMENT	VARIOUS LOCATIONS	VARIOUS LOCATIONS	LIGHTING	4470031 8-3	C 1-17	A-4
JENKINS RD	GLADES CUT OFF RD	ORANGE AVENUE	PD&E/EMO STUDY	4463311 8-3	C 1-18	A-4
KESTOR DR	DARWIN BOULEVARD	BECKER RD	SIDEWALK	4489981 8-11	C 1-19	A-5
KINGS HWY	400 feet S OF OKEECHOBEE RD	NORTH OF PICOS RD	ADD LANES & RECONSTRUCT	2302566 8-2	C 1-20	A-4
KINGS HWY	NORTH OF COMMERCIAL CIR	ST LUCIE BLVD	ADD LANES & RECONSTRUCT	4383792 8-2	C 1-21	A-4
KINGS HWY	N OF I-95 OVERPASS	N OF COMMERCIAL CIR	ADD LANES & RECONSTRUCT	4383791 8-2	C 1-22	A-4
KINGS HWY	NORTH OF PICOS RD	NORTH OF I-95 OVERPASS	ADD LANES & RECONSTRUCT	2302567 8-2	C 1-23	A-4

KINGS HWY	N OF I-95 OVERPASS	SOUTH OF ANGLE	ADD LANES & RECONSTRUCT	4383794 8-2 C 1-26 A-4
MIDWAY RD	GLADES CUT OFF RD	SELVITZ ROAD	ADD LANES & RECONSTRUCT	2314403 8-2 C 1-27 A-4, 5
MIDWAY RD	JENKINS RD	SELVITZ RD	ADD LANES & RECONSTRUCT	2314405 8-11 C 1-28 A-4, 5
OKEECHOBEE RD	IDEAL HOLDING RD	ROCK RD	RESURFACING	4476531 3-9 C 1-29 A-4
OLEANDER AVE	SOUTH MARKET AVE	EDWARDS RD	SIDEWALK	4480661 8-11 C 1-30 A-4
ORANGE AVE	KINGS HWY	E OF I-95 SB RAMP	INTERCHANGE - ADD LANES	4461681 8-3 C 1-31 A-4
ORANGE AVE	KINGS HWY	US-1	ATMS - ARTERIAL TRAFFIC MGMT	4496961 8-11 C 1-32 A-4
ORANGE AVE	NORTH 32ND ST	US-1	RESURFACING	4461691 3-9 C 1-33 A-4
OUTFALL FOR VIRGINIA AVE	OLEANDER BLVD	INDIAN HILLS DR	DRAINAGE IMPROVEMENTS	4417151 3-9 C 1-34 A-4
PORT ST. LUCIE BLVD	BECKER RD	PAAR DRIVE	ADD LANES & RECONSTRUCT	4317523 8-2 C 1-35 A-5
PORT ST. LUCIE BLVD	PAAR DRIVE	DARWIN BLVD	ADD LANES & RECONSTRUCT	4317522 8-2 C 1-36 A-5
PORT ST. LUCIE BLVD	SOUTH OF PAAR DR	SOUTH OF ALCANTARRA BLVD	ADD LANES & RECONSTRUCT	4317525 8-2 C 1-37 A-5
PORT ST. LUCIE BLVD	SHELTER DR	US-1	RESURFACING	4463761 3-9 C 1-38 A-5
PRIMA VISTA BLVD TSM&O	AIROSO BLVD	NARANJA AVE	ITS COMMUNICATION SYSTEM	4481341 8-11 C 1-39 A-5
S 25TH ST	N OF EDWARDS RD	N OF VIRGINIA AVE	RESURFACING	4461701 3-9 C 1-40 A-4
SAVANNAS PRESERVE STATE PARK GAP	(LENNARD RD	SAVANNAS RECREATION AREA	BIKE PATH/TRAIL	4399993 8-3 C 1-41 A-4, 5
SAVANNAS PRESERVE STATE PARK GAP	WALTON RD	LENNARD RD	BIKE PATH/TRAIL	4399992 8-2 C 1-42 A-5
SELVITZ RD	NW FLORESTA DRIVE	NW BAYSHORE BLVD	BIKE LANE/SIDEWALK	4460741 8-2 C 1-43 A-5
ST. LUCIE BLVD	EAST OF N 25 ST	WEST OF US-1	RESURFACING	4484491 3-9 C 1-44 A-4
ST. LUCIE COUNTY PORT OF FT. PIERCE	PORT OF FT. PIERCE	PORT OF FT. PIERCE	SEAPORT REVENUE/OPERAT PROJECT	4150862 3-9 C 8-2 A-4
TURNPIKE RESURFACING	MP 169.3	MP 173	RESURFACING	4444021 3-9 C 7-5 A-4
US HIGHWAY 1	EDWARDS RD	TENNESSEE AVE	DRAINAGE IMPROVEMENTS	4417141 3-9 C 1-45 A-4
US HIGHWAY 1	MARTIN/ST. LUCIE COUNTY LINE	PORT ST. LUCIE BLVD	RESURFACING	4476521 3-9 C 1-46 A-5
US HIGHWAY 1	NORTH OF VIRGINIA AVE	SUNNY LANE	RESURFACING	4461091 3-9 C 1-47 A-4
US HIGHWAY 1	SOUTH OF JUANITA AVE	NORTH OF KINGS HWY	RESURFACING	4484501 3-9 C 1-48 A-4
WALTON RD	800 FEET EAST OF LENNARD RD	GREEN RIVER PKWY	SIDEWALK	4483081 8-11 C 1-49 A-5

GATLIN BLVD FROM WEST OF I-95 TO PORT ST. LUCIE BLVD 4447071 Non-SIS



Prior Year Cost: 5,000 Future Year Cost: 0

Total Project Cost: 633,000

LRTP: Page 8-3

Project Description: TRAFFIC CONTROL DEVICES/SYSTEM

Extra Description: 2021 TPO CMP PRIORITY #1 AND #2 LAP WITH PORT ST LUCIE INSTALL TRAFFIC CAMERAS AT SIGNALIZED INTERSECTIONS; OPTIMIZE GREEN TIME, ADD ADAPTIVE TRAFFIC SIGNAL

CONTROL

Lead Agency: MANAGED BY FDOTFrom: WEST OF I-95County: ST. LUCIETo: PORT ST. LUCIE BLVD

Length: 2.672

Phase Group: PRELIMINARY ENGINEERING, CONSTRUCTION

Phase	Fund Code	2023	2024	2025	2026	2027	Total
CST	GFSU	314,000	0	0	0	0	314,000
CST	SU	314,000	0	0	0	0	314,000
		628,000					628,000

PORT ST. LUCIE BLVD FROM PAAR DR TO DARWIN BLVD 4317522 Non-SIS



Prior Year Cost: 6,521,844

Future Year Cost: 0

Total Project Cost: 24,028,578

LRTP: Page 8-2

Project Description: ADD LANES & RECONSTRUCT

Extra Description: 2020 TPO PRIORITY #2 WIDENING FROM 2 TO 4 LANES CONSTRUCTION SPLIT OUT TO SEG 5 AND 6 PH43 INCLUDES \$121 TO COVER RECORDING FEES LFA WITH CITY OF PORT

ST. LUCIE

Lead Agency: MANAGED BY FDOTFrom: PAAR DRCounty: ST. LUCIETo: DARWIN BLVD

Length: 1.946

Phase Group: PRELIMINARY ENGINEERING, RIGHT OF WAY, ENVIRONMENTAL

Phase	Fund Code	2023	2024	2025	2026	2027	Total
PE	LF	131,977	0	0	0	0	131,977
		131,977					131,977

Appendix G
ITE Trip Generation Information

Convenience Store/Gas Station - GFA (2-4k)

(945)

Vehicle Trip Ends vs: Vehicle Fueling Positions

On a: Weekday

Setting/Location: General Urban/Suburban

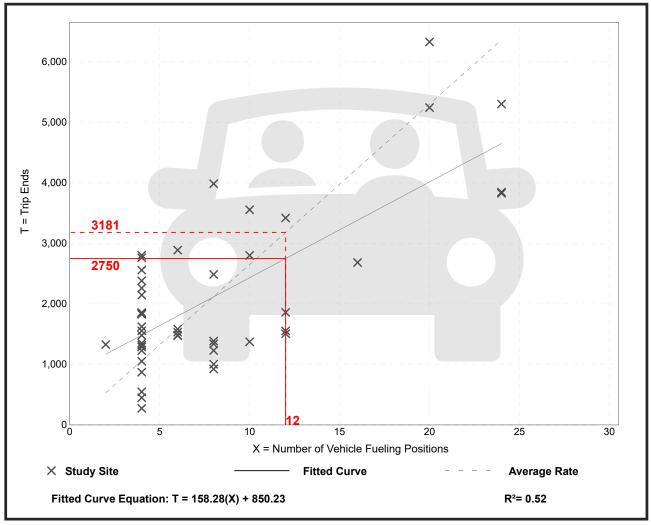
Number of Studies: 48 Avg. Num. of Vehicle Fueling Positions: 8

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
265.12	68.50 - 701.00	142.37

Data Plot and Equation



Convenience Store/Gas Station - GFA (2-4k)

(945)

Vehicle Trip Ends vs: Vehicle Fueling Positions

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

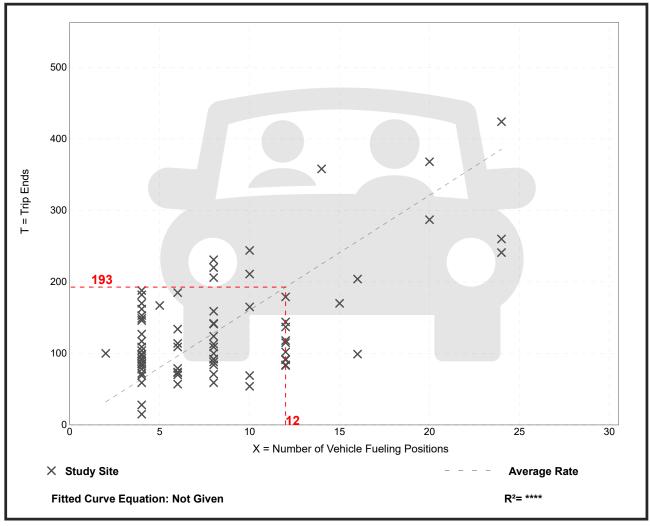
Number of Studies: 76

Avg. Num. of Vehicle Fueling Positions: 8

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
16.06	3.75 - 50.00	8.79



Convenience Store/Gas Station - GFA (2-4k)

(945)

