

SANDPIPER BAY SCHOOL

PORT ST. LUCIE, FL

TRAFFIC IMPACT STATEMENT

PREPARED FOR:

RPS Academies 4500 SE Pine Valley Street Port St. Lucie, Florida 34952

JOB NO. 24-124

DATE: 10/16/2024 Revised 10/30/2024

Bryan G. Kelley, Professional Engineer, State of Florida, License No. 74006

This Item has been digitally signed and sealed by Bryan G. Kelley, P.E., on 10/30/2024.

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1.0 SITE DATA

The subject parcel is located within the Sandpiper Bay Resort on SE Pine Valley Street in the City of Port St. Lucie, Florida. As part of the PUD and Site Plan amendment for the approximately 223.72 acre project to known as Club Med/Sandpiper PUD, the applicant is seeking approval for a conditional use request for a school. It should be noted the school is existing and currently operational with 117 students. For purposes of this traffic study, a 150student private high school will be assumed. Site access is not proposed to change and is existing via SE Pine Valley Street and SE Morningside Boulevard. For additional information on site layout, please refer to the site plan prepared by Keith. Note the existing school is located within building M shown on the Site Plan.

2.0 TRAFFIC GENERATION

The traffic to be generated by the existing development has been calculated in accordance with the ITE Trip Generation Manual, 11th Edition. Note the A.M. and P.M. peak hour of generators were used in the trip generation and not the adjacent street peak hour of traffic. Table 1 shows the daily traffic generation and Tables 2 and 3 show the A.M. and P.M. peak hour traffic generation. The traffic generated by the existing 117-student private high school may be summarized as follows:

```
Existing Development
Daily Traffic Generation
                                                 = 254 \text{ tpd}
A.M. Peak Hour Traffic Generation (In/Out) = 77 pht (45 In/32 Out)
P.M. Peak Hour Traffic Generation (In/Out) = 47 \text{ pht } (18 \text{ In}/29 \text{ Out})
```

The traffic to be generated by the proposed development has also been calculated using a the ITE Trip Generation Manual, 11th Edition. The traffic to be generated by the proposed plan of development consisting of a 150-student private high school is shown in Tables 4-6 and may be summarized as follows:

```
Proposed Development
Daily Traffic Generation
                                                      = 326 \text{ tpd}
A.M. Peak Hour Traffic Generation (In/Out) = 99 \text{ pht } (58 \text{ In}/41 \text{ Out})
P.M. Peak Hour Traffic Generation (In/Out) = 60 \text{ pht } (23 \text{ In}/37 \text{ Out})
```

The net new trips associated with the difference between the proposed and existing developments is shown in Table 7 and may be summarized as follows:

```
Net Trips (Proposed – Existing)
Daily Traffic Generation
                                               72 tpd
A.M. Peak Hour Traffic Generation (In/Out) =
                                               22 pht (13 In/9 Out)
P.M. Peak Hour Traffic Generation (In/Out) =
                                               13 pht (5 In/8 Out)
```

The number of existing and proposed trips shown above are based on the ITE Trip Generation rates and can be considered conservative. The school is a specialized private school in which over 50% of the student population are boarding students. These students do not have their own vehicle and are chauffeured by school faculty as needed. Approximately 15% of the student population live in the nearby residences and arrive to school by walking or biking. The remaining students arrive to school by vehicle and are dropped off by 8:00 A.M. and picked up after 5:00 P.M.

3.0 TRAFFIC ANALYSIS

Per Appendix B of the St. Lucie County Standardized TIS Methodology and Procedures, the radius of influence for the project is ½ mile based on the daily trips. Figure 1 attached to this report shows the project trip distribution on the surrounding roadway network and the radius of influence. Note several roadways outside the radius of influence were included in the analysis to be conservative. The project's impact to each of the surrounding roadways are shown in Table 8 and Table 9 attached to this report which calculates the project trips relative to the Level of Service (LOS) D service volume threshold. In order to be conservative, the traffic analysis was based on a 150-student private school and not the difference between the proposed 150 student school and the existing 117 student school. Additionally, no discount was taken for the boarding students which significantly reduces trip volumes. The LOS D thresholds were based on the 2023 FDOT Q/LOS tables and the roadway classifications were determined from the FDOT Preliminary Context Classification GIS map.

Roadways in which the project trip impact was greater than 1.0% impact were further evaluated per the St. Lucie TPO Standardized TIS Methodology and Procedures report. The existing traffic counts were taken from the 2024 St. Lucie Traffic Counts and Level of Service Report. A 2.5% background growth rate was applied to the existing traffic counts to the buildout year of 2027 based on historical population growth in St. Lucie County (See Appendix E). Additionally, background traffic was also considered for the Ravello Development and Westmoreland Riverwalk projects.

The roadway segment analysis is shown in Tables 9 and 10 and demonstrate each of the impacted roadways will meet Level of Service requirements.

4.0 **SCHOOL OPERATIONS**

As previously stated, over 50% of the student population are boarding students and do not own vehicles. An additional 15% of students live in nearby residences and walk and bike to school. The remaining students are dropped off in the morning and picked up in the afternoon. The school does not have a traditional school queue line due to the low number of students being dropped off and picked up on a daily basis. The school shares parking with the resort as shown in Figure 2 attached to this report. Parents will briefly park to allow their students to exit the vehicle. Once the student has safely exited the vehicle, parents will depart the parking lot. School faculty also utilizes this same parking lot. School events are to be coordinated with the resort to minimize peak impact and to ensure adequate parking is available for the resort guests and the school events. School events typically occur within the resort and therefore parking and traffic are already accounted for.

5.0 CONCLUSION

The attached tables document the daily, A.M. peak hour and P.M. peak hour traffic generation for the proposed development. The proposed development will conservatively generate 72 new daily trips, 22 new A.M. peak hour trips and 13 new P.M. peak hour trips. Based on the findings of this report, the surrounding roadway network will continue to meet acceptable Level of Service standards inclusive of the proposed development.

bk: x:/docs/trafficdrainage/tis.24124

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EXISTING DEVELOPMENT

TABLE 1 - Daily Traffic Generation

| | ITE | | | | Dir | Split | | Inte | ernalization | | Pass- | -by | |
|---------------------|------|-----|---------------|---------------|-----|-------|-------------|------|--------------|----------------|-------|-------|-----------|
| Landuse | Code | I | ntensity | Rate/Equation | In | Out | Gross Trips | % | Total | External Trips | % | Trips | Net Trips |
| Private High School | 534 | 117 | Students | 2.17 | | | 254 | | 0 | 254 | 0% | 0 | 254 |
| | | | Grand Totals: | | | | 254 | 0.0% | 0 | 254 | 0% | 0 | 254 |

TABLE 2 - AM Peak Hour Traffic Generation

| | ITE | | | | Dir | Split | Gı | oss T | rips | Inte | ernali | zation | | Ext | ernal | Trips | Pass- | by | 1 | Net Tri | ps |
|---------------------|------|-----|---------------|---------------|------|-------|----|-------|-------|------|--------|--------|-------|-----|-------|-------|-------|-------|----|---------|-------|
| Landuse | Code | | ntensity | Rate/Equation | In | Out | In | Out | Total | % | In | Out | Total | In | Out | Total | % | Trips | In | Out | Total |
| Private High School | 534 | 117 | Students | 0.66 | 0.59 | 0.41 | 45 | 32 | 77 | 0.0% | 0 | 0 | 0 | 45 | 32 | 77 | 0% | 0 | 45 | 32 | 77 |
| | | | Grand Totals: | | | | 45 | 32 | 77 | 0.0% | 0 | 0 | 0 | 45 | 32 | 77 | 0% | 0 | 45 | 32 | 77 |

