



TRAFFIC IMPACT ANALYSIS

GLADES CUT-OFF ROAD INDUSTRIAL

CITY OF PORT ST. LUCIE, FL

Prepared for:

SEEFRIED INDUSTRIAL PROPERTIES

Prepared by:

KIMLEY-HORN AND ASSOCIATES, INC.

JANUARY 2022

Kimley»»Horn



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January 2022
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Date: 01/12/2022

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INTRODUCTION

Kimley-Horn has performed a traffic impact analysis (TIA) to support a warehousing facility proposed on the south side of Midway Road, east of LTC Parkway, and west of Glades Cut-Off Road in the City of Port St. Lucie, Florida. The proposed development will be comprised of two (2) buildings: Building 100 consisting of 162,000 square feet of industrial warehousing and Building 200 consisting of 192,500 square feet of industrial warehousing. Access to the site will be provided via two right-in/right-out (RIRO) access driveways and one full access driveway connection along LTC Parkway, one RIRO access driveway on Midway Road, and one full access driveway connection on Glades Cut-Off Road. Project buildout is anticipated by the year 2023. The project location is illustrated in **Figure 1** and a conceptual site plan is provided in **Appendix A**.

The TIA evaluates operational conditions under PM peak hour conditions and identifies transportation improvement needs within the study area under existing (2021), future (2023) background, and future (2023) buildout traffic conditions. Traffic from the nearby proposed Speedway convenience market and gas station development and the proposed Project Midway industrial development on LTC Parkway are included in future background and future buildout analyses. The analysis has been performed in accordance with guidelines published in the City of Port St. Lucie Land Development Code, the St. Lucie County Land Development Code, and the St. Lucie Transportation Planning Organization (TPO) Standardized Traffic Impact Studies (TIS) Methodology and Procedures document.

This traffic analysis is based on traffic data collected in the field and supplemented by information obtained from the City of Port St. Lucie and Florida Department of Transportation (FDOT) sources. The study follows procedures established in Institute of Transportation Engineers (ITE) sources, FDOT sources, and the *Highway Capacity Manual*, 6th Edition (*HCM 6*).