Vehicle Trip Ends vs: Vehicle Fueling Positions

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

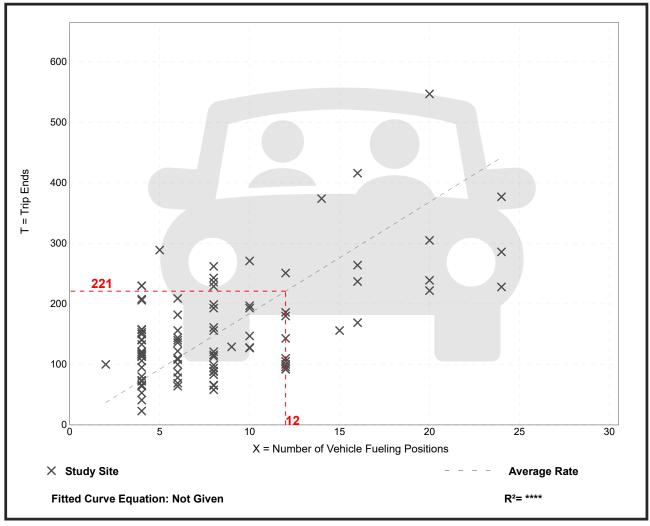
Setting/Location: General Urban/Suburban

Number of Studies: 93 Avg. Num. of Vehicle Fueling Positions: 8

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
18.42	5.75 - 57.80	10.16



				e Pass-By Ra							
		So	urce: ITE	Trip Generatio	n Manual , 1	L1th Edition					
Land Use Code		945									
Land Use		Convenience Store/Gas Station									
Setting						in/Suburban					
Time Period					/eekday AM	Peak Period			100115		
# Data Sites Average Pass-By Rate	,	16 Sites with bet 50% for Sites with b				7	28 Sites with b				
Average Pass-by Rate		50% for Sites with t	etween z		haracteristic	s for Individual		n between:	9 and 20 VFP		
				1 400 27 0			0.000				
			Survey		Pass-By	No	n-Pass-By Trips		Adj Street Peak		
GFA (000)	VFP	State or Province	Year	# Interviews	Trip (%)	Primary (%)	Diverted (%)	Total (%)	Hour Volume	Source	
2	8	Maryland	1992	46	87	13	0	13	2235	25	
2.1	6	Maryland	1992	26	58	23	19	42	2080	25	
2.1	6 8	Maryland	1992 1992	26 31	58 47	23 34	19 19	42 53	2080 1785	25 25	
2.2	< 8	Maryland Indiana	1992	79	56	6	38	44	635	25	
2.2	8	Maryland	1993	35	78	9	13	22	7080	25	
2.3	6	Maryland	1992	37	32	41	27	68	2080	25	
2.3	< 8	Kentucky	1993	58	64	5	31	36	1255	2	
2.3	6	Maryland	1992	37	32	41	27	68	2080	25	
2.4	< 8	Kentucky	1993	_	48	17	35	52	1210	2	
2.6	< 8	Kentucky	1993	_	72	15	13	28	940	2	
2.8	< 8	Kentucky	1993	_	54	11	35	46	1240	2	
3	< 8	Indiana	1993	62	74	10	16	26	790	2	
3.6	< 8	Kentucky	1993	49	67	4	29	33	1985	2	
3.7	< 8	Kentucky	1993	49	66	16	18	34	990	2	
4.694	12	Maryland	2000	_	72	_	-	28	2440	30	
4.694 4.694	12 12	Maryland Maryland	2000	_	78 79	_		22 21	1561 2764	30 30	
4.848	12	Virginia	2000	_	55	_		45	1398	30	
5.06	12	Pennsylvania	2000	_	84	_	_	16	3219	30	
5.242	12	Virginia	2000	_	74	_	_	26	1160	30	
5.242	12	Virginia	2000	_	71	_	1	29	548	30	
5.488	12	Delaware	2000	_	80	_	_	20	_	30	
5.5	12	Pennsylvania	2000	_	85	_	_	15	2975	30	
4.2	< 8	Kentucky	1993	47	62	19	19	38	1705	2	
4.694	16	Maryland	2000	_	90	_	_	10	2278	30	
4.694	16	Delaware	2000	_	74	_	_	26	2185	30	
4.694 4.694	16 16	Delaware Delaware	2000 2000	_	58 84	_		42 16	962 2956	30 30	
4.694	16	New Jersey	2000	_	79	_		21	1859	30	
4.694	20	Delaware	2000	_	84	_		16	3864	30	
4.848	16	Virginia	2000	_	68	_	_	32	2106	30	
4.848	16	Virginia	2000	_	85	_	_	15	2676	30	
4.848	16	Virginia	2000	_	75	_	_	25	3244	30	
4.848	16	Virginia	2000	_	71	_	_	29	1663	30	
4.993	16	Pennsylvania	2000	_	75	_		25	1991	30	
5.094	16	New Jersey	2000	_	86	_	_	14	1260	30	
5.5	16	Pennsylvania	2000	_	82	_		18	1570	30	
5.543	16 16	Pennsylvania	2000	_	84	_	_	16	1933	30	
5.565 5.565	16 16	Pennsylvania Pennsylvania	2000	_	77 68	_		23 32	2262 2854	30 30	
5.565	16	New Jersey	2000	_	58	_		42	1253	30	
5.565	16	New Jersey	2000	_	79	_		21	1928	30	
5.565	16	New Jersey	2000		84			16	1953	30	

			Vehicl	e Pass-By Ra	tes by Land	d Use					
		So		Trip Generatio							
Land Usa Cada					0.4						
Land Use Code		945									
Land Use		Convenience Store/Gas Station General Urban/Suburban									
Setting						-					
Time Period		40.6%			еекаау РМ	Peak Period	20.6%		1201/50		
# Data Sites		12 Sites with bety				_	28 Sites with b				
Average Pass-By Rate		56% for Sites with b	etween 2				5% for Sites wit	h between s	9 and 20 VFP		
		<u> </u>		Pass-By C	haracteristic	s for Individual	Sites				
		1	C		D D	l N-	- D DT-i		Adi Charat Baal	1	
274 (222)			Survey		Pass-By		n-Pass-By Trips		Adj Street Peak	<u> </u>	
GFA (000)	VFP	State or Province	Year	# Interviews	Trip (%)	Primary (%)	Diverted (%)	Total (%)	Hour Volume	Sour	
2.1	8	Maryland	1992	31	52	13	35	48	1785	25	
2.1	6	Maryland	1992	30	53	20	27	47	1060	25	
2.2	< 8	Indiana	1993	115	48	16	36	52	820	2	
2.3	< 8	Kentucky	1993	67	57	16	27	43	1954	2	
2.3	6	Maryland	1992	55	40	11	49	60	2760	25	
2.4	< 8	Kentucky	1993	_	58	13	29	42	2655	2	
2.6	< 8	Kentucky	1993	68	67	15	18	33	950	2	
2.8	< 8	Kentucky	1993	_	62	11	27	38	2875	2	
3	< 8	Indiana	1993	80	65	15	20	35	1165	2	
3.6	< 8	Kentucky	1993	60	56	17	27	44	2505	2	
3.7	< 8	Kentucky	1993	70	61	16	23	39	2175	2	
4.2	< 8	Kentucky	1993	61	58	26	16	42	2300	2	
4.694	12	Maryland	2000	_	78	_	_	22	3549	30	
4.694	12	Maryland	2000	_	67	_	_	33	2272	30	
4.694	12	Maryland	2000	_	66	_	_	34	3514	30	
4.848	12	Virginia	2000	_	71	_	_	29	2350	30	
5.06	12	Pennsylvania	2000	_	91	_	_	9	4181	30	
5.242	12	Virginia	2000	_	70			30	2445	30	
5.242	12	Virginia	2000		56	_		44	950	30	
5.488	12	Delaware	2000	_	73	_		27	950	30	
				_			_			_	
5.5	12	Pennsylvania	2000	_	84	_	_	16	4025	30	
4.694	16	Maryland	2000	_	89	_	_	11	2755	30	
4.694	16	Delaware	2000	_	73	_	_	27	1858	30	
4.694	16	Delaware	2000	_	59	_	_	41	1344	30	
4.694	16	Delaware	2000	_	72	_	_	28	3434	30	
4.694	16	New Jersey	2000	_	81	_		19	1734	30	
4.694	20	Delaware	2000	_	76	_		24	1616	30	
4.848	16	Virginia	2000	_	67	_	_	33	2.954	30	
4.848	16	Virginia	2000	_	78	_	_	22	3086	30	
4.848	16	Virginia	2000	_	83	_		17	4143	30	
4.848	16	Virginia	2000	_	73	_	_	27	2534	30	
4.993	16	Pennsylvania	2000	_	72	_	_	28	2917	3	
5.094	16	New Jersey	2000	_	86	_		14	1730	30	
5.5	16	Pennsylvania	2000	_	90	_	_	10	2616	30	
5.543	16	Pennsylvania	2000	_	87	_	_	13	2363	30	
5.565	16	Pennsylvania	2000	_	81	_	_	19	2770	30	
5.565	16	Pennsylvania	2000	_	76	_	_	24	3362	30	
5.565	16	New Jersey	2000	_	61	_	_	39	1713	30	
5.565	16	New Jersey	2000	_	86	_	_	14	1721	30	
5.565	16	New Jersey	2000		81			19	2227	30	
		1121130100,		1		 					

General Office Building

(710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday

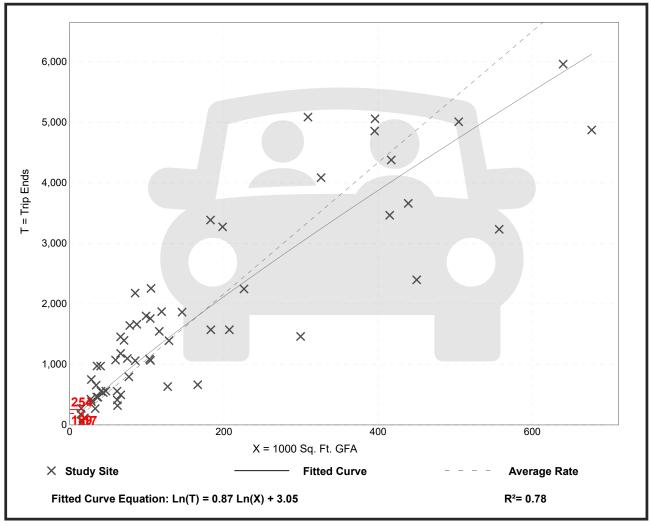
Setting/Location: General Urban/Suburban

Number of Studies: 59 Avg. 1000 Sq. Ft. GFA: 163

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
10.84	3.27 - 27.56	4.76



General Office Building

(710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

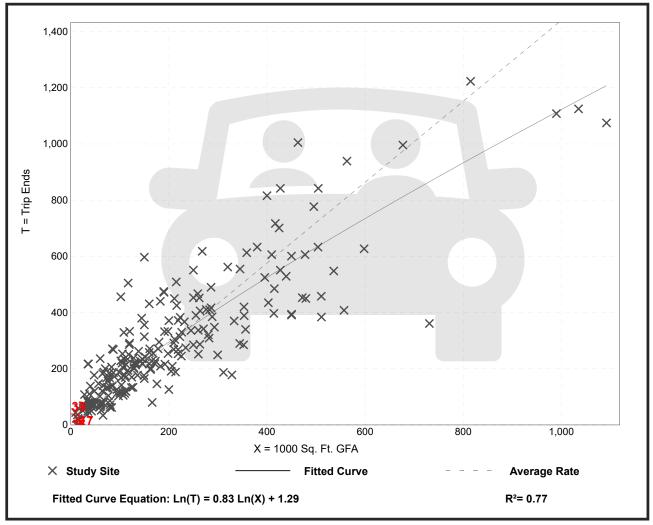
Setting/Location: General Urban/Suburban