TABLE 3 - PM Peak Hour Traffic Generation

| | ITE | | | | Dir | Split | Gr | oss T | rips | Inte | ernali | zation | | Ext | ernal | Trips | Pass- | by | N | let Tri | ps |
|---------------------|------|-----|---------------|---------------|------|-------|----|-------|-------|------|--------|--------|-------|-----|-------|-------|-------|-------|----|---------|-------|
| Landuse | Code | I | ntensity | Rate/Equation | In | Out | In | Out | Total | % | In | Out | Total | ln | Out | Total | % | Trips | ln | Out | Total |
| Private High School | 534 | 117 | Students | 0.4 | 0.39 | 0.61 | 18 | 29 | 47 | 0.0% | 0 | 0 | 0 | 18 | 29 | 47 | 0% | 0 | 18 | 29 | 47 |
| | | | Grand Totals: | | | | 18 | 29 | 47 | 0.0% | 0 | 0 | 0 | 18 | 29 | 47 | 0% | 0 | 18 | 29 | 47 |



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PROPOSED DEVELOPMENT

TABLE 4 - Daily Traffic Generation

| | ITE | | | | Dir | Split | | Inte | ernalization | | Pass- | -by | |
|---------------------|------|-----|---------------|---------------|-----|-------|-------------|------|--------------|----------------|-------|-------|-----------|
| Landuse | Code | 1 | ntensity | Rate/Equation | In | Out | Gross Trips | % | Total | External Trips | % | Trips | Net Trips |
| Private High School | 534 | 150 | Students | 2.17 | | | 326 | | 0 | 326 | 0% | 0 | 326 |
| | | | Grand Totals: | | | | 326 | 0.0% | 0 | 326 | 0% | 0 | 326 |

TABLE 5 - AM Peak Hour Traffic Generation

| | ITE | | | | Dir | Split | Gr | oss T | rips | Inte | ernali | zation | | Ext | ernal | Trips | Pass- | -by | 1 | let Tri | ps |
|---------------------|------|-----|---------------|---------------|------|-------|----|-------|-------|------|--------|--------|-------|-----|-------|-------|-------|-------|----|---------|-------|
| Landuse | Code | ı | ntensity | Rate/Equation | In | Out | In | Out | Total | % | In | Out | Total | In | Out | Total | % | Trips | In | Out | Total |
| Private High School | 534 | 150 | Students | 0.66 | 0.59 | 0.41 | 58 | 41 | 99 | 0.0% | 0 | 0 | 0 | 58 | 41 | 99 | 0% | 0 | 58 | 41 | 99 |
| | | | Grand Totals: | | | | 58 | 41 | 99 | 0.0% | 0 | 0 | 0 | 58 | 41 | 99 | 0% | 0 | 58 | 41 | 99 |

TABLE 6 - PM Peak Hour Traffic Generation

| | ITE | | | | Dir | Split | Gr | oss T | rips | Inte | ernali | zation | | Ext | ernal [*] | Trips | Pass- | by | N | let Tri | ps |
|---------------------|------|-----|---------------|---------------|------|-------|----|-------|-------|------|--------|--------|-------|-----|--------------------|-------|-------|-------|----|---------|-------|
| Landuse | Code | l | ntensity | Rate/Equation | In | Out | In | Out | Total | % | In | Out | Total | In | Out | Total | % | Trips | ln | Out | Total |
| Private High School | 534 | 150 | Students | 0.4 | 0.39 | 0.61 | 23 | 37 | 60 | 0.0% | 0 | 0 | 0 | 23 | 37 | 60 | 0% | 0 | 23 | 37 | 60 |
| | | | Grand Totals: | | | | 23 | 37 | 60 | 0.0% | 0 | 0 | 0 | 23 | 37 | 60 | 0% | 0 | 23 | 37 | 60 |



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TABLE 7
TRAFFIC GENERATION DIFFERENCE - NET TRIPS

| | | AM | PEAK HO | DUR | PM | PEAK HO | DUR |
|------------------------|-------|-------|---------|-----|-------|---------|-----|
| | DAILY | TOTAL | IN | OUT | TOTAL | IN | OUT |
| EXISTING DEVELOPMENT = | 254 | 77 | 45 | 32 | 47 | 18 | 29 |
| PROPOSED DEVELOPMENT = | 326 | 99 | 58 | 41 | 60 | 23 | 37 |
| DIFFERENCE = | 72 | 22 | 13 | 9 | 13 | 5 | 8 |



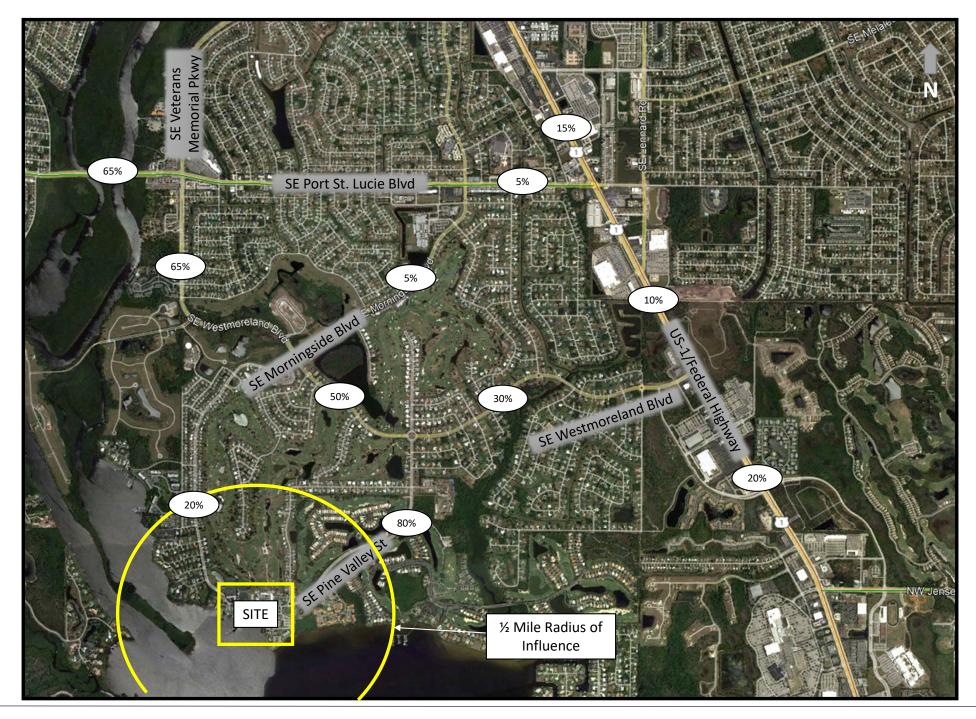
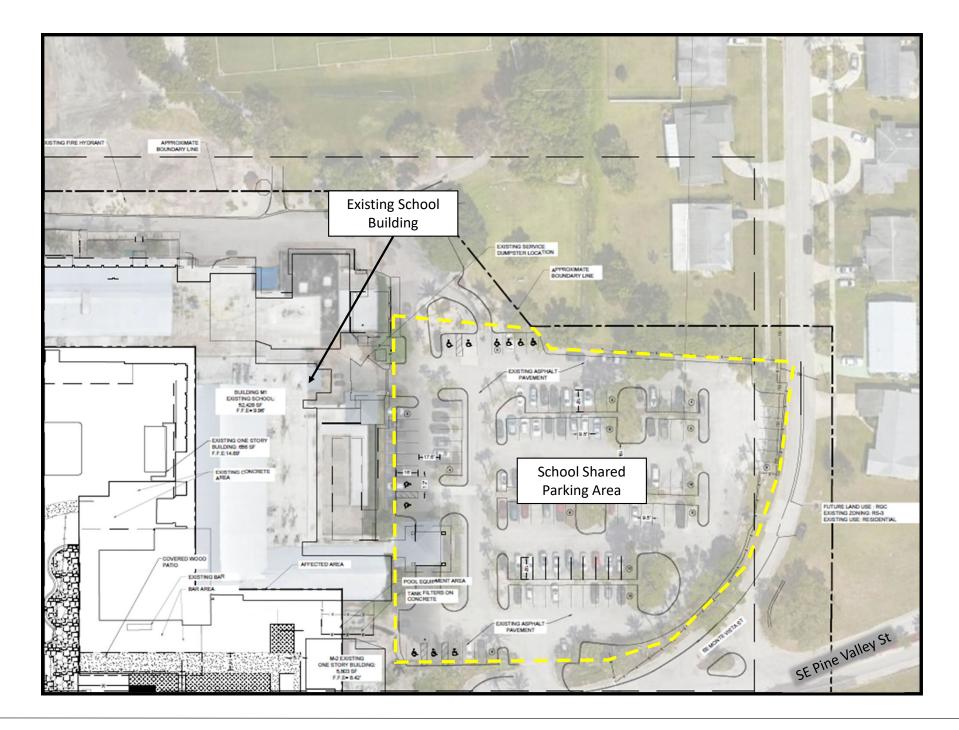