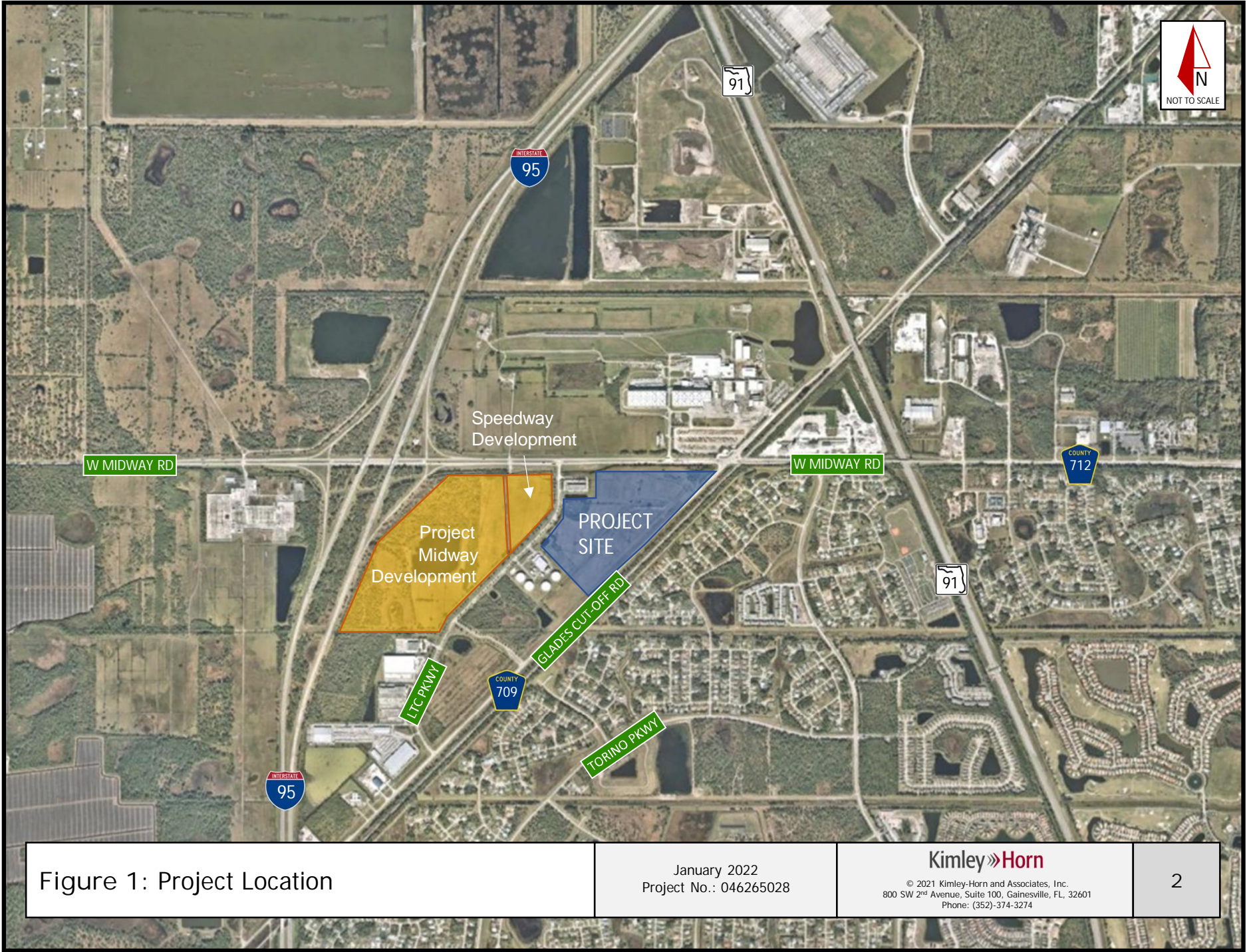


Figure 1: Project Location

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PROJECT TRIP GENERATION

Trip generation for the proposed development was calculated according to the ITE *Trip Generation Manual*, 10th Edition. Trip rates for ITE land use code 150 (Warehousing) were applied to the proposed project.

The resulting new trip calculations indicate that the proposed development is anticipated to generate 70 PM peak hour trips (19 inbound and 51 outbound) upon buildout. **Table 1** summarizes the trip generation calculations. The overall trip generation was divided between the two buildings proportionately with their respective sizes relative to the total. The trip generation potential of the proposed development was utilized to assess project impacts on the surrounding roadway network.

Table 1: Trip Generation Calculations

ITE Land Use Code	Land Use	Size	Units	Daily Trips	AM Peak			PM Peak		
					Total	In	Out	Total	In	Out
150	Warehousing	354.5	1,000 Sq Ft	606	68	52	16	70	19	51
<u>Building 100</u>		162.0	1,000 Sq Ft	277	31	24	7	32	9	23
<u>Building 200</u>		192.5	1,000 Sq Ft	329	37	28	9	38	10	28
Total External Project Trips				606	68	52	16	70	19	51
<p><i>Trip generation was calculated using data from the ITE Trip Generation Manual, 10th Edition.</i></p> <p><u>Warehousing [ITE 150]</u></p> <p>Daily T = 1.58(X) + 45.54, X is 1,000 sq. ft.</p> <p>AM Peak Hour of Adjacent Street Traffic T = 0.12(X) + 25.32, X is 1,000 sq. ft. (77% in / 23% out)</p> <p>PM Peak Hour of Adjacent Street Traffic T = 0.12(X) + 27.82, X is 1,000 sq. ft. (27% in / 73% out)</p>										

TRIP DISTRIBUTION, ASSIGNMENT, AND STUDY AREA

The project's trip distribution was developed based on output from the area's adopted transportation planning model. Land use data for the project was entered into a new traffic analysis zone (TAZ) within the Treasure Coast Regional Planning Model and situated within the existing roadway network. The model was used to assign trips between allocated origin and destination pair using a select-zone analysis. Hand adjustments to the model were applied based on engineering judgment and knowledge of the surrounding transportation network. The model output reflecting the manual adjustments is provided in **Appendix B**. **Figure 2** illustrates the anticipated project trip distribution onto the surrounding roadway network. **Figures 3 and 4** illustrate the anticipated project trips movements at each intersection.