Number of Studies: 232 Avg. 1000 Sq. Ft. GFA: 199

Directional Distribution: 17% entering, 83% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.44	0.26 - 6.20	0.60



Strip Retail Plaza (<40k)

(822)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies: 4 Avg. 1000 Sq. Ft. GLA: 19

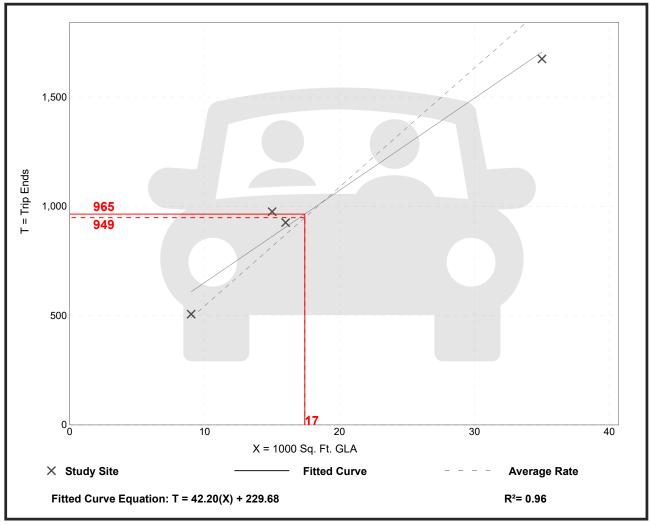
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation	
54.45	47.86 - 65.07	7.81	

Data Plot and Equation

Caution - Small Sample Size



Strip Retail Plaza (<40k)

(822)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

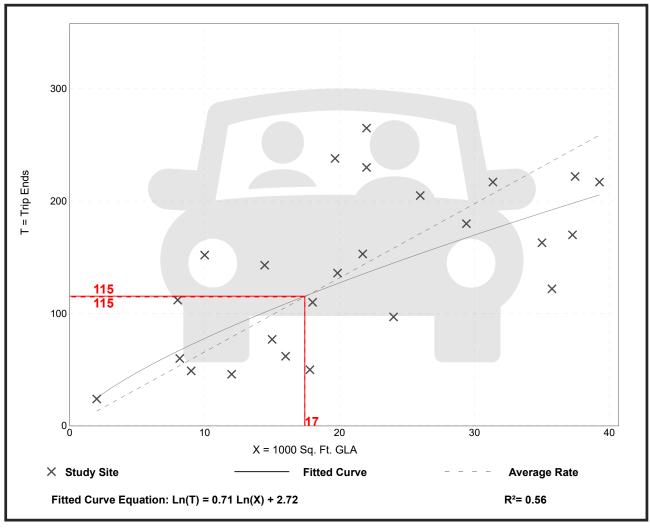
Setting/Location: General Urban/Suburban

Number of Studies: 25 Avg. 1000 Sq. Ft. GLA: 21

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
6.59	2.81 - 15.20	2.94



Appendix H
Historical Counts and Growth Rate Calculation

COUNTY: 94 - ST.LUCIE

SITE: 8555 - SW CAMEO BLVD, N OF SW MONTANA TERRACE.

YEAR	AADT	DIE	RECTION 1	DII	RECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	7700 C	N	4200	S	3500	9.00	50.90	13.20
2020	7300 S	N	3800	S	3500	9.00	51.30	5.30
2019	7700 F	N	4000	S	3700	9.00	51.00	5.30
2018	7700 C	N	4000	S	3700	9.00	51.30	5.30
2017	8300 S	N	4400	S	3900	9.00	50.90	4.10
2016	8100 F	N	4300	S	3800	9.00	50.90	4.10
2015	8100 C	N	4300	S	3800	9.00	51.00	4.10

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

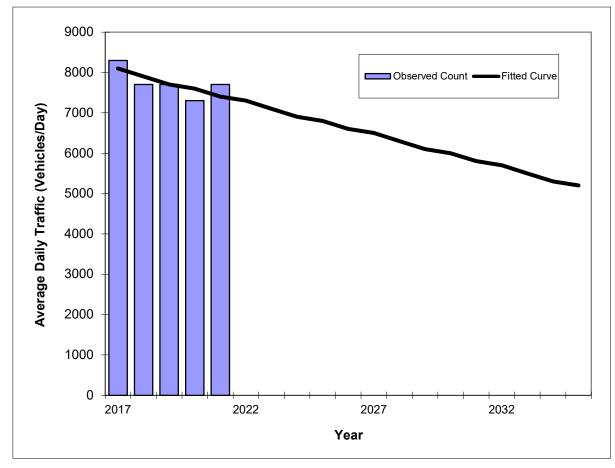
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a SW CAMEO BLVD -- N OF SW MONTANA TERRACE

FIN#	1234
Location	1

County:	St. Lucie (94)
Station #:	8555
Highway:	SW CAMEO BLVD



** Annual Trend Increase:	-160
Trend R-squared:	50.00%
Trend Annual Historic Growth Rate:	-2.16%
Trend Growth Rate (2021 to Design Year):	-2.10%
Printed:	23-Mar-23
Straight Line Growth Option	

	Traffic (ADT/AADT)			
Year	Count*	Trend**		
2017	8300	8100		
2018	7700	7900		
2019	7700	7700		
2020	7300	7600		
2021	7700	7400		
202	3 Opening Yea	r Trend		
2023	N/A	7100		
	025 Mid-Year T			
2025	N/A	6800		
2030	30 Design Year N/A			
	PLAN Forecas	6000		
IIVAIN	LAN Forecas	ts/Helius		

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 7062 - ON DARWIN BLVD - N OF BECKER RD (COUNTY 235)

YEAR	AADT	DII	RECTION 1	DII	RECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	7400 T	N	3700	S	3700	9.00	50.90	7.20
2020	7400 S	N	3700	S	3700	9.00	51.30	31.50
2019	7800 F	N	3900	S	3900	9.00	51.00	7.80
2018	7800 C	N	3900	S	3900	9.00	51.30	5.80
2017	5000 V	N	2400	S	2600	9.00	50.90	10.00
2016	5000 R	N	2400	S	2600	9.00	50.90	6.20
2015	5000 T	N	2400	S	2600	9.00	51.00	41.80
2014	5000 S	N	2400	S	2600	9.00	50.80	49.50
2013	5000 F	N	2400	S	2600	9.00	50.80	11.90
2012	5000 C	N	2400	S	2600	9.00	56.80	7.10
2011	5000 S	N	2400	S	2600	9.00	57.20	3.60
2010	5000 F	N	2400	S	2600	10.32	55.40	3.60
2009	5000 C	N	2400	S	2600	10.27	57.35	3.60
2008	5000 C	N	2600	S	2400	10.45	58.06	15.70

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

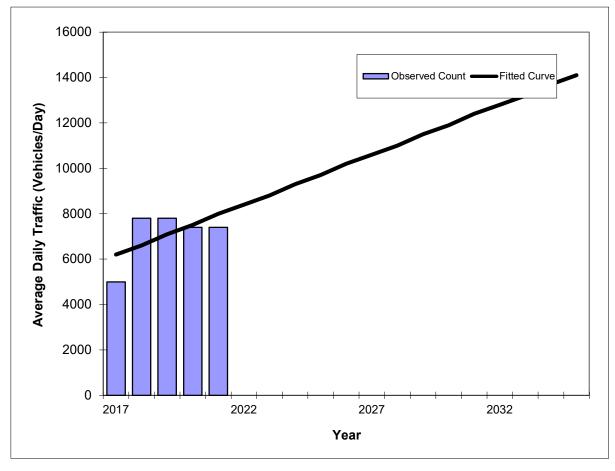
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a DARWIN BLVD -- N OF BECKER RD

FIN# **1234** Location **1**

County:	St. Lucie (94)	
Station #:	7062	
Highway:	DARWIN BLVD	



** Annual Trend Increase:	440
Trend R-squared:	34.77%
Trend Annual Historic Growth Rate:	7.26%
Trend Growth Rate (2021 to Design Year):	5.42%
Printed:	27-Mar-23
Straight Line Growth Option	

	Traffic (ADT/AADT)			
Year	Count*	Trend**		
2017	5000	6200		
2018	7800	6600		
2019	7800	7100		
2020	7400	7500		
2021	7400	8000		
202	3 Opening Yea	r Trend		
2023	N/A	8800		
20	025 Mid-Year T	rend		
2025	N/A	9700		
	30 Design Year			
2030	N/A	11900		
TRAN	PLAN Forecas	ts/Trends		

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 8534 - DEL RIO BLVD. FROM PORT ST LUCIE BLVD TO NW CALIFORNIA BLVD (COUNTY 311) (HPMS)

YEAR	AADT	DIE	RECTION 1	DII	RECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	10100 C	N	5000	S	5100	9.00	50.90	9.60
2020	10400 S	N	5200	S	5200	9.00	51.30	5.50
2019	10800 F	N	5400	S	5400	9.00	51.00	5.50
2018	10800 C	N	5400	S	5400	9.00	51.30	5.50
2017	9300 S	N	4600	S	4700	9.00	50.90	4.60
2016	9100 F	N	4500	S	4600	9.00	50.90	4.60
2015	9100 C	N	4500	S	4600	9.00	52.70	4.60
2014	10700 F	N	5600	S	5100	9.00	52.50	4.50
2013	10700 C	N	5600	S	5100	9.00	55.90	4.50
2012	10800 C	N	5200	S	5600	9.00	55.80	4.50
2011	10700 C	N	5100	S	5600	9.00	56.20	3.80
2010	11700 C	N	5900	S	5800	11.16	56.34	11.40
2009	11000 C	N	5200	S	5800	11.51	56.49	4.70
2008	11400 C	N	5500	S	5900	11.31	55.19	6.70

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

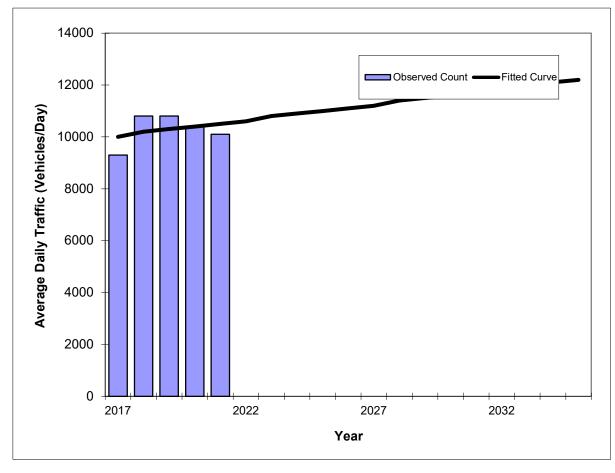
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a

DEL RIO BLVD. -- from Port St Lucie Blvd to NW California Blvd

FIN#	1234	
Location	1	

County:	St. Lucie (94)
Station #:	8534
Highway:	DEL RIO BLVD.