Figure 1 - Trip Distribution Sandpiper Bay School Project # 24-124











ROADWAY SEGMENT ANALYSIS

SANDPIPER BAY SCHOOL

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TABLE 8
PROJECT SIGNIFICANCE CALCULATION
AM PEAK HOUR

TOTAL AM PEAK HOUR PROJECT TRIPS (IN) = 58
TOTAL AM PEAK HOUR PROJECT TRIPS (OUT) = 41

| ROADWAY | FROM | то | PROJECT DISTRIBUTION | PEAK HOUR PROJECT TRIPS | EXISTING LANES | CLASS | LOS D STANDARD | TOTAL PROJECT IMPACT | PROJECT SIGNIFICANT |
|------------------------|------------------------|-------------------------|-------------------------|-------------------------------|-------------------|-------|-------------------|----------------------------|------------------------|
| SE PORT ST. LUCIE BLVD | FLORESTA DRIVE | VETERANS MEMORIAL PKWY | 65% | 38 | 6D | C4 | 2810 | 1.34% | YES |
| SE PORT ST. LUCIE BLVD | VETERANS MEMORIAL PKWY | MORNINGSIDE BLVD | 0% | 0 | 6D | C3R | 2730 | 0.00% | NO |
| SE PORT ST. LUCIE BLVD | MORNINGSIDE BLVD | US-1 | 5% | 3 | 6D | C3R | 2730 | 0.11% | NO |
| US-1 | MARTIN COUNTY LINE | LENNARD ROAD | 20% | 12 | 6D | C3C | 2680 | 0.43% | NO |
| US-1 | LENNARD ROAD | SE PORT ST. LUCIE BLVD | 10% | 6 | 6D | C3C | 2680 | 0.22% | NO |
| US-1 | LENNARD ROAD | JENNINGS ROAD | 15% | 9 | 6D | C3C | 2680 | 0.32% | NO |
| MORNINGSIDE BOULEVARD | SITE | WESTMORELAND BOULEVARD | 20% | 12 | 2 | C3R | 1110 | 1.05% | YES |
| MORNINGSIDE BOULEVARD | WESTMORELAND BOULEVARD | SE PORT ST. LUCIE BLVD | 5% | 3 | 2 | C3R | 1110 | 0.26% | NO |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | PORT ST LUCIE BOULEVARD | 70% | 41 | 2 | C3R | 1110 | 3.66% | YES |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | SE PINE VALLEY STREET | 55% | 32 | 2 | C3R | 1110 | 2.87% | YES |
| WESTMORELAND BOULEVARD | SE PINE VALLEY STREET | US-1 | 25% | 15 | 2 | C3R | 1110 | 1.31% | YES |



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TABLE 9
PROJECT SIGNIFICANCE CALCULATION
PM PEAK HOUR

TOTAL PM PEAK HOUR PROJECT TRIPS (IN) = 23
TOTAL PM PEAK HOUR PROJECT TRIPS (OUT) = 37

| ROADWAY | FROM | то | PROJECT DISTRIBUTION | PEAK HOUR PROJECT TRIPS | EXISTING LANES | CLASS | LOS D STANDARD | TOTAL PROJECT IMPACT | PROJECT SIGNIFICANT |
|------------------------|------------------------|-------------------------|-------------------------|-------------------------------|-------------------|-------|-------------------|----------------------------|------------------------|
| SE PORT ST. LUCIE BLVD | FLORESTA DRIVE | VETERANS MEMORIAL PKWY | 65% | 24 | 6D | C4 | 2810 | 0.86% | NO |
| SE PORT ST. LUCIE BLVD | VETERANS MEMORIAL PKWY | MORNINGSIDE BLVD | 0% | 0 | 6D | C3R | 2730 | 0.00% | NO |
| SE PORT ST. LUCIE BLVD | MORNINGSIDE BLVD | US-1 | 5% | 2 | 6D | C3R | 2730 | 0.07% | NO |
| US-1 | MARTIN COUNTY LINE | LENNARD ROAD | 20% | 7 | 6D | C3C | 2680 | 0.28% | NO |
| US-1 | LENNARD ROAD | SE PORT ST. LUCIE BLVD | 10% | 4 | 6D | C3C | 2680 | 0.14% | NO |
| US-1 | LENNARD ROAD | JENNINGS ROAD | 15% | 6 | 6D | C3C | 2680 | 0.21% | NO |
| MORNINGSIDE BOULEVARD | SITE | WESTMORELAND BOULEVARD | 20% | 7 | 2 | C3R | 1110 | 0.67% | NO |
| MORNINGSIDE BOULEVARD | WESTMORELAND BOULEVARD | SE PORT ST. LUCIE BLVD | 5% | 2 | 2 | C3R | 1110 | 0.17% | NO |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | PORT ST LUCIE BOULEVARD | 70% | 26 | 2 | C3R | 1110 | 2.33% | YES |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | SE PINE VALLEY STREET | 55% | 20 | 2 | C3R | 1110 | 1.83% | YES |
| WESTMORELAND BOULEVARD | SE PINE VALLEY STREET | US-1 | 25% | 9 | 2 | C3R | 1110 | 0.83% | NO |