The study area for the project includes all road segments on which the peak hour directional project traffic consumes either one percent (1%) or more of the existing or committed peak hour directional service capacity where development traffic makes its first connection to the roadway or five percent (5%) or more of the existing or committed peak hour directional service capacity. The St. Lucie County Land Development Code calls for all major roadways within two (2) miles of the proposed development to be included in the study area, as well.

Table 2 summarizes the project impact calculations. The following roadway segments are included in the study area:

- Midway Road from McCarty Road to I-95, I-95 to Glades Cut-Off Road, Glades Cut-Off Road to East Torino Parkway, East Torino Parkway to Milner Road, Milner Road to west of Selvitz Road, and west of Selvitz Road to Selvitz Road
- LTC Parkway from Glades Cut-Off Road to Midway Road
- I-95 from St. Lucie West Boulevard to Midway Road and Midway Road to Okeechobee Road
- Glades Cut-Off Road from Commerce Center Drive to Midway Road, Midway Road to Jenkins Road, and Jenkins Road to Selvitz Road
- East Torino Parkway from Cashmere Boulevard to Torino Parkway and Torino Parkway to Midway Road
- Torino Parkway from Cashmere Boulevard to California Boulevard and California Boulevard to East Torino Parkway

In addition to the above noted roadway segments, the following intersections are included within the study area:

- Midway Road & I-95 SB Ramp (signalized)
- Midway Road & I-95 NB Ramp (signalized)
- Midway Road & LTC Parkway (unsignalized)
- Midway Road & Glades Cut-Off Road (signalized)
- Midway Road & East Torino Parkway (signalized)



Figure 2: Project Trip Distribution
 Glades Cut-Off Road Industrial | Port St. Lucie, Florida

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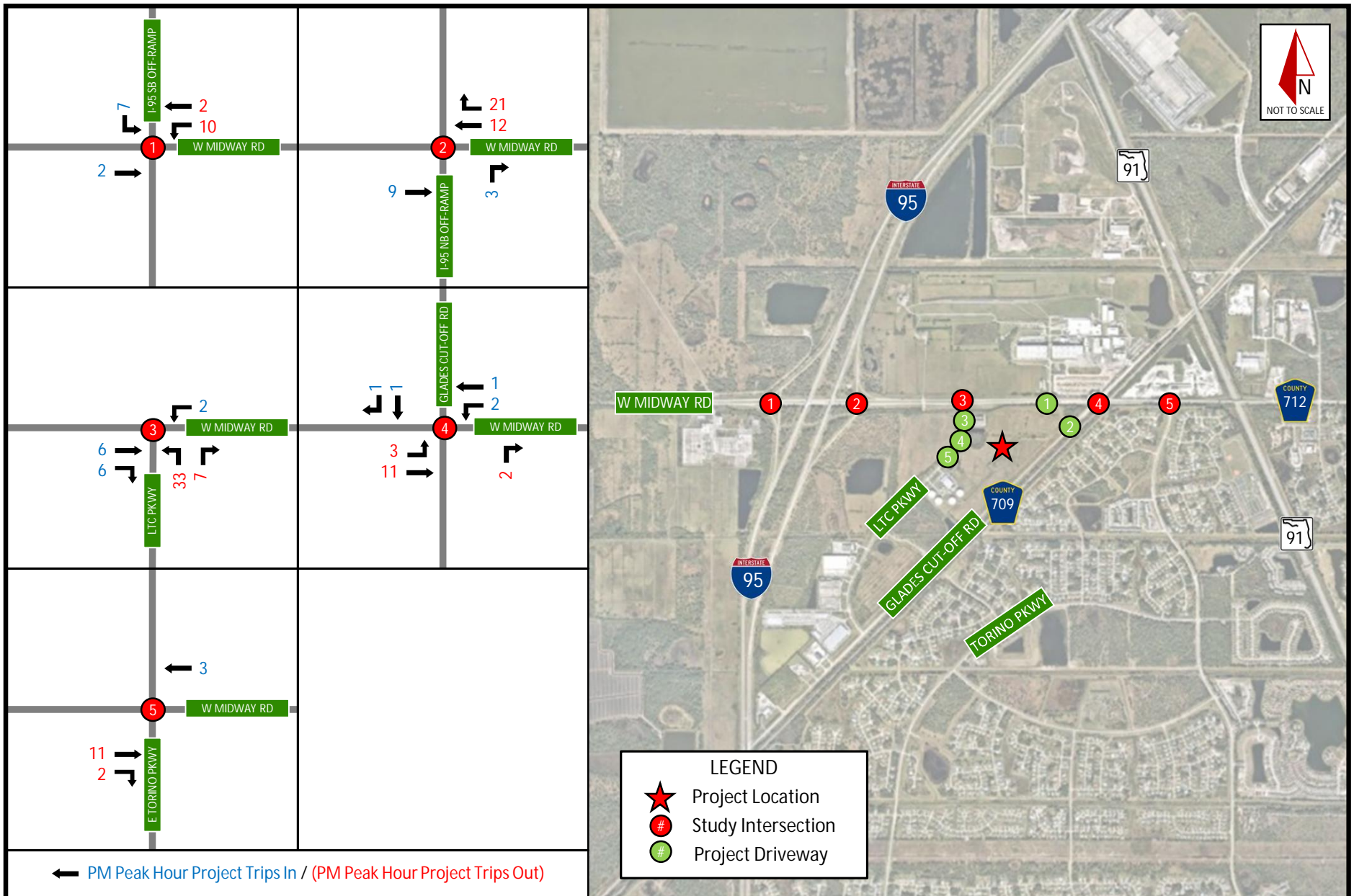