** Annual Trend Increase:	120
Trend R-squared:	9.30%
Trend Annual Historic Growth Rate:	1.25%
Trend Growth Rate (2021 to Design Year):	1.16%
Printed:	23-Mar-23
Straight Line Growth Option	

	Traffic (AD	T/AADT)
Year	Count*	Trend**
2017	9300	10000
2018	10800	10200
2019	10800	10300
2020	10400	10400
2021	10100	10500
000		
2023	3 Opening Yea N/A	10800
	D25 Mid-Year T	
2025	N/A	11000
	B5 Design Year	
2035	N/A	12200
	PLAN Forecas	
	70,100,000,000	

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 5075 - GATLIN BLVD - E OF I-95 IN PORT ST LUCIE (COUNTY 5075)

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	46000 C	E 21000	W 25000	9.00	50.90	4.80
2020	48500 F	E 23000	W 25500	9.00	51.30	4.80
2019	50500 C	E 24000	W 26500	9.00	51.00	4.80
2018	38000 C	E 17500	W 20500	9.00	51.30	4.60
2017	34000 C	E 16000	W 18000	9.00	50.90	4.60
2016	36500 C	E 18000	W 18500	9.00	50.90	4.60
2015	28500 C	E 13000	W 15500	9.00	51.00	11.90
2014	32500 C	E 16500	W 16000	9.00	50.80	11.90
2013	32500 C	E 16000	W 16500	9.00	50.80	11.90
2012	30500 C	E 14500	W 16000	9.00	56.80	4.90
2011	31500 C	E 15500	W 16000	9.00	57.20	4.90
2010	32500 C	E 15500	W 17000	10.32	55.40	4.90
2009	38500 F	E 19000	W 19500	10.27	57.35	5.50
2008	38500 C	E 19000	W 19500	10.45	58.06	5.50
2007	34000 C	E 16500	W 17500	10.31	58.74	9.10
2006	27500 C	E 13500	W 14000	10.73	65.89	10.60

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a **GAITLIN BLVD -- E OF I-95 IN PORT ST LUCIE**

FIN#	1234
Location	1

County:	St. Lucie (94)		
Station #:	5075		
Highway:	GAITLIN BLVD		

Year 2017

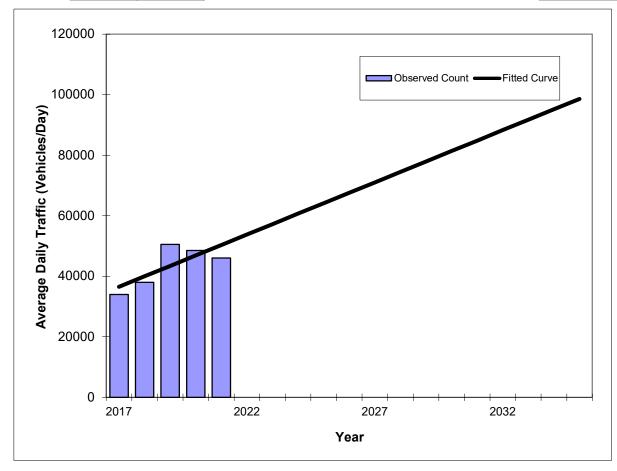
Traffic (ADT/AADT)

Count*

34000

Trend**

36500



2018 2019 2020 2021	38000 50500 48500 46000	40000 43400 46900 50300
	3 Opening Yea	
2023	N/A 025 Mid-Year T	57200
2025	N/A	64100
	30 Design Year	
2030	N/A	81400
TRAN	PLAN Forecas	ts/Trends

3,450 ** Annual Trend Increase: Trend R-squared: 59.30% **Trend Annual Historic Growth Rate:** 9.45% Trend Growth Rate (2021 to Design Year): 6.87% Printed: 27-Mar-23 **Straight Line Growth Option**

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 5074 - SR 716/PT ST LUCIE BLVD - W OF TPK OVERPASS BR (COUNTY 5074)

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	50000 C	E 29000	W 21000	9.00	50.90	6.30
2020	44500 C	E 21500	W 23000	9.00	51.30	6.80
2019	49500 C	E 23500	W 26000	9.00	51.00	4.10
2018	51000 C	E 25000	W 26000	9.00	51.30	4.10
2017	48500 C	E 24000	W 24500	9.00	50.90	4.10
2016	43000 C	E 21500	W 21500	9.00	50.90	4.10
2015	44000 C	E 22000	W 22000	9.00	51.00	4.10
2014	43000 C	E 22000	W 21000	9.00	50.80	4.20
2013	45000 C	E 21500	W 23500	9.00	50.80	4.20
2012	45000 C	E 22500	W 22500	9.00	56.80	4.20
2011	46500 F	E 23000	W 23500	9.00	57.20	3.60
2010	46500 C	E 23000	W 23500	10.32	55.40	3.60
2009	41500 C	E 20500	W 21000	10.27	57.35	5.10
2008	46000 C	E 23500	W 22500	10.45	58.06	5.10
2007	49500 C	E 25500	W 24000	10.31	58.74	6.90
2006	50000 C	E 25000	W 25000	10.73	65.89	10.60

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a SR 716/PT ST LUCIE BLVD -- W OF TPK OVERPASS BR

FIN#	1234
Location	1

County:	St. Lucie (94)		
Station #:	5074		
Highway:	SR 716/PT ST LUCIE BLVD		

Year

2017

2018

2019

2020

2021

Traffic (ADT/AADT)

Trend**

49400

49100

48700

48400

48000

Count*

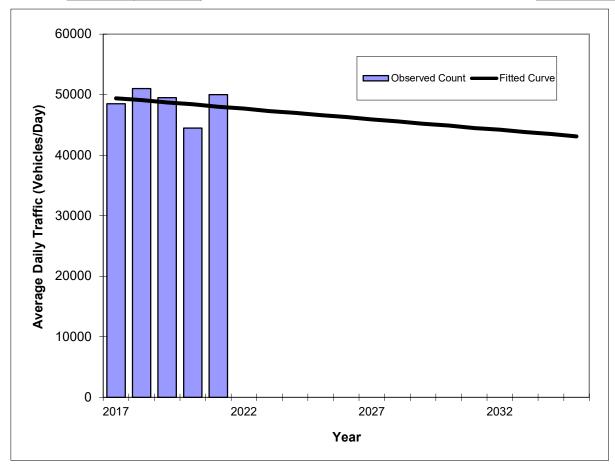
48500

51000

49500

44500

50000



	202	3 Opening Yea	r Trend
	2023	N/A	47300
	20	025 Mid-Year 1	rend
	2025	N/A	46600
	203	0 Design Year	Trend
	2030	N/A	44900
	TRAN	PLAN Forecas	ts/Trends
•		•	•

** Annual Trend Increase: -350 Trend R-squared: 4.84% -0.71% **Trend Annual Historic Growth Rate:** -0.72% Trend Growth Rate (2021 to Design Year): Printed: 23-Mar-23 **Straight Line Growth Option**

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 8518 - SW PORT ST LUCIE BLV FROM BECKER RD TO HAMBERLAND AVE (HPMS)

YEAR	AADT	DIE	RECTION 1	DII	RECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	10000 C	N	5100	S	4900	9.00	50.90	12.80
2020	8200 S	N	4200	S	4000	9.00	51.30	5.40
2019	8600 F	N	4400	S	4200	9.00	51.00	5.40
2018	8600 C	N	4400	S	4200	9.00	51.30	5.40
2017	7800 S	N	4000	S	3800	9.00	50.90	5.10
2016	7600 F	N	3900	S	3700	9.00	50.90	5.10
2015	7600 C	N	3900	S	3700	9.00	51.00	5.10
2014	7500 F	N	3500	S	4000	9.00	50.80	1.80
2013	7500 C	N	3500	S	4000	9.00	50.80	1.80
2012	7600 C	N	3900	S	3700	9.00	56.80	1.80
2011	5100 T		0		0	9.00	57.20	4.60
2010	5100 S	N	2800	S	2300	10.32	55.40	6.70
2009	5100 F	N	2800	S	2300	10.27	57.35	6.70
2008	5100 C	N	2800	S	2300	10.45	58.06	6.70
2007	5600 C	N	2700	S	2900	10.31	58.74	5.70

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

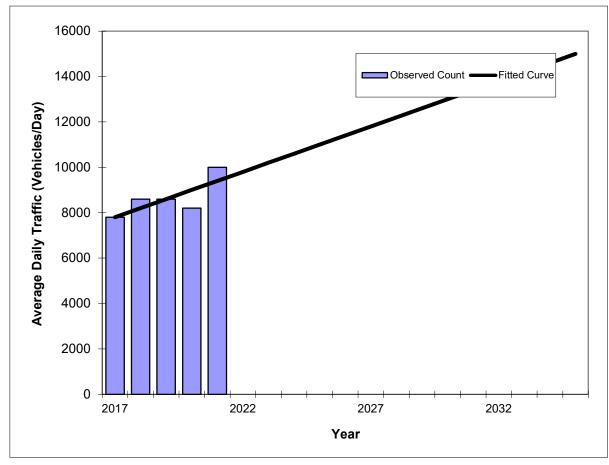
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a SW PORT ST LUCIE -- FROM BECKER RD TO HAMBERLAND AVE

FIN#	1234
Location	1

County:	St. Lucie (94)
Station #:	8518
Highway:	SW PORT ST LUCIE



** Annual Trend Increase:	400
Trend R-squared:	58.14%
Trend Annual Historic Growth Rate:	5.13%
Trend Growth Rate (2021 to Design Year):	4.26%
Printed:	27-Mar-23
Straight Line Growth Option	

	Traffic (ADT/AADT)					
Year	Count*	Trend**				
2017	7800	7800				
2018	8600	8200				
2019	8600	8600				
2020	8200	9000				
2021	10000	9400				
202	3 Opening Yea	r Trend				
2023	N/A	10200				
	025 Mid-Year T	rend				
2025	N/A	11000				
	30 Design Year					
2030	N/A	13000				
TRAN	PLAN Forecas	ts/Trends				

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 8519 - SW PORT ST LUCIE BLV FROM SW ALCANTARRA BLVD TO TULIP BLVD (HPMS)