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TABLE 10
ROADWAY SEGMENT ANALYSIS - AM PEAK HOUR

2027 BUILD OUT
TOTAL AM PEAK HOUR PROJECT TRII58
TOTAL AM PEAK HOUR PROJECT TRII41
BACKGROUND GROWTH = 2.5°

| DAGROROURD CROTTIII - | 2.070 | | | | | | | | | | | | | |
|------------------------|------------------------|--------------------------|--------------|-----------|-------|----------|------------|-------------|--------------|--------|----------|-------|----------|-------|
| | | | | PEAK HOUR | | | 2027 | RAVELLO | WESTMORELAND | 2027 | | | | |
| | | | PROJECT | PROJECT | COUNT | EXISTING | BACKGROUND | DEVELOPMENT | RIVERWALK | TOTAL | EXISTING | | LOS D | MEETS |
| ROADWAY | FROM | то | DISTRIBUTION | TRIPS | YEAR | VOLUMES | GROWTH | TRAFFIC | TRAFFIC | VOLUME | LANES | CLASS | STANDARD | LOS? |
| SE PORT ST. LUCIE BLVD | FLORESTA DRIVE | VETERANS MEMORIAL PI | 65% | 38 | 2023 | 2361 | 245 | 3 | 20 | 2667 | 6D | C4 | 2810 | YES |
| MORNINGSIDE BOULEVARD | SITE | WESTMORELAND BOULE | 20% | 12 | 2022 | 113 | 15 | 2 | 3 | 144 | 2 | C3R | 1110 | YES |
| MORNINGSIDE BOULEVARD | WESTMORELAND BOULEVARI | D SE PORT ST. LUCIE BLVD | 5% | 3 | 2022 | 113 | 15 | 5 | 3 | 139 | 2 | C3R | 1110 | YES |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | PORT ST LUCIE BOULEVA | 70% | 41 | 2023 | 784 | 81 | 5 | 39 | 950 | 2 | C3R | 1110 | YES |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | SE PINE VALLEY STREET | 55% | 32 | 2022 | 477 | 63 | 5 | 20 | 597 | 2 | C3R | 1110 | YES |
| WESTMORELAND BOULEVARD | SE PINE VALLEY STREET | US-1 | 25% | 15 | 2022 | 477 | 63 | 5 | 20 | 579 | 2 | C3R | 1110 | YES |



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TABLE 11 ROADWAY SEGMENT ANALYSIS - PM PEAK HOUR

2027 BUILD OUT TOTAL PM PEAK HOUR PROJECT TRIF23 TOTAL PM PEAK HOUR PROJECT TRIF37
BACKGROUND GROWTH = 2.5%

| DACKGROUND GROWTH - | 2.3 /0 | | | | | | | | | | | | | |
|------------------------|------------------------|------------------------|--------------|-----------|-------|----------|------------|-------------|--------------|--------|----------|-------|----------|-------|
| | | | | PEAK HOUR | | | 2027 | RAVELLO | WESTMORELAND | 2027 | | | | |
| | | | PROJECT | PROJECT | COUNT | EXISTING | BACKGROUND | DEVELOPMENT | RIVERWALK | TOTAL | EXISTING | | LOS D | MEETS |
| ROADWAY | FROM | то | DISTRIBUTION | TRIPS | YEAR | VOLUMES | GROWTH | TRAFFIC | TRAFFIC | VOLUME | LANES | CLASS | STANDARD | LOS? |
| | | | | | | | | | | | | | | |
| MORNINGSIDE BOULEVARD | SITE | WESTMORELAND BOULE | 20% | 7 | 2022 | 113 | 15 | 1 | 4 | 140 | 2 | C3R | 1110 | YES |
| MORNINGSIDE BOULEVARD | WESTMORELAND BOULEVARD | SE PORT ST. LUCIE BLVD | 5% | 1 | 2022 | 113 | 15 | 3 | 4 | 136 | 2 | C3R | 1110 | YES |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | PORT ST LUCIE BOULEVA | 70% | 26 | 2023 | 884 | 92 | 2 | 51 | 1056 | 2 | C3R | 1110 | YES |
| | | | | | | | | 3 | 31 | | 2 | | | |
| WESTMORELAND BOULEVARD | MORNINGSIDE BOULEVARD | SE PINE VALLEY STREET | 55% | 20 | 2022 | 522 | 69 | 3 | 26 | 640 | 2 | C3R | 1110 | YES |
| WESTMORELAND BOULEVARD | SE PINE VALLEY STREET | US-1 | 25% | 9 | 2022 | 522 | 69 | 3 | 26 | 629 | 2 | C3R | 1110 | YES |





ST. LUCIE TPO DATA

Traffic Counts and Level of Service Report 2024

| | | | 2024 | Last Physical | Pk Hr | AM I | Pk Hr Pk C | Dir | РМ | Pk Hr Pk [| Dir |
|------------------|--|---------------|-----------|------------------|---------------------|--------|------------|------|--------|------------|------|
| Roadway Name | Location | STATION ID | AADT * | Count Year | Service Capacity | Volume | LOS | V/C | Volume | LOS | V/C |
| LENNARD RD | WALTON RD to S OF SAVANNA CLUB BLVD | 679 | 3,734 | 2021 | 790 | 258 | С | 0.33 | 245 | С | 0.31 |
| LYNGATE DR | VETERANS MEMORIAL PKWY to MORNINGSIDE BLVD | 306 | 10,212 | 2023 | 920 | 645 | С | 0.70 | 582 | С | 0.63 |
| LYNGATE DR | MORNINGSIDE BLVD to US 1 | 306 | 10,212 | 2023 | 920 | 645 | С | 0.70 | 582 | С | 0.63 |
| MARIPOSA AVE | LENNARD RD to HALLAHAN ST | 166 | 6,758 | 2023 | 880 | 526 | С | 0.60 | 501 | С | 0.57 |
| MCCARTY RD | WILLIAMS RD to MIDWAY RD | 680 | 368 | 2022 | 540 | 27 | С | 0.05 | 25 | С | 0.05 |
| MCCARTY RD | MIDWAY RD to OKEECHOBEE RD | 681 | 300 | 2024 | 540 | 24 | С | 0.04 | 21 | С | 0.04 |
| MCNEIL RD | OKEECHOBEE RD to KIRBY LOOP RD | 682 | 5,510 | 2023 | 790 | 345 | С | 0.44 | 336 | С | 0.43 |
| MCNEIL RD | KIRBY LOOP RD to EDWARDS RD | 682 | 5,510 | 2023 | 540 | 345 | D | 0.64 | 336 | D | 0.62 |
| MELALEUCA BLVD | LENNARD RD to GREEN RIVER PKWY | 683 | 9,600 | 2024 | 920 | 613 | С | 0.67 | 586 | С | 0.64 |
| MIDWAY RD | EAST TORINO PKWY to MILNER DR | 134 | 25,500 | 2024 | 880 | 1,275 | F | 1.45 | 1,380 | F | 1.57 |
| MIDWAY RD | MILNER DR to W OF SELVITZ RD | 134 | 25,500 | 2024 | 790 | 1,275 | F | 1.61 | 1,380 | F | 1.75 |
| MIDWAY RD | OKEECHOBEE RD to SHINN RD | 940732 | 6,743 | 2023 | 760 | 342 | С | 0.45 | 342 | С | 0.45 |
| MIDWAY RD | SHINN RD to MCCARTY RD | 940732 | 6,743 | 2023 | 630 | 342 | С | 0.54 | 342 | С | 0.54 |
| MIDWAY RD | MCCARTY RD to I-95 | 940732 | 6,743 | 2023 | 700 | 342 | С | 0.49 | 342 | С | 0.49 |
| MIDWAY RD | I-95 to GLADES CUT-OFF RD | 945140 | 21,637 | 2023 | 2,100 | 1,060 | С | 0.50 | 1,060 | С | 0.50 |
| MIDWAY RD | GLADES CUT-OFF RD to EAST TORINO PKWY | 228 | 23,000 | 2024 | 2,100 | 1,203 | С | 0.57 | 1,193 | С | 0.57 |
| MIDWAY RD | W OF SELVITZ RD to SELVITZ RD | 134 | 25,500 | 2024 | 2,100 | 1,275 | С | 0.61 | 1,380 | С | 0.66 |
| MIDWAY RD | SELVITZ RD to CHRISTENSEN RD | 132 | 22,500 | 2024 | 2,100 | 1,155 | С | 0.55 | 1,222 | С | 0.58 |
| MIDWAY RD | CHRISTENSEN RD to 25TH ST | 132 | 22,500 | 2024 | 2,100 | 1,155 | С | 0.55 | 1,222 | С | 0.58 |
| MIDWAY RD | 25TH ST to SUNRISE BLVD | 130 | 25,000 | 2024 | 2,100 | 1,943 | С | 0.93 | 1,569 | С | 0.75 |
| MIDWAY RD | SUNRISE BLVD to OLEANDER AVE | 130 | 25,000 | 2024 | 2,100 | 1,943 | С | 0.93 | 1,569 | С | 0.75 |
| MIDWAY RD | OLEANDER AVE to US 1 | 242 | 19,000 | 2024 | 2,100 | 1,050 | С | 0.50 | 972 | С | 0.46 |
| MIDWAY RD | US 1 to WALLACE ST | 940023 | 3,813 | 2023 | 790 | 189 | С | 0.24 | 189 | С | 0.24 |
| MIDWAY RD | WALLACE ST to WEATHERBEE RD | 940023 | 3,813 | 2023 | 920 | 189 | С | 0.21 | 189 | С | 0.21 |
| MIDWAY RD | WEATHERBEE RD to INDIAN RIVER DR | 940023 | 3,813 | 2023 | 630 | 189 | С | 0.30 | 189 | С | 0.30 |
| MORNINGSIDE BLVD | WESTMORELAND BLVD to PORT ST LUCIE BLVD | 333 | 2,110 | 2022 | 920 | 113 | С | 0.12 | 113 | С | 0.12 |
| MORNINGSIDE BLVD | PORT ST LUCIE BLVD to LYNGATE DR | 331 | 3,200 | 2024 | 880 | 262 | С | 0.30 | 258 | С | 0.29 |
| NEBRASKA AVE | 25TH ST to 13TH ST | 684 | 3,437 | 2022 | 1,710 | 228 | С | 0.13 | 176 | С | 0.10 |
| OAKRIDGE DR | MOUNTWELL ST to OAKLYN ST | 621 | 6,100 | 2024 | 700 | 304 | С | 0.43 | 289 | С | 0.41 |
| OHIO AVE | SUNRISE BLVD to COLONIAL RD | 686 | 3,632 | 2022 | 540 | 192 | С | 0.36 | 212 | С | 0.39 |
| OHIO AVE | COLONIAL RD to US 1 | 686 | 3,632 | 2022 | 750 | 192 | С | 0.26 | 212 | С | 0.28 |
| OKEECHOBEE RD | OKEECHOBEE C.L. to BLUEFIELD RD | 687 | 9,900 | 2024 | 1,580 | 536 | В | 0.34 | 542 | В | 0.34 |