Figure 3: Project Trip Assignment
 Glades Cut-Off Industrial | Port St. Lucie, Florida

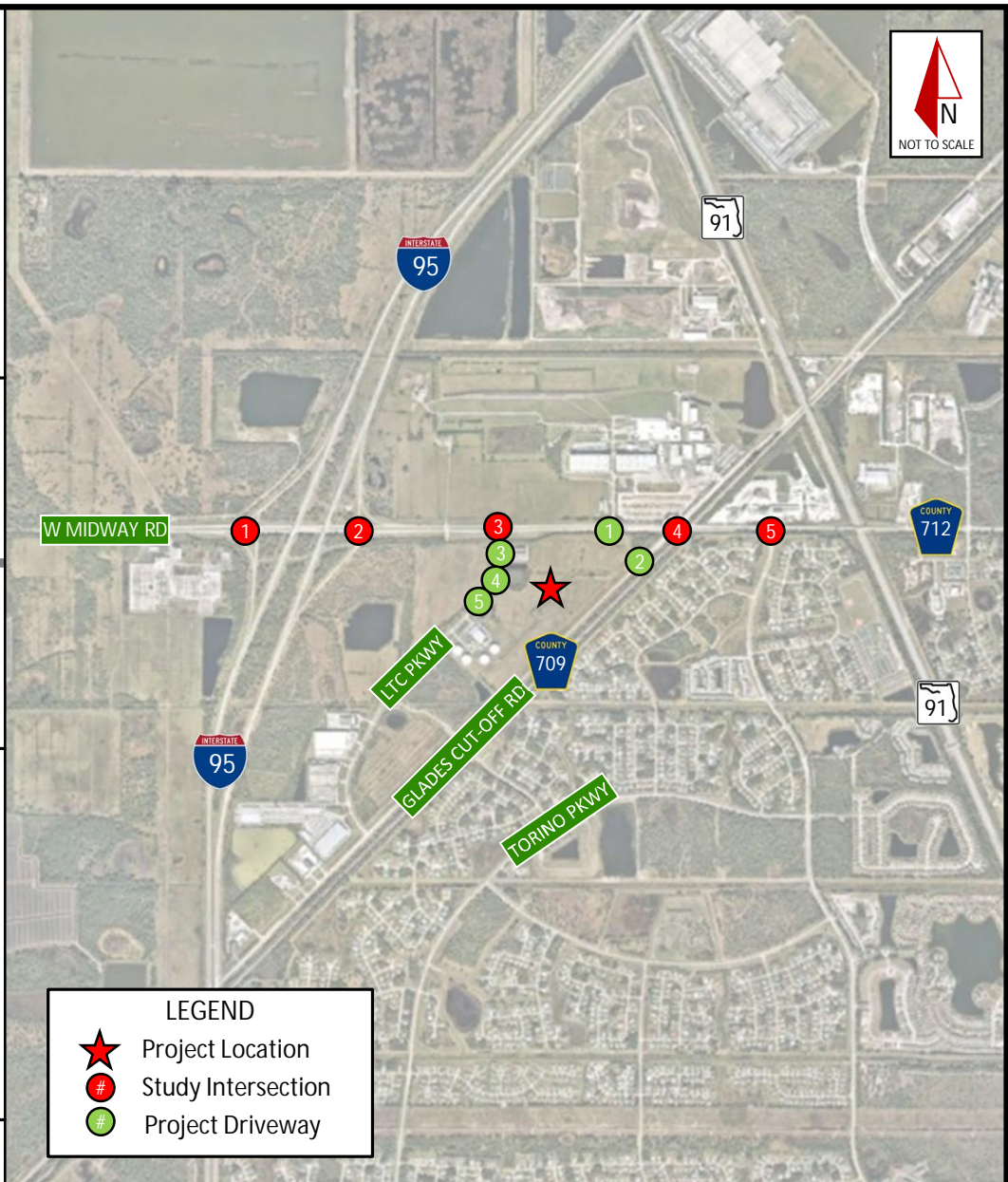