YEAR	AADT	DII	RECTION 1	DII	RECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	17500 C	N	8800	S	8700	9.00	50.90	9.70
2020	14600 S	N	7100	S	7500	9.00	51.30	7.60
2019	15200 F	N	7400	S	7800	9.00	51.00	7.60
2018	15300 C	N	7400	S	7900	9.00	51.30	7.60
2017	16000 S	N	8200	S	7800	9.00	50.90	4.70
2016	15800 F	N	8100	S	7700	9.00	50.90	4.70
2015	15600 C	N	8000	S	7600	9.00	51.00	4.70
2014	13100 F	N	6600	S	6500	9.00	50.80	2.10
2013	13100 C	N	6600	S	6500	9.00	50.80	2.10
2012	16900 C	N	8300	S	8600	9.00	56.80	2.10
2011	14500 T		0		0	9.00	57.20	4.60
2010	14500 S	N	7200	S	7300	10.32	55.40	11.70
2009	14500 F	N	7200	S	7300	10.27	57.35	11.70
2008	14700 C	N	7300	S	7400	10.45	58.06	11.70
2007	15600 C	N	7600	S	8000	10.31	58.74	4.20

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

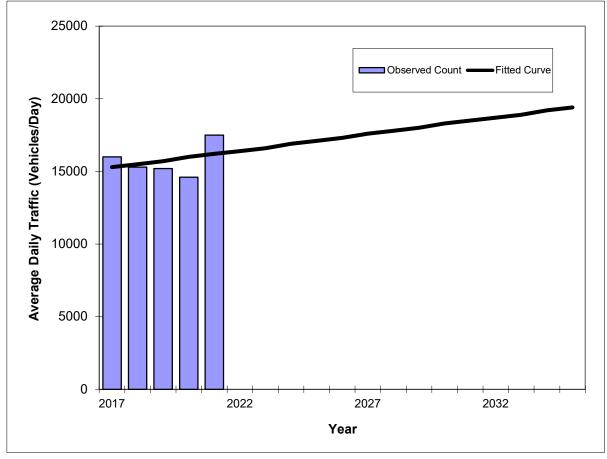
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a SW Port St Lucie Blv -- From SW Alcantarra Blvd to Tulip Blvd

FIN#	1234
Location	1 1

County:	St. Lucie (94)
Station #:	8519
Highway:	SW Port St Lucie Blv



** Annual Trend Increase:	230
Trend R-squared:	10.69%
Trend Annual Historic Growth Rate:	1.47%
Trend Growth Rate (2021 to Design Year):	1.44%
Printed:	23-Mar-23
Straight Line Growth Option	

	Traffic (AD	T/AADT)
Year	Count*	Trend**
2017	16000	15300
2018	15300	15500
2019	15200	15700
2020	14600	16000
2021	17500	16200
202	3 Opening Yea	r Trend
2023	N/A	16600
	025 Mid-Year T	
2025	N/A	17100
	0 Design Year	
2030	N/A	18300
TRAN	PLAN Forecas	is/Trenas

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 7063 - SAVONA BLVD - N OF BECKER RD (COUNTY 236)

YEAR	AADT	DII	RECTION 1	DII	RECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	7800 T	N	3700	S	4100	9.00	50.90	7.20
2021	8000 S	N	3800	S	4200	9.00	51.30	31.50
2019	8400 F	N	4000	S	4400	9.00	51.00	7.80
2018	8400 C	N	4000	S	4400	9.00	51.30	5.80
2017	5300 V	N	2600	S	2700	9.00	50.90	10.00
2016	5300 R	N	2600	S	2700	9.00	50.90	6.20
2015	5300 T	N	2600	S	2700	9.00	51.00	41.80
2014	5300 S	N	2600	S	2700	9.00	50.80	49.50
2013	5300 F	N	2600	S	2700	9.00	50.80	11.90
2012	5300 C	N	2600	S	2700	9.00	56.80	7.10
2011	4700 T		0		0	9.00	57.20	7.60
2010	4700 S	N	1800	S	2900	10.32	55.40	2.80
2009	4700 F	N	1800	S	2900	10.27	57.35	2.80
2008	4700 C	N	1800	S	2900	10.45	58.06	2.80

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

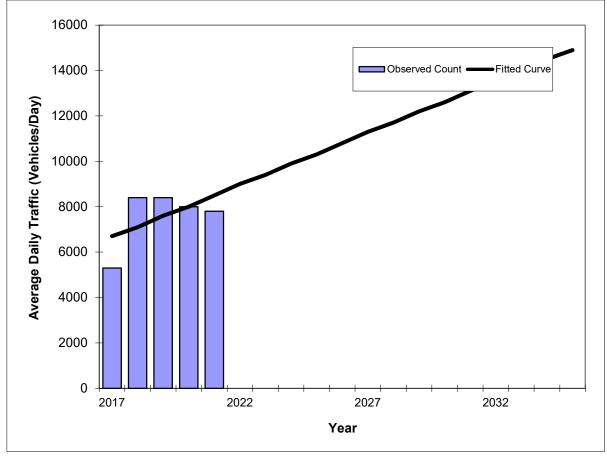
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a SAVONA BLVD. -- N OF BECKER RD

FIN#	1234
Location	1

County:	St. Lucie (94)
Station #:	7063
Highway:	SAVONA BLVD.



** Annual Trend Increase:	460
Trend R-squared:	31.26%
Trend Annual Historic Growth Rate:	6.72%
Trend Growth Rate (2021 to Design Year):	5.36%
Printed:	27-Mar-23
Straight Line Growth Option	

	Traffic (AD	T/AADT)
Year	Count*	Trend**
2017	5300	6700
2018	8400	7100
2019	8400	7600
2020	8000	8000
2021	7800	8500
202	3 Opening Yea	r Trend
2023	N/A	9400
	025 Mid-Year T	
2025	N/A	10300
	30 Design Year	
2030	N/A	12600
TRAN	PLAN Forecas	ts/Trends

*Axle-Adjusted

COUNTY: 94 - ST.LUCIE

SITE: 0080 - SW TULIP BLVD- SOUTH OF SW CHERRY HILL RD

YEAR	AADT	DIE	RECTION 1	DII	RECTION 2	*K FACTOR	D FACTOR	T FACTOR
2021	6400 T	N	3300	S	3100	9.00	50.90	7.20
2020	6400 S	N	3300	S	3100	9.00	51.30	31.50
2019	6700 F	N	3500	S	3200	9.00	51.00	7.80
2018	6700 C	N	3500	S	3200	9.00	51.30	5.80
2017	3900 V		0		0	9.00	50.90	10.00
2016	3800 R		0		0	9.00	50.90	6.20
2015	3800 T		0		0	9.00	51.00	41.80
2014	3800 S					9.00	50.80	49.50
2013	3800 F		0		0	9.00	50.80	11.90
2012	3800 C	E	0	W	0	9.00	56.80	7.10

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE

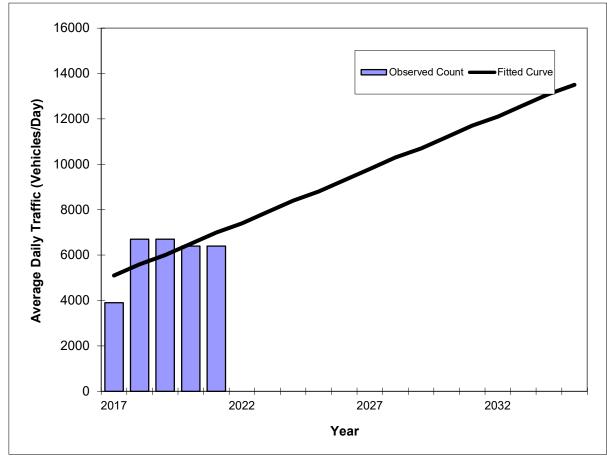
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE

V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN

Traffic Trends - V03.a SW TULIP BLVD -- SOUTH OF SW CHERRY HILL RD

FIN#	1234
Location	1

County:	St. Lucie (94)
Station #:	0080
Highway:	SW TULIP BLVD



** Annual Trend Increase:	470
Trend R-squared:	38.70%
Trend Annual Historic Growth Rate:	9.31%
Trend Growth Rate (2021 to Design Year):	6.67%
Printed:	23-Mar-23
Straight Line Growth Option	

	Traffic (AD	T/AADT)
Year	Count*	Trend**
2017	3900	5100
2018	6700	5600
2019	6700	6000
2020 2021	6400 6400	6500 7000
2021	6400	7000
	3 Opening Yea	
2023	N/A	7900
2025	025 Mid-Year T N/A	rend 8800
	30 Design Year	
2030	N/A	11200
	PLAN Forecas	ts/Trends

*Axle-Adjusted

Appendix I
Intersection Volume Worksheets

Current Year 2022

Intersection Volumes

Period	.i															
	מ			Tgen	Enter	Exit		Pass By	Enter	Exit	SF	Years			Legend	
AM Pe	ak				26	25		Trips	38	39	1.00	2			Existing + [Pass-	by] + (Project) = Total
Inters			CW	Dowt Ct I	wolo	Dhad 0			Duite							1
Approac			SF	Port St Adjusted	GR		Vested		% PB Ext		9/ Duni Funt	%Proj Ext	Dunings	Total		Formula
Approac	n wvmt	Raw 0		Adjusted 0	1.00	Aaj Bgʻa O	vested 0	% PB Ent	% PB EXT	Passby 0	%Proj Ent	%Proj Ext	Project 0	ı otai 0		Formula
EB	T	0	1.00	0	1.00	0	0			0			0	0		
LD	R	0	1.00	0	1.00	0	0			0			0	0		
	L	0	1.00	0	1.00	0	0			0			0	0		
WB	T	0	1.00	0	1.00	0	0			0			0	0		
	R.	1	1.00	1	1.00	1	0		45%	21		30%	7	-	1 + [21] + (7) = 29	
	L	0		0	1.00	0	0		1070	0		0070	0	0	1 - [2 - 1] - (1) 20	
NB	T	1363	1.00	1.363	1.00	1344	0			0	15%		4	1348	1344 + (4) = 1348	
	R	6	1.00	6	1.00	6	0	45%		19	20%		5		6 + [19] + (5) = 30	
	L	0	1.00	0	1.00	0	0			0			0	0	1 1 (1)	
SB	Т	1285	1.00	1,285	1.00	1285	0			0	45%		13	1298	1285 + (13) = 1298	
	R	0	1.00	0	1.00	0	0			0			0	0	. ,	
Inters	sectio	n=	sw	Aviation	Δνα.	& Full A	rraee l	Drivowa	av.							2
Approac			SF	Adjusted	GR	Adj Bg'd			% PB Ext	Passby	% Proj Ent	%Proj Ext	Project	Total		Formula
Approac		0		Aujusteu	1.00	0 Auj Dy u	0	50%	/0 FD LXL	17	75%	/8FTOJ EXT	20		[17] + (20) = 37	Torridia
EB	T	47	1.00	47	1.00	47	0	0070		0	.0.0		0	47		
	R.	0	1.00	0	1.00	0	0			0			0	0		
	L	0	1.00	0	1.00	0	0			0			0	0		
WB	T	108	1.00	108	1.00	106	0			0			0	106	106	
	R	0	1.00	0	1.00	0	0	5%		2	5%		1		[2] + (1) = 3	
	L	0	1.00	0	1.00	0	0			0			0	0		
NB	Т	0	1.00	0	1.00	0	0			0			0	0		
	R	0	1.00	0	1.00	0	0			0			0	0		
	L	0	1.00	0	1.00	0	0		5%	1		5%	1	2	[1] + (1) = 2	
SB	Т	0	1.00	0	1.00	0	0			0			0	0		
	R	0	1.00	0	1.00	0	0		50%	17		65%	17	34	[17] + (17) = 34	
Inters	sectio	n=	SW	Port St I	Lucie	Blvd &	SW Avi	ation A	ve							3
Approac			SF	Adjusted	GR	Adj Bg'd	Vested		% PB Ext	Passby	%Proi Ent	%Proj Ext	Project	Total		Formula
	L		1.00	52	1.00	52	0			0	.,		0	52	52	•
EB	Т	0		0	1.00	0	0			0	5%		1		(1)	
	R	28	1.00	28	1.00	28	0			0			0	28	28	
	L	3	1.00	3	1.00	3	0			0		45%	11	14	3 + (11) = 14	
WB	T	2	1.00	2	1.00	2	0			0		5%	1	3	2 + (1) = 3	
	R	103	1.00	103	1.00	103	0			0		15%	5	108	103 + (5) = 108	
	U	1	1.00	1	1.00	1	0			0			0	1	1	
	L	20	1.00	20	1.00	20	0			0			0	20		
NB	T	1208	1.00	1,208	1.00	1208	0			0	20%		5		1208 + (5) = 1213	
	R	12	1.00	12	1.00	12	0			0	25%		6		12 + (6) = 18	
	U	6	1.00	6	1.00	6	0			0			0	6		
	L	35	1.00	35	1.00	35	0	45%		17	45%		13		35 + [17] + (13) = 65	
SB	T	1167	1.00	1,167	1.00	1149	0			0			0	1149		
	R	77	1.00	77	1.00	77	0			0			0	77	77	