^{*} **NOTE:** A six digit number in the "STATION ID" column identifies segment counted by FDOT. FDOT count stations use standard K and D factors to determine peak hour values. Peak hour data is not available for locations on State roads due to differences in data availability, LOS Methodologies, and service level thresholds. Please refer to FDOT sources for detailed data on FDOT traffic counts.

^{*} Volumes shown were adjusted using FDOT Seasonal Factors

^{*} AADT = Annual Average Daily Traffic (volumes for both directions where applicable)

^{*} NOTE: If the Last Count Year is older than the year of the report, the AADT is projected from historical traffic count data.

Traffic Counts and Level of Service Report 2024

| | | | 2024 | Last Physical | Pk Hr | AM Pk Hr Pk Dir | | | РМ | Pk Hr Pk C | Dir |
|-------------------|--|---------------|-----------|------------------|---------------------|-----------------|-----|------|--------|------------|------|
| Roadway Name | Location | STATION ID | AADT * | Count Year | Service Capacity | Volume | LOS | V/C | Volume | LOS | V/C |
| VIRGINIA AVE | 35TH ST to 25TH ST | 940032 | 23,450 | 2023 | | | | | | | |
| VIRGINIA AVE | OKEECHOBEE RD to HARTMAN RD | 940030 | 22,526 | 2023 | | | | | | | |
| VIRGINIA AVE | HARTMAN RD to 35TH ST | 940030 | 22,526 | 2023 | | | | | | | |
| VIRGINIA AVE | 25TH ST to 13TH ST | 940033 | 21,782 | 2023 | | | | | | | |
| VIRGINIA AVE | 13TH ST to 11TH ST | 940794 | 23,667 | 2023 | | | | | | | |
| VIRGINIA AVE | 11TH ST to SUNRISE BLVD | 940794 | 23,667 | 2023 | | | | | | | |
| VIRGINIA AVE | SUNRISE BLVD to OLEANDER AVE | 940792 | 20,380 | 2023 | | | | | | | |
| VIRGINIA AVE | OLEANDER AVE to COLONIAL RD | 940034 | 18,402 | 2023 | | | | | | | |
| VIRGINIA AVE | COLONIAL RD to US 1 | 940034 | 18,402 | 2023 | | | | | | | |
| WALTON RD | US 1 to VILLAGE GREEN DR | 330 | 10,000 | 2024 | 1,710 | 581 | С | 0.34 | 589 | С | 0.34 |
| WALTON RD | VILLAGE GREEN DR to LENNARD RD | 328 | 17,500 | 2024 | 1,710 | 957 | D | 0.56 | 1,057 | D | 0.62 |
| WALTON RD | LENNARD RD to GREEN RIVER PKWY | 326 | 12,000 | 2024 | 880 | 747 | С | 0.85 | 757 | С | 0.86 |
| WALTON RD | GREEN RIVER PKWY to INDIAN RIVER DR | 324 | 6,014 | 2022 | 630 | 386 | С | 0.61 | 366 | С | 0.58 |
| WEATHERBEE RD | OLEANDER AVE to US 1 | 721 | 3,164 | 2023 | 750 | 198 | С | 0.26 | 180 | С | 0.24 |
| WEATHERBEE RD | US 1 to MIDWAY RD | 158 | 5,987 | 2023 | 750 | 379 | D | 0.51 | 379 | D | 0.51 |
| WESTCLIFFE LN | TREMONTE AVE to VILLAGE PKWY | 722 | 6,219 | 2023 | 1,470 | 457 | С | 0.31 | 419 | С | 0.29 |
| WESTMORELAND BLVD | MORNINGSIDE BLVD to PORT ST LUCIE BLVD | 339 | 14,645 | 2023 | 920 | 784 | С | 0.85 | 884 | D | 0.96 |
| WESTMORELAND BLVD | MARTIN C.L. to MORNINGSIDE BLVD | 245 | 9,076 | 2022 | 920 | 477 | С | 0.52 | 522 | С | 0.57 |

Countywide Performance

Weighted V/C = 64.29

% VMT below Standard = **77.98%**

^{*} **NOTE:** A six digit number in the "STATION ID" column identifies segment counted by FDOT. FDOT count stations use standard K and D factors to determine peak hour values. Peak hour data is not available for locations on State roads due to differences in data availability, LOS Methodologies, and service level thresholds. Please refer to FDOT sources for detailed data on FDOT traffic counts.