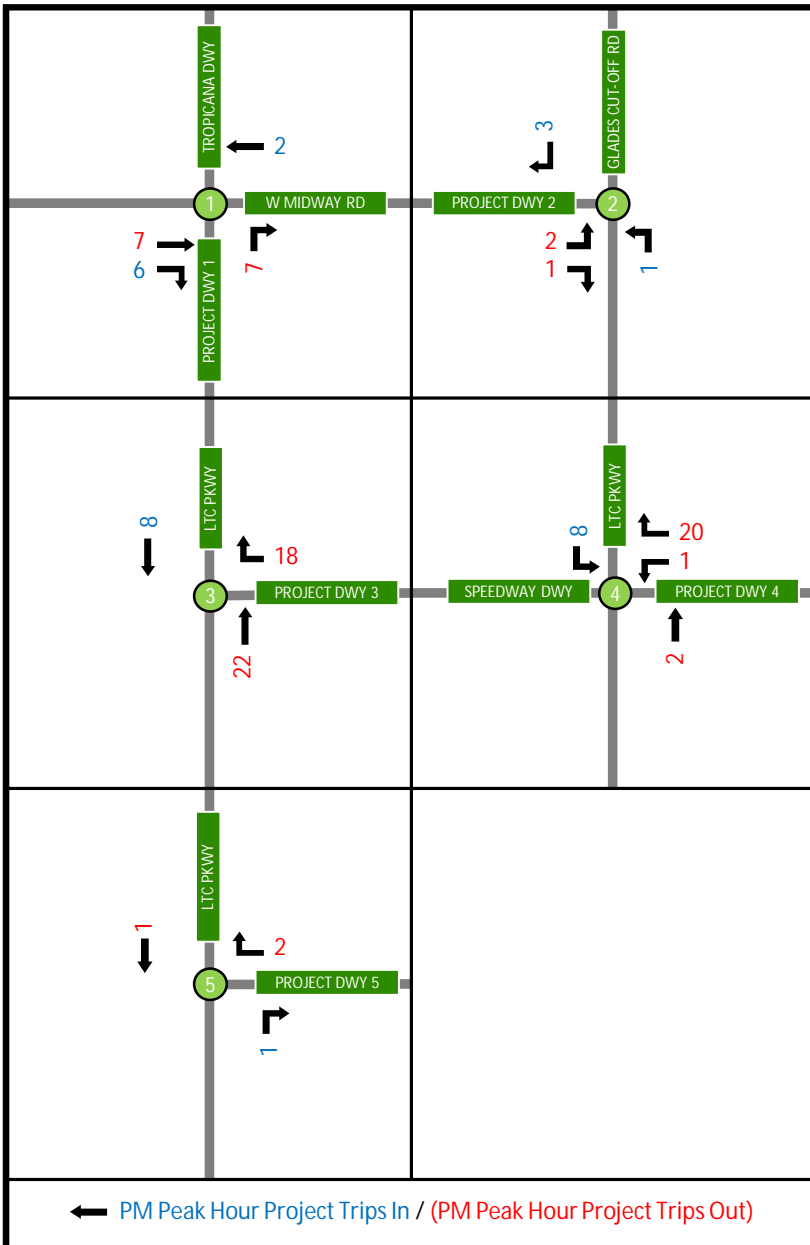
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LEGEND