Current Year 2022

Intersection Volumes

Period	Tgen	Enter	Exit	Pass By	Enter	Exit	SF	Years	Legend
PM Peak		33	32	Trips	41	41	1.00	2	Existing + [Pass-by] + (Project) = Total

Inte	rsectio	n=	S	W F	Port St L	.ucie	Blvd &	RI/RO A	Access	Drivewa	ay							1
Appro	ach Mvmt	Raw	SF		Adjusted	GR	Adj Bg'd	Vested	% PB Ent	% PB Ext	Passby	%Proj Ent	%Proj Ext	Project	Total		Formula	
	L	(0 1	.00	0	1.00	0	0	· <u></u>	· · · · · · · · · · · · · · · · · · ·	0	<u> </u>		0	0			
EB	Т	(0 1	.00	0	1.00	0	0			0			0	0			
	R	(0 1	.00	0	1.00	0	0			0			0	0			
	L		0 1	.00	0	1.00	0	0			0			0	0			
WB	Т	(0 1	.00	0	1.00	0	0			0			0	0			
	R		9 1	.00	9	1.00	9	0		45%	13		30%	10	32 9	9 + [13] + (10) = 32		
	L	(0 1	.00	0	1.00	0	0			0			0	0			
NB	Т	1394	1 1	.00	1,394	1.00	1374	0			0	15%		5	1379 1	1374 + (5) = 1379		
	R	3	3 1	.00	3	1.00	3	0	45%		20	20%		7	30 3	3 + [20] + (7) = 30		
	L		0 1	.00	0	1.00	0	0			0			0	0			
SB	Т	1911	1 1	.00	1,911	1.00	1911	0			0	45%		14	1925 1	1911 + (14) = 1925		
	R	(0 1	.00	0	1.00	0	0			0			0	0			

Appro	ach Mvmt	Raw	SF	Adjusted	GR	Adj Bg'd	Vested	% PB Ent	% PB Ext	Passby	%Proj Ent	%Proj Ext	Project	Total	Formula	
	L	0	1.00	0	1.00	0	0	50%	_	20	75%		24	44 [20] + (24) = 44		
ЕΒ	Т	179	1.00	179	1.00	159	0			0			0	159 159		
	R	0	1.00	0	1.00	0	0			0			0	0		
	L	0	1.00	0	1.00	0	0			0			0	0	•	
VΒ	Т	105	1.00	105	1.00	104	0			0			0	104 104		
	R	0	1.00	0	1.00	0	0	5%		1	5%		2	3 [1] + (2) = 3		
	L	0	1.00	0	1.00	0	0		_	0			0	0		
NΒ	Т	0	1.00	0	1.00	0	0			0			0	0		
	R	0	1.00	0	1.00	0	0			0			0	0		
	L	0	1.00	0	1.00	0	0		5%	2		5%	1	3 [2] + (1) = 3		
SB	T	0	1.00	0	1.00	0	0			0			0	0		
	R	0	1.00	0	1.00	0	0		50%	26		65%	21	47 [26] + (21) = 47		

	rsectio			Port St L											
Appro	ach Mvmt	Raw	SF	Adjusted	GR	Adj Bg'd	Vested	% PB Ent	% PB Ext	Passby	%Proj Ent	%Proj Ext	Project	Total	Formula
	L	74	1.00	74	1.00	74	0			0			0	74 74	
EB	Т	5	1.00	5	1.00	5	0			0	5%		2	7 5 + (2) = 7	
	R	30	1.00	30	1.00	30	0			0			0	30 30	
	L	1	1.00	1	1.00	1	0			0		45%	14	15 1 + (14) = 15	
WB	Т	2	1.00	2	1.00	2	0			0		5%	2	4 2 + (2) = 4	
	R	102	1.00	102	1.00	102	0			0		15%	5	107 102 + (5) = 107	
	U	1	1.00	1	1.00	1	0			0			0	1 1	
	L	16	1.00	16	1.00	16	0			0			0	16 16	
NB	Т	1218	1.00	1,218	1.00	1218	0			0	20%		7	1225 1218 + (7) = 1225	
	R	32	1.00	32	1.00	32	0			0	25%		8	40 32 + (8) = 40	
	U	9	1.00	9	1.00	9	0			0			0	9	
	L	142	1.00	142	1.00	142	0	45%		20	45%		14	176 142 + [20] + (14) = 176	3
SB	Т	1603	1.00	1,603	1.00	1583	0			0			0	1583 1583	
	R	157	1.00	157	1.00	157	0			0			0	157 157	

Appendix J
HCM Projected Conditions

Intersection Int Delay, s/veh	3.3														
	5.5														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations		र्स	7		र्स	7		ă	ተ ተጉ			ă	ተ ተጉ		
Traffic Vol, veh/h	52	0	28	3	2	103	1	20	1208	12	6	35	1149	77	
Future Vol, veh/h	52	0	28	3	2	103	1	20	1208	12	6	35	1149	77	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	-	None	-	-	-	None	
Storage Length	_	_	105	_	_	115	_	350	_	-	_	310	_	-	
Veh in Median Storage	.# -	1	-	_	1	-	_	-	0	_	_	-	0	_	
Grade, %	, <i>''</i>	0	_	_	0	_	_	_	0	_	_	_	0	_	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	97	97	
Heavy Vehicles, %	2	3	2	3	3	8	2	2	11	2	2	2	2	2	
Mvmt Flow	54	0	29	3	2	106	1	21	1245	12	6	36	1185	79	
THE TOTAL	0.				_	100	•		1210				1100	, ,	
Major/Minor N	/linor2		N	Minor1		1	Major1			N	//ajor2				
Conflicting Flow All	1852	2610	632	1853	2643	629	923	1264	0	0	918	1257	0	0	
Stage 1	1309	1309		1295	1295	-	-	-	_	-	-	-	_	_	
Stage 2	543	1301	-	558	1348	-	-	-	_	_	-	-	_	_	
Critical Hdwy	6.44	6.56	7.14	6.46	6.56	7.26	5.64	5.34	_	-	5.64	5.34	-	_	
Critical Hdwy Stg 1	7.34	5.56	-	7.36	5.56	-	-	-	_	_	-	-	_	_	
Critical Hdwy Stg 2	6.74	5.56	_	6.76	5.56	-	_	-	_	-	-	-	_	_	
Follow-up Hdwy	3.82	4.03	3.92	3.83	4.03	3.98	2.32	3.12	_	_	2.32	3.12	_	_	
Pot Cap-1 Maneuver	78	24	363	77	23	353	485	291	_	_	488	294	_	_	
Stage 1	121	225	-	123	229	-	-	-	_	_	-	-	_	_	
Stage 2	449	227	-	437	216	-	_	-	_	-	_	-	_	_	
Platoon blocked, %									_	_			_	_	
Mov Cap-1 Maneuver	~ 45	19	363	60	18	353	296	296	_	_	301	301	_	_	
Mov Cap-2 Maneuver	87	91	-	96	92	-	-	-	_	_	-	-	_	_	
Stage 1	112	194	-	114	212	-	_	-	_	-	_	-	_	_	
Stage 2	288	210	-	346	186	_	-	_	_	_	-	-	_	_	
J 11 G															
Approach	EB			WB			NB				SB				
HCM Control Delay, s	69			20.7			0.3				0.6				
HCM LOS	F			С											
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1	EBLn2V	VBLn1V	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)		296	-	-	87	363	94	353	301	-	-				
HCM Lane V/C Ratio		0.073	-	-	0.616	0.08	0.055		0.14	-	-				
HCM Control Delay (s)		18.1	-	-	97.6	15.8	45.5	19.5	18.9	-	-				
HCM Lane LOS		С	-	-	F	С	Ε	С	С	-	-				
HCM 95th %tile Q(veh)		0.2	-	-	2.9	0.3	0.2	1.2	0.5	-	-				
Notes															
~: Volume exceeds cap	acity	\$: De	lay exc	eeds 30)0s	+: Com	outation	Not De	efined	*: All r	maior v	olume i	in plato	on	