^{*} Volumes shown were adjusted using FDOT Seasonal Factors

^{*} AADT = Annual Average Daily Traffic (volumes for both directions where applicable)

^{*} NOTE: If the Last Count Year is older than the year of the report, the AADT is projected from historical traffic count data.

COUNTY: 94 STATION: 0778

DESCRIPTION: PORT ST LUCIE BLVD - E OF FLORESTA DR (COUNTY 778)

START DATE: 05/23/2023

START TIME: 0000

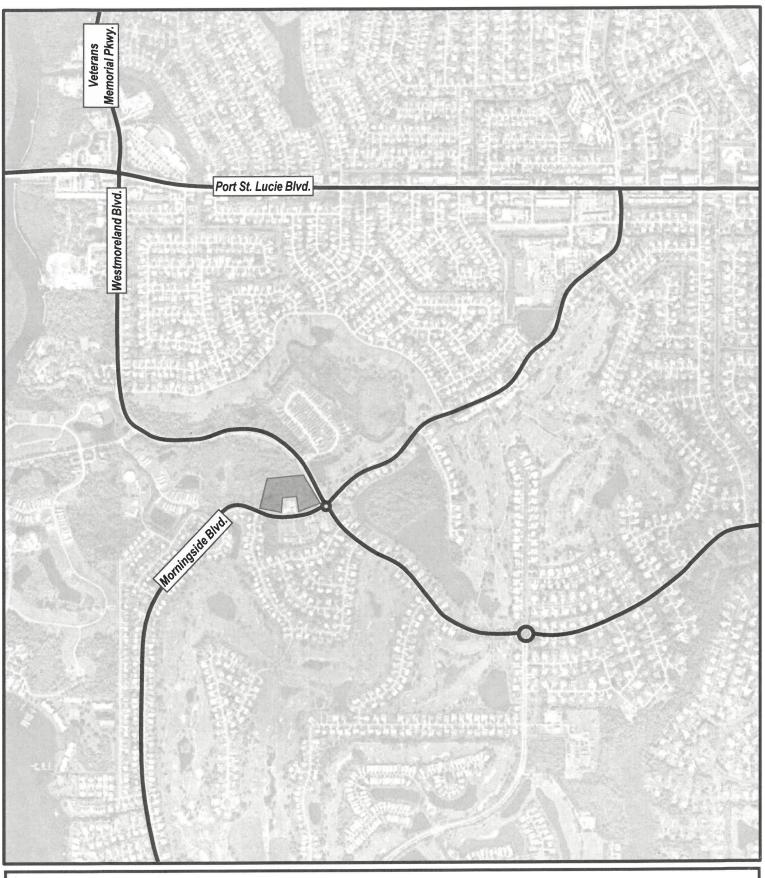
DIRECTION: E DIRECTION: W COMBINED 2ND 3RD 4TH TOTAL 1ST 2ND 3RD 4TH TOTAL TOTAL TIME 1ST28 136 | 54 18 17 15 65 28 23 98 11 16 15 49 20 16 15 79 12 22 34 45 16 62 -71 177 298 1090 578 (2361) 2.87 479 1796 436 1774 517 1772 224 1037 181 831 230 1140 147 129 635 217 189 907 112 97 113 87 409 135 111 579 53 53 55 112 99 89 418

| 24-HOUR TOTALS: | 27236 | 24579 | 51815 |
|-----------------|-------|-------|-------|
| | | | |

| | | | PEAK VOLUME | INFORMATION | | |
|---------|------------|---------|-------------|-------------|----------|------------|
| | DIRECT | TION: E | DIREC | TION: W | COMBINED | DIRECTIONS |
| | HOUR | VOLUME | HOUR | VOLUME | HOUR | VOLUME |
| A.M. | 800 | 2361 | 830 | 1208 | 745 | 3543 |
| P.M. | 1645 | 1952 | 1615 | 2231 | 1645 | 4149 |
| DAILY | 800 | 2361 | 1615 | 2231 | 1645 | 4149 |
| TRUCK I | PERCENTAGE | 4.51 | | 3.80 | | 4.17 |

| | CHASSIFICATION SUMMARI DATABASE | | | | | | | | | | | | | | | | |
|-----|---------------------------------|-------|------|----|-----|----|----|-----|----|----|----|----|----|----|----|--------|--------|
| | | | | | | | | | | | | | | | | | |
| DIR | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | TOTTRK | TOTVOL |
| E | 12 | 18483 | 7512 | 74 | 887 | 53 | 11 | 121 | 80 | 1 | 0 | 0 | 1 | 0 | 1 | 1228 | 27236 |
| W | 27 | 17907 | 5711 | 87 | 598 | 51 | 12 | 114 | 69 | 2 | 0 | 0 | 1 | 0 | 0 | 934 | 24579 |







LEGEND



Site Location

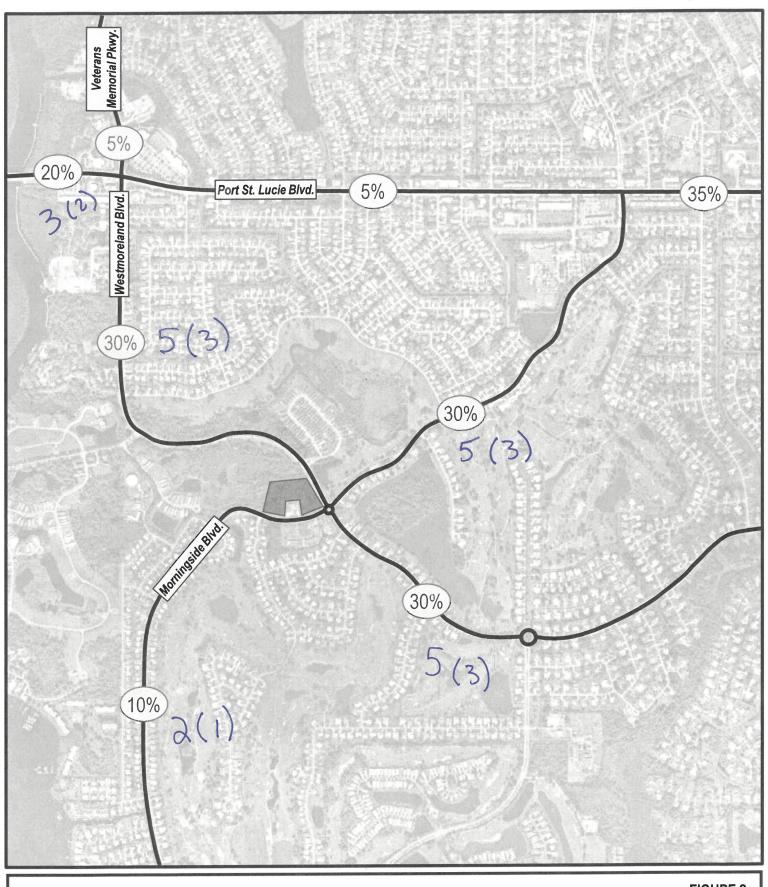
FIGURE 1
Ravello Development
Site Location

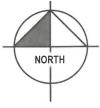
Kimley»Horn



Table 2: Trip Generation Table

| | Interesting | D.II T. | 3.3.121 | AM Peak Ho | ur | | PM Peak Hou | |
|---------------------------------|------------------------------|---------------|---|------------|--------------|------------|-------------|---------|
| Land Use | Intensity | Daily Trips | Total | In | Out | Total | In | Out |
| | Existi | ng Scenario > | 5 Years | | | | | |
| Assisted Living Facility | 150 Beds | 390 | 27 | 16 | 11 | 36 | 14 | 22 |
| | Subtotal | 390 | 27 | 16 | 11 | 36 | 14 | 22 |
| Pass-By Capture | | | | | | | | |
| Assisted Living Facility | 0.0% | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | Subtotal | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Driveway V | olumes | 390 | 27 | 16 | 11 | 36 | 14 | 22 |
| Net New Exter | | 390 | 27 | 16 | 11 | 36 | 14 | 22 |
| | Pr | oposed Scen | ario | | | | | |
| Assisted Living Facility | 75 Beds | 195 | 14 | 8 | 6 | 18 | 7 | 11 |
| Multifamily Mid-Rise | 75 DU | 341 | 28 | 6 | 22 | 29 | 18 | 11 |
| | Subtotal | 536 | 42 | 14 | 28 | 47 | 25 | 22 |
| Pass-By Capture | | | | | | | | |
| Assisted Living Facility | 0.0% | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Multifamily Mid-Rise | 0.0% | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| | Subtotal | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Driveway V | olumes | 536 | 42 | 14 | 28 | 47 | 25 | 22 |
| Net New Exter | rnal Trips | 536 | 42 | 14 | 28 | 47 | 25 | 22 |
| Proposed Net External Trips-Ext | sting Net New External Trips | 146 | 15 | -2 | 17 | 11 | (11) | 0 |
| Radius of Develop | ment Influence: | | | Direc | tly Accessed | Links | | |
| Land Use | Daily | A | M Peak Hou | r | <u>P</u> | M Peak Hou | ır | Pass By |
| Assisted Living Facility | 2.6 trips/Beds | 0.18 trips | 0.18 trips/Beds (60% in, 40% out) 0.24 trips/Beds (39% in, 61% out) | | | 61% out) | 0.0% | |
| Multifamily Mid-Rise | 4.54 trips/DU | 0.37 trip | 0.37 trips/DU (23% in, 77% out) 0.39 trips/DU (61% in, 39% | | | 9% out) | 0.0% | |





LEGEND



Site Location



Traffic Assignment

FIGURE 2

Ravello Development

Project Distribution



City of Port St. Lucie P# 17-004 A2
PSLUSD No. 5001-31 1 0 2 lucido & associates 5.14.2021 MRY Response to SAPG comm 6.9.2021 MRY Response to SAPG comm REG. # 1018 Thomas P. Lucido WESTMORELAND RIVERWALK Site Plan on St. Lucie, Florida March 17, 2021 PROJECT Date By Description 701 E. Coure Maril, Stant, Florida Salbin 100 America's Suite 24 Foot Please, Florida 2000 827 North Territon America. Oberido, Florida 2000 33 O O WESTMORELAND BLVD -Eusting Botanical Carden Sign Main - Ges Main Legal Description 9 0 Por S. Live Bld Ecology 2,430 of (1 Story) FFE 12.5 © 1 71 Spaces (68 grass) ... 9 62 Spaces (existing) Overflow Parking 83 Spaces (grass) Esseng Pascock Horse 2,111 ef (2 Story) FFE 12,5 © 9 اق اله ا D.CA.D.A METAL OR WIND TOOK PAREL DAVANCES SETAL ON FRANC W/ Z 1 Z* VISING & URDS 9.0 x 8 5 76 Spaces Edating Botan Garden Sign ALT TO JUST 1800 ALT TO Average Daily Trips (ADT)

Cly Park

-16,74 InputAces = 1.89 * 22.50 Acres

a Quelly Park

Total ADT = I TYPICAL PARKING SPACE DETAILS 18.00 18.00 119 Spaces icap spaces) 151 Spaces 0 -- PARCING SIGN (THF.)
-- HANDICUP RAMP (THF.)
-- DOUBLE 4" NICE STREETS
(2) COATS BLUE Parking Required.

Restaurant: (8,350 st/ 75 st = 111)

Hastoric Villege: (4,561 st/ 200 st = 23) Parking Requirements 40 Spaces Drainage Statement Existing Botanical Garden Paths FUTURE NESTAURANT LOCATION ++ 8,350 SF 5.00 THE PARTY OF THE P The state of the s NOTE.

NOTE: THE STATE OF THE S 167.000 Mt. (2428 etc.) 29% 14.515 wt. (1238 etc.) 5% 82.200 wt. (1308 etc.) 13% 22.007 wt. (1518 etc.) 3% 255.102 wt. (1518 etc.) 3% 255.102 wt. (1528 etc.) 95% 3.518 wt. (10.00 etc.) 7% ACCESSIBLE SIGN Proposed Floating Docks HINTS.

1, SHI FIT THE SALE OF SHILL HAVE A NOTICE POTICE OF SUR SHULL HAVE A RELECTORIZE THEE BACKGROUN WEN BLACK DYAZE LEGIO AND BORDLY. Kayak and Canoe Dock Existing — Boardwalk Proposed -Stage Over Water Existing Floating Docks Seating INTERIOR OF THE PARTY NORTH FORK ST LUCIE RIVER

301- 20(26)



APPENDIX D

2023 FDOT Q/LOS TABLES



C3C & C3R

Motor Vehicle Arterial Generalized Service Volume Tables

Peak Hour Directional

Peak Hour Two-Way

AADT



| | В | С | D | Е |
|--------|---|-------|-------|----|
| 1 Lane | * | 760 | 1,070 | ** |
| 2 Lane | * | 1,520 | 1,810 | ** |
| 3 Lane | * | 2,360 | 2,680 | ** |
| 4 Lane | * | 3,170 | 3,180 | ** |

| В | С | D | Е |
|---|-------|-------------------------------|---|
| * | 1,380 | 1,950 | ** |
| * | 2,760 | 3,290 | ** |
| * | 4,290 | 4,870 | ** |
| * | 5,760 | 5,780 | ** |
| | * * | * 1,380 * 2,760 * 4,290 | * 1,380 1,950 * 2,760 3,290 * 4,290 4,870 |

| | В | С | D | E |
|--------|---|--------|--------|----|
| 2 Lane | * | 15,300 | 21,700 | ** |
| 4 Lane | * | 30,700 | 36,600 | ** |
| 6 Lane | * | 47,700 | 54,100 | ** |
| 8 Lane | * | 64,000 | 64,200 | ** |



(C3R-Suburban Residential)

| | В | С | D | E |
|--------|---|-------|-------|----|
| 1 Lane | * | 970 | 1,110 | ** |
| 2 Lane | * | 1,700 | 1,850 | ** |
| 3 Lane | * | 2,620 | 2,730 | ** |

| | В | С | D | Е |
|--------|---|-------|-------|----|
| 2 Lane | * | 1,760 | 2,020 | ** |
| 4 Lane | * | 3,090 | 3,360 | ** |
| 6 Lane | * | 4,760 | 4,960 | ** |

| | В | С | D | Е |
|--------|---|--------|--------|----|
| 2 Lane | * | 19,600 | 22,400 | ** |
| 4 Lane | * | 34,300 | 37,300 | ** |
| 6 Lane | * | 52,900 | 55,100 | ** |

Adjustment Factors

The peak hour directional service volumes should be adjust by multiplying by 1.2 for one-way facilities. The AADT service volumes should be adjusted by multiplying 0.6 for one way facilities 2 Lane Divided Roadway with an Exclusive Left Turn Lane(s): Multiply by 1.05

2 lane Undivided Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.80

Exclusive right turn lane(s): Multiply by 1.05
Multilane Undivided Roadway with an Exclusive Left Turn Lane(s): Multiply by 0.95
Multilane Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.75
Non-State Signalized Roadway: Multiply by 0.90

This table does not constitute a standard and should be used only for general planning applications. The table should not be used for corridor or intersection design, where more refined techniques exist.