Intersection															
Int Delay, s/veh	40.9														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations		र्स	7		र्स	7		Ä	ተ ተጉ			Ä	ተተጐ		
Traffic Vol, veh/h	74	5	30	1	2	102	1	16	1218	32	9	142	1583	157	
Future Vol, veh/h	74	5	30	1	2	102	1	16	1218	32	9	142	1583	157	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	-	None	-	-	-	None	
Storage Length	_	_	105	_	-	115	-	350	_	-	-	310	_	-	
Veh in Median Storage	. # -	1	-	_	1	-	-	-	0	_	_	-	0	_	
Grade, %	-	0	_	_	0	_	_	_	0	_	_	_	0	_	
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94	94	94	
Heavy Vehicles, %	2	2	3	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	79	5	32	1	2	109	1	17	1296	34	10	151	1684	167	
VIVIIICT IOW	10	U	02	'		100	'		1200	04	10	101	1004	107	
Major/Minor I	Minor2		ľ	Minor1			Major1			N	Major2				
Conflicting Flow All	2645	3456	926	2347	3522	665	1351	1851	0	0	971	1330	0	0	
Stage 1	2090	2090	-	1349	1349	-	-	_	-	-	_	-	-	-	
Stage 2	555	1366	_	998	2173	-	-	-	-	_	-	-	-	_	
Critical Hdwy	6.44	6.54	7.16	6.44	6.54	7.14	5.64	5.34	_	_	5.64	5.34	_	-	
Critical Hdwy Stg 1	7.34	5.54	-	7.34	5.54	_	-	-	_	_	-	-	_	_	
Critical Hdwy Stg 2	6.74	5.54	_	6.74	5.54	_	-	_	_	_	_	_	_	_	
Follow-up Hdwy	3.82	4.02	3.93	3.82	4.02	3.92	2.32	3.12	_	_	2.32	3.12	_	_	
Pot Cap-1 Maneuver	~ 25	7	231	38	6	345	280	149	_	_	456	270	_	_	
Stage 1	~ 33	93		113	217	-			_	_	-		_	_	
Stage 2	441	213	_	236	84	_	_	_	_	_	_	_	_	_	
Platoon blocked, %	771	210		200	O-T				_	_			_	_	
Mov Cap-1 Maneuver	~ 8	~ 3	231	12	~ 2	345	152	152	_	_	273	273	_	_	
Mov Cap-2 Maneuver	~ 23	16	-	37	14	0-10	102	102	_	_	210	210	_	_	
Stage 1	~ 29	38	_	100	191	_	_	_		_	_	_	_	_	
Stage 2	264	188	_	72	34			_	_	_			_	_	
Stage 2	204	100	-	12	34					-				-	
Approach	EB			WB			NB				SB				
HCM Control Delay, \$ 1	1187 9			26.6			0.4				2.8				
HCM LOS	F			D			V .,				2.0				
				_											
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1	EBLn2V	VBLn1\	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)		152	-	-	22	231	18	345	273	-	-				
HCM Lane V/C Ratio		0.119	-	-	3.82	0.138	0.177	0.315	0.588	-	-				
HCM Control Delay (s)		31.8	-	\$	1630.2	23.1	244	20.2	35.5	-	-				
HCM Lane LOS		D	-	-	F	С	F	С	Е	-	-				
HCM 95th %tile Q(veh))	0.4	-	-	10.7	0.5	0.5	1.3	3.4	-	-				
Notes															

Intersection						
Int Delay, s/veh	0.2					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			444			^ ^
Traffic Vol, veh/h	0	29	1348	30	0	1298
Future Vol, veh/h	0	29	1348	30	0	1298
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	32	1465	33	0	1411
WWITHER TOW	U	UZ	1400	00	U	1711
Major/Minor N	/linor1	ا	Major1	N_	/lajor2	
Conflicting Flow All	-	749	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	_	-	-	-	-
Critical Hdwy	_	7.14	_	_	_	_
Critical Hdwy Stg 1	_		_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	3.92	_			
		304		-	-	-
Pot Cap-1 Maneuver	0		-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	304	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	_	_	_	_	-	_
go <u>-</u>						
Approach	WB		NB		SB	
HCM Control Delay, s	18.2		0		0	
HCM LOS	С					
N. 1 (N. 1 N. 1		NDT	NDD	MDL 4	ODT	
Minor Lane/Major Mvm	t	NBT	NBK	VBLn1	SBT	
Capacity (veh/h)		-	-		-	
HCM Lane V/C Ratio				0.104	-	
HCM Control Delay (s)		-	-	18.2	-	
HCM Lane LOS		-	-	С	-	
HCM 95th %tile Q(veh)		-	-	0.3	-	
(1011)						

Intersection						
Int Delay, s/veh	0.2					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations			ተተጐ			444
Traffic Vol, veh/h	0	32	1379	30	0	
Future Vol, veh/h	0	32	1379	30	0	1925
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	,# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	35	1499	33	0	2092
WWW.	U	00	1700	- 00	- 0	2002
Major/Minor N	/linor1	l l	Major1	<u> </u>	/lajor2	
Conflicting Flow All	-	766	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	7.14	-	_	_	_
Critical Hdwy Stg 1	_		_	_	_	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	<u>-</u>	3.92	_	_	_	_
Pot Cap-1 Maneuver	0	296		_	0	_
	0			_	0	-
Stage 1		-	-			
Stage 2	0	-	-	-	0	-
Platoon blocked, %		205	-	-		-
Mov Cap-1 Maneuver	-	296	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
A	MD		ND		O.P.	
Approach	WB		NB		SB	
HCM Control Delay, s	18.8		0		0	
HCM LOS	С					
Minor Lane/Major Mvm		NDT	NIDDV	MDI p1	SBT	
		NBT		WBLn1		
Capacity (veh/h)		-	-	_00	-	
HCM Lane V/C Ratio		-	-	0.118	-	
HCM Control Delay (s)		-	-	18.8	-	
HCM Lane LOS		-	-	С	-	
HCM 95th %tile Q(veh)		-	-	0.4	-	

Intersection						
Int Delay, s/veh	2.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	1₃	WDIX	Y	ODIN
Traffic Vol, veh/h	37	47	106	3	2	34
Future Vol, veh/h	37	47	106	3	2	34
Conflicting Peds, #/hr	0	0	0	0	0	0
•	Free	Free	Free	Free	Stop	
Sign Control						Stop
RT Channelized	-		-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	40	51	115	3	2	37
Major/Minor	Major1	N	Major2		Minor2	
Conflicting Flow All	118	0	- viajoiz	0	248	117
	110	-			117	- 117
Stage 1			-	-		
Stage 2	- 4.40	-	-	-	131	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1470	-	-	-	740	935
Stage 1	-	-	-	-	908	-
Stage 2	-	-	-	-	895	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1470	-	-	-	719	935
Mov Cap-2 Maneuver		-	-	-	719	-
Stage 1	-	-	_	_	883	-
Stage 2	_	_	_	_	895	_
Olago 2					000	
Approach	EB		WB		SB	
HCM Control Delay, s	3.3		0		9.1	
HCM LOS					Α	
Minor Lanc/Major Mun	nt	EBL	EBT	\\/DT	WBR	CDI 51
Minor Lane/Major Mvr	iit			WBT	WDK	
Capacity (veh/h)		1470	-	-	-	920
HCM Lane V/C Ratio	,	0.027	-	-		0.043
HCM Control Delay (s)	7.5	0	-	-	9.1
HCM Lane LOS		Α	Α	-	-	Α
HCM 95th %tile Q(veh	1)	0.1	-	-	-	0.1

Intersection						
Int Delay, s/veh	2.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EDL			WDK		אמט
Lane Configurations	11	વ	104	2	Y	17
Traffic Vol, veh/h	44	159	104	3	3	47
Future Vol, veh/h	44	159	104	3	3	47
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	48	173	113	3	3	51
WWIIICTIOW	40	170	110	U	U	01
	Major1		Major2		Minor2	
Conflicting Flow All	116	0	-	0	384	115
Stage 1	-	-	-	-	115	-
Stage 2	-	-	-	-	269	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	_	_	_	_	5.42	_
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_	_	3.518	
Pot Cap-1 Maneuver	1473	_	_	_	619	937
Stage 1	-		_	_	910	331
	-	_			776	
Stage 2	-	-	-	-	110	-
Platoon blocked, %	4.470	-	-	-	507	007
Mov Cap-1 Maneuver	1473	-	-	-	597	937
Mov Cap-2 Maneuver	-	-	-	-	597	-
Stage 1	-	-	-	-	877	-
Stage 2	-	-	-	-	776	-
Approach	EB		WB		SB	
	1.6		0			
HCM Control Delay, s	1.0		U		9.2	
HCM LOS					Α	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1473	-			906
HCM Lane V/C Ratio		0.032	_	_	_	0.06
HCM Control Delay (s)		7.5	0	_	_	9.2
HCM Lane LOS		7.5 A	A		_	9.2 A
HCM 95th %tile Q(veh)	١	0.1		-	-	0.2
			_	-	-	U.Z

Intersection															
Int Delay, s/veh	4.5														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations		सी	7		4	7		4	^			A	11		
Traffic Vol, veh/h	52	1	28	14	3	108	1	20	1213	18	6	65	1149	77	
Future Vol, veh/h	52	1	28	14	3	108	1	20	1213	18	6	65	1149	77	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	-	None	-	-	-	None	
Storage Length	-	-	105	-	-	115	-	350	-	-	-	310	-	-	
Veh in Median Storage	,# -	1	-	-	1	-	-	-	0	-	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	-	0	-	-	-	0	-	
Peak Hour Factor	97	97	97	97	97	97	97	97	97	97	97	97	97	97	
Heavy Vehicles, %	2	3	2	3	3	8	2	2	11	2	2	2	2	2	
Mvmt Flow	54	1	29	14	3	111	1	21	1251	19	6	67	1185	79	
	•	•					•					· ·			
Major/Minor N	/linor2		N	Minor1			Major1			N	Major2				
Conflicting Flow All	1917	2685	632	1926	2715	635	923	1264	0	0	926	1270	0	0	
Stage 1	1371	1371	032	1305	1305	033	923	1204	-	-	920	1270	-	-	
		1314		621				-	-	_	-				
Stage 2	546		711		1410	7.06	E 64	- - 24	-	-	F 6.4	- - 24	-	-	
Critical Hdwy	6.44	6.56	7.14	6.46	6.56	7.26	5.64	5.34	-	-	5.64	5.34	-	-	
Critical Hdwy Stg 1	7.34	5.56	-	7.36	5.56	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.74	5.56	-	6.76	5.56	-	- 0.00	- 0.40	-	-	-	-	-	-	
Follow-up Hdwy	3.82	4.03	3.92	3.83	4.03	3.98	2.32	3.12	-	-	2.32	3.12	-	-	
Pot Cap-1 Maneuver	71	21	363	70	20	350	485	291	-	-	483	289	-	-	
Stage 1	109	210	-	121	226	-	-	-	-	-	-	-	-	-	
Stage 2	447	224	-	400	201	-	-	-	-	-	-	-	-	-	
Platoon blocked, %									-	-			-	-	
Mov Cap-1 Maneuver	~ 36	15	363	49	14	350	296	296	-	-	292	292	-	-	
Mov Cap-2 Maneuver	76	73	-	89	79	-	-	-	-	-	-	-	-	-	
Stage 1	101	158	-	112	209	-	-	-	-	-	-	-	-	-	
Stage 2	278	207	-	274	151	-	-	-	-	-	-	-	-	-	
Approach	EB			WB			NB				SB				
HCM Control Delay, s	89.3			25			0.3				1.2				
HCM LOS	F			D											
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1	EBLn2V	VBLn1\	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)		296	-		76	363	87	350	292						
HCM Lane V/C Ratio		0.073	_		0.719	0.08		0.318		_	_				
HCM Control Delay (s)		18.1	-		128.1	15.8	56.5	20	21.4		_				
HCM Lane LOS		10.1 C		-	120.1 F	13.6 C	50.5 F	20 C	21.4 C						
HCM 95th %tile Q(veh)		0.2	-	-	3.4	0.3	0.7	1.3	1	-	-				
		U.Z	-	_	3.4	0.3	0.7	1.3		-	_				
Notes															
~: Volume exceeds cap	acity	\$: De	lay exc	eeds 30	00s	+: Com	putation	n Not De	efined	*: All	major v	olume i	n plato	on	