* Cannot be achieved using table input value defaults.

^{**} Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached.



C2T, C4, C5, & C6

Motor Vehicle Arterial Generalized Service Volume Tables



Peak Hour Directional

| | В | С | D | E |
|--------|---|-------|-------|----|
| 1 Lane | * | 720 | 940 | ** |
| 2 Lane | * | 1,140 | 1,640 | ** |
| 3 Lane | * | 2,120 | 2,510 | ** |

Peak Hour Two-Way

| В | С | D | E |
|---|-------|--------------------|--------------------------------|
| * | 1,310 | 1,710 | ** |
| * | 2,070 | 2,980 | ** |
| * | 3,850 | 4,560 | ** |
| | * | * 1,310 * 2,070 | * 1,310 1,710 * 2,070 2,980 |

AADT

| | В | С | D | Е |
|--------|---|--------|--------|----|
| 2 Lane | * | 13,800 | 18,000 | ** |
| 4 Lane | * | 21,800 | 31,400 | ** |
| 6 Lane | * | 40,500 | 48,000 | ** |



(C4-Urban General)

| | В | С | D | E |
|--------|---|-------|-------|-------|
| 1 Lane | * | * | 870 | 1,190 |
| 2 Lane | * | 1,210 | 1,790 | 2,020 |
| 3 Lane | * | 2,210 | 2,810 | 2,990 |
| 4 Lane | * | 2,590 | 3,310 | 3,510 |

| | В | С | D | E |
|--------|---|-------|-------|-------|
| 2 Lane | * | * | 1,580 | 2,160 |
| 4 Lane | * | 2,200 | 3,250 | 3,670 |
| 6 Lane | * | 4,020 | 5,110 | 5,440 |
| 8 Lane | * | 4,710 | 6,020 | 6,380 |

| | В | С | D | E |
|--------|---|--------|--------|--------|
| 2 Lane | * | * | 17,600 | 24,000 |
| 4 Lane | * | 24,400 | 36,100 | 40,800 |
| 6 Lane | * | 44,700 | 56,800 | 60,400 |
| 8 Lane | * | 52,300 | 66,900 | 70,900 |



(C5-Urban Center)

| | В | С | D | E |
|--------|---|-------|-------|-------|
| 1 Lane | * | * | 690 | 1,080 |
| 2 Lane | * | 1,290 | 1,900 | 2,130 |
| 3 Lane | * | 1,410 | 2,670 | 3,110 |
| 4 Lane | * | 2,910 | 3,560 | 3,640 |
| | | | | |

| | В | С | D | Е |
|--------|---|-------|-------|-------|
| 2 Lane | * | * | 1,250 | 1,960 |
| 4 Lane | * | 2,350 | 3,450 | 3,870 |
| 6 Lane | * | 2,560 | 4,850 | 5,650 |
| 8 Lane | * | 5,290 | 6,470 | 6,620 |

| | В | С | D | E |
|--------|---|--------|--------|--------|
| 2 Lane | * | * | 13,900 | 21,800 |
| 4 Lane | * | 26,100 | 38,300 | 43,000 |
| 6 Lane | * | 28,400 | 53,900 | 62,800 |
| 8 Lane | * | 58,800 | 71,900 | 73,600 |



| and the same | |
|--------------|-------|
| (C6- | Urban |
| | re) |

| | В | С | D | E |
|---------------------|---|-----|-------|-------|
| 1 Lane | * | *** | 790 | 1,030 |
| 2 Lane | * | *** | 1,490 | 1,920 |
| ² 3 Lane | * | *** | 2,730 | 2,940 |
| 4 Lane | * | *** | 3,250 | 3,490 |

| | В | С | D | Е |
|--------|---|-----|-------|-------|
| 2 Lane | * | *** | 1,440 | 1,870 |
| 4 Lane | * | *** | 2,710 | 3,490 |
| 6 Lane | * | *** | 4,960 | 5,350 |
| 8 Lane | * | *** | 5,910 | 6,350 |

| | В | С | D | E |
|--------|---|-----|--------|--------|
| 2 Lane | * | *** | 16,000 | 20,800 |
| 4 Lane | * | *** | 30,100 | 38,800 |
| 6 Lane | * | *** | 55,100 | 59,400 |
| 8 Lane | * | *** | 65,700 | 70,600 |

Adjustment Factors

The peak hour directional service volumes should be adjust by multiplying by 1.2 for one-way facilities. The AADT service volumes should be adjusted by multiplying 0.6 for one way facilities 2 Lane Divided Roadway with an Exclusive Left Turn Lane(s): Multiply by 1.05

 $2\ lane\ Undivided\ Roadway\ with\ No\ Exclusive\ Left\ Turn\ Lane(s):\ Multiply\ by\ 0.80$

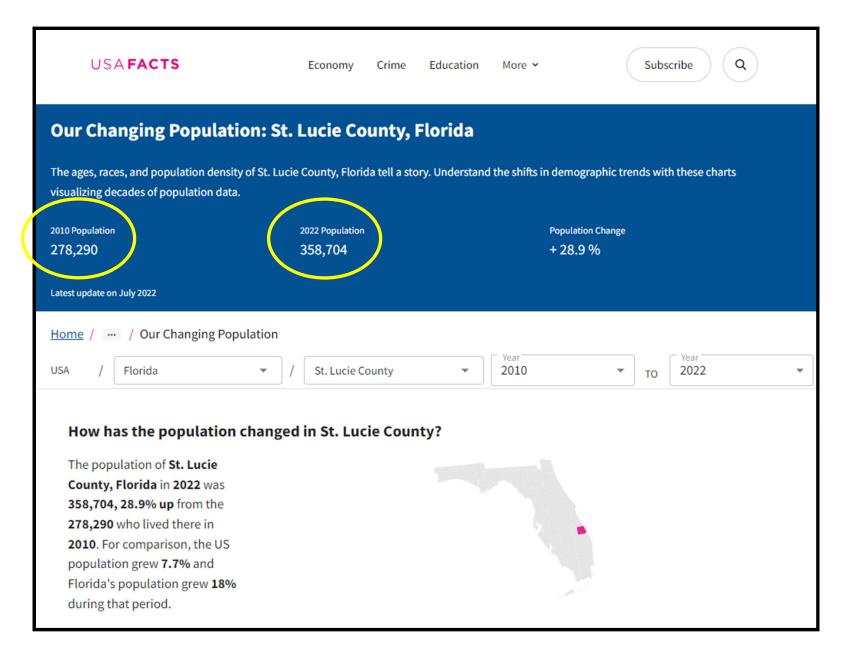
Exclusive right turn lane(s): Multiply by 1.05 Multilane Undivided Roadway with an Exclusive Left Turn Lane(s): Multiply by 0.95 Multilane Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.75 Non-State Signalized Roadway: Multiply by 0.90

This table does not constitute a standard and should be used only for general planning applications. The table should not be used for corridor or intersection design, where more refined techniques exist.

^{*}Cannot be achieved using table input value defaults. **Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached.
***LOS C thresholds are not applicable for C6 as C6 roadway facilities are neither planned nor designed to achieve automobile LOS C.



ST. LUCIE COUNTY HISTROICAL POPULATION GROWTH



12-year annual growth = 2.14%. Use 2.5% to be conservative