Intersection															
Int Delay, s/veh	15.7														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations		र्स	7		र्स	7		ă	ተ ተጉ			ă	ተ ተጉ		
Traffic Vol, veh/h	74	7	30	15	4	107	1	16	1225	40	9	176	1583	157	
Future Vol, veh/h	74	7	30	15	4	107	1	16	1225	40	9	176	1583	157	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	-	None	-		-	None	
Storage Length	_	_	105	_	_	115	_	350	_	-	_	310	_	-	
Veh in Median Storage	e.# -	1	-	_	1	-	_	-	0	_	_	-	0	_	
Grade, %	- -	0	_	_	0	_	_	_	0	_	_	_	0	_	
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94	94	94	
Heavy Vehicles, %	2	2	3	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	79	7	32	16	4	114	1	17	1303	43	10	187	1684	167	
IVIVIII(I IOW	13	1	JZ	10	7	117		17	1000	40	10	107	1004	107	
Major/Minor	Minor2		N	Minor1		ľ	Major1			N	/lajor2				
Conflicting Flow All	2721	3544	926	2432	3606	673	1351	1851	0	0	982	1346	0	0	
Stage 1	2162	2162	-	1361	1361	-	-	-	-	-	-	-	-	-	
Stage 2	559	1382	-	1071	2245	-	_	_	-	-	-	-	-	-	
Critical Hdwy	6.44	6.54	7.16	6.44	6.54	7.14	5.64	5.34	_	_	5.64	5.34	-	_	
Critical Hdwy Stg 1	7.34	5.54	-	7.34	5.54	_	-	-	_	_	-	-	_	_	
Critical Hdwy Stg 2	6.74	5.54	_	6.74	5.54	_	_	_	_	_	_	_	_	_	
Follow-up Hdwy	3.82	4.02	3.93	3.82	4.02	3.92	2.32	3.12	_	_	2.32	3.12	_	_	
Pot Cap-1 Maneuver	~ 22	~ 6	231	34	5	341	280	149	_	_	450	265	_	_	
Stage 1	~ 29	85		111	215	-		-	_	_	-		_	_	
Stage 2	439	210	_	212	77	_	_	_	_	_	_	_	_	_	
Platoon blocked, %	700	210		212	11				_	_			_	_	
Mov Cap-1 Maneuver	_	~ 1	231	~ 12	~ 1	341	152	152	_	_	267	267	_	_	
Mov Cap-1 Maneuver	~ 16	~ -30	231	~ 13	~ 2	J 4 I	102	132	_	_	201	201	_		
Stage 1	~ 26	22		98	190			-		_	_		_	_	
	252	185	-	32	20	_	-	-	-	_	-	_	_		
Stage 2	202	100	-	32	20	-	_	-	-	-	_	-	_	-	
Approach	EB			WB			NB				SB				
HCM Control Delay, s				\$ 354			0.4				4.7				
HCM LOS	_			F											
				•											
						-DI 6	VD1 4	VD1 -	0-1	05-	05.5				
Minor Lane/Major Mvn	nt	NBL	NBT	NBR I	BLn1	EBLn2V			SBL	SBT	SBR				
Capacity (veh/h)		152	-	-	-	231	6	341	267	-	-				
HCM Lane V/C Ratio		0.119	-	-	-	0.138		0.334	0.737	-	-				
HCM Control Delay (s)		31.8	-	-	-		2230.5	20.8	48.7	-	-				
HCM Lane LOS		D	-	-	-	С	F	С	Е	-	-				
HCM 95th %tile Q(veh)	0.4	-	-	-	0.5	3.8	1.4	5.3	-	-				
Notes															
~: Volume exceeds ca	nacity	¢. Da	lay ovo	eeds 30)Oc	+: Com	outation	Not D	ofined	*. All	maiory	olumo	n plato	nn -	
. volume exceeds ca	pacity	φ. De	lay exc	ccus 3 (005	+. Com	Julation	INUL DE	Sillieu	. All	najoi V	olullie	ii piatot	ווע	

Appendix KTurn Lane Warrants

8.12.7 Right Turn Lanes

(a) The use of a continuous right turn lane shall be avoided. Exclusive right turn lanes for driveways are required when the operational aspects of the driveway meet the volume and speed criteria presented in Table 8-5, where a traffic study indicates that the LOS is degraded by the proposed development, or where required for safety reasons even though the peak hour turn volumes may be lower than specified in Table 8-5.

Table 8-5 Unsignalized Driveway Right Turn Lanes ^{1, 4, 5}								
Roadway Posted Speed Limit	Number of Right Turns Per Hour							
45 mph or less	80-125 ²							
Over 45 mph	35-55 ³							

¹Source: FDOT Access Management Guidebook, Table 27

- (b) An exclusive right turn lane shall be required, even if the speed and volume criteria is not met, when one of the following conditions exist:
 - (1) Developments that have a high volume of buses, trucks, or trailers.
 - (2) Poor internal circulation that may cause backups onto the roadway.
 - (3) Heavier than normal peak flows on the roadway.
 - (4) Very high operating speeds on the roadway.
 - (5) Areas where turns are not expected.
 - (6) Roadways with curves, hills, or other sight distance restrictions.
 - (7) Gated entrances.
 - (8) An area with a history of crashes, especially rear end collisions.
 - (9) Intersections or driveways just after a signalized intersection where acceleration typically occurs.
 - (10) A driveway with a severe skewered angle.
 - (11) Areas of heightened safety concern.

²The lower threshold of eighty right turn vehicles per hour would be most used for higher volume (greater than 600 vehicles per hour, per lane in one direction on the major roadway) or two-lane roads where lateral movement is restricted. The 125 right turn vehicles per hour upper threshold would be most appropriate on lower volume roadways, multilane highways, or driveways with a large entry radius (fifty feet or greater).

³The lower threshold of thirty-five right turn vehicles per hour would be most appropriately used on higher volume two lane roadways where lateral movement is restricted. The fifty-five right turn vehicles per hour upper threshold would be most appropriate on lower volume roadways, multilane highways, or driveways with large entry radius (fifty feet or greater).

⁴A posted speed limit of over forty-five mph may be used if the operating speeds are known to be over forty-five mph during the time of peak right turn demand.

⁵Projecting turning volumes is, at best, a knowledgeable estimate. Keep this in mind especially if the projections of right turns are close to meeting the guidelines. In that case, consider requiring the turn lane.

8.12.8 Left Turn Lanes

A left turn lane for driveways shall be provided:

- (a) Whenever a driveway is served by a median opening.
- (b) On a two-lane road, on curves, or whenever speeds are forty-five mph and greater.
- (c) Where a traffic study shows that the LOS is degraded by the proposed traffic.
- (d) When warranted by the NCHRP Report 745 analysis and Report 279.

8.13 Clear Visibility Triangle

In order to provide a clear view of intersecting streets and driveway entrances, a triangular area of clear visibility shall meet the following standards.

- (a) Nothing shall be located, erected, placed, planted, or allowed to grow in such a manner as to impede vision between a height of three feet and eight feet within the triangular area.
- (b) Road Intersections: The clear visibility triangle shall be formed by drawing a line twenty-five feet along each property line abutting the right-of-way starting at the point where the two property lines intersect or their projections intersect, then connecting the two end points with a straight line as shown in Figure 8-2.

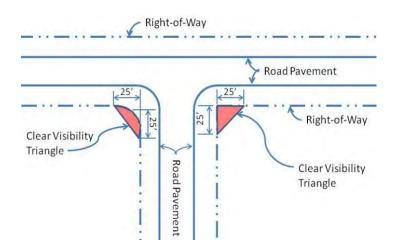
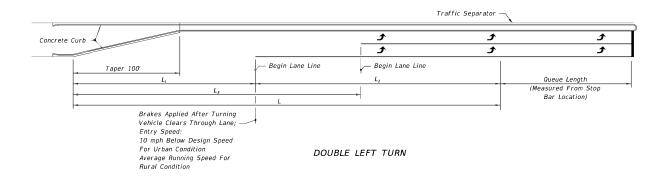
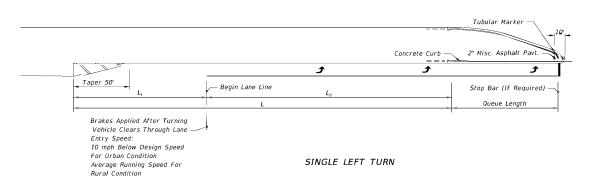


Figure 8-2 Clear Visibility Triangle at Intersection

(c) Driveways: A clear visibility triangle shall be formed as shown in Figure 8-3. Beginning at the intersection of the driveway with the road right-of-way, then along the right-of-way for a distance of twenty-five feet, then in a straight line across the property to a point on the edge of the driveway twenty-five feet from the point of beginning. Where driveways are curved or intersect with the street at other than right angles, the visibility triangle shall be measured from the point of the curve most projecting into the driveway.

MEDIAN TURN LANES MINIMUM DECELERATION LENGTHS





MEDIAN TURN LANES											
			URBA	AN CONDIT	IONS	RURAL CONDITIONS					
Design Speed (mph)	Entry Speed (mph)	Clearance Distance L; (ft.)	Brake To Stop Distance L₂ (ft.)	Total Decel. Distance L (ft.)	Clearance Distance L ₃ (ft.)	Brake To Stop Distance L ₂ (ft.)	Total Decel. Distance L (ft.)	Clearance Distance L ₃ (ft.)			
35	25	70	75	145	110						
40	30	80	75	155	120						
45	35	85	100	185	135						
50	40/44	105	135	240	160	185	290	160			
55	48	125				225	350	195			
60	52	145				260	405	230			
65	55	170				290	460	270			

NOT TO SCALE

EXHIBIT 212-1 01/01/2023

PM Peak Hour-Left Turn Lane Warrant

SW Aviation Ave and Full Access Dvwy

Figure 2 - 5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.

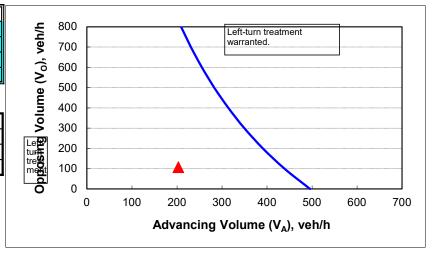
2-lane roadway (English)

INPUT

Variable	Value
85 th percentile speed, mph:	25
Percent of left-turns in advancing volume (V _A), %:	22%
Advancing volume (V _A), veh/h:	203
Opposing volume (V _o), veh/h:	107

OUTPUT

Variable	Value					
Limiting advancing volume (V _A), veh/h:	435					
Guidance for determining the need for a major-road left-turn bay:						
Left-turn treatment NOT warranted.						



CALIBRATION CONSTANTS

Variable	Value
Average time for making left-turn, s:	3.0
Critical headway, s:	5.0
Average time for left-turn vehicle to clear the advancing lane, s:	1.9