- Project Location
- Study Intersection
- Project Driveway

Figure 4: Project Trip Assignment, (cont.)
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Table 2: Study Area Determination

Roadway From To		Roadway Attributes ¹					Peak Hour Directional Maximum Service Volume ²	PM Peak Hour Project Traffic				Significant Impact? ⁵	Within Two Miles?	Include In Study Area? ⁶
		Functional Classification	Jurisdiction	Area Type	Adopted LOS	Number of Lanes		% Assign ³	NB / EB	SB / WB	% Impact ⁴			
Midway Road														
McCarty Road	I-95	Principal Arterial	St. Lucie County	R	D	2U	700	2.0%	0	1	0.14%	No	Yes	Yes
I-95	Glades Cut-Off Road	Principal Arterial	St. Lucie County	U	E	4D	2,100	64.0%	12	33	1.57%	Yes	Yes	Yes
Glades Cut-Off Road	East Torino Parkway	Principal Arterial	St. Lucie County	U	E	4D	2,100	27.0%	14	5	0.67%	No	Yes	Yes
East Torino Parkway	Milner Dr	Principal Arterial	St. Lucie County	U	E	2U	880	25.0%	13	5	1.48%	No	Yes	Yes
Milner Dr	W of Selvitz Rd	Principal Arterial	St. Lucie County	U	E	2U	790	25.0%	13	5	1.65%	No	Yes	Yes
W of Selvitz Rd	Selvitz Road	Principal Arterial	St. Lucie County	U	E	2U	920	24.0%	12	5	1.30%	No	Yes	Yes
LTC Parkway														
Glades Cut-Off Road	Midway Road	Collector	Port St. Lucie	U	D	2U	680	50.0%	10	26	3.82%	Yes	Yes	Yes
I-95														
St. Lucie West Boulevard	Midway Road	Interstate	FDOT	U	D	6F	5,500	20.0%	4	10	0.18%	No	Yes	Yes
Midway Road	Okeechobee Road	Interstate	FDOT	U	D	6F	5,500	41.0%	21	8	0.38%	No	Yes	Yes
Glades Cut-Off Road														
Commerce Center Drive	Midway Road	Minor Arterial	St. Lucie County	U	E	2U	920	2.0%	0	1	0.11%	No	Yes	Yes
Midway Road	Jenkins Road	Minor Arterial	St. Lucie County	U	E	2U	790	7.0%	4	1	0.51%	No	Yes	Yes
Jenkins Road	Selvitz Road	Minor Arterial	St. Lucie County	U	E	2U	830	6.0%	3	1	0.36%	No	Yes	Yes
East Torino Parkway														
Cashmere Boulevard	Torino Parkway	Minor Arterial	Port St. Lucie	U	E	2U	830	2.0%	0	1	0.12%	No	Yes	Yes
Torino Parkway	Midway Road	Minor Arterial	Port St. Lucie	U	E	2U	880	2.0%	0	1	0.11%	No	Yes	Yes
Torino Parkway														
Cashmere Boulevard	California Boulevard	Minor Arterial	Port St. Lucie	U	E	2U	630	0.0%	0	0	0.00%	No	Yes	Yes
California Boulevard	East Torino Parkway	Major Collector	Port St. Lucie	U	D	2U	630	0.0%	0	0	0.00%	No	Yes	Yes

- Notes:
1. Roadway attributes were obtained from the latest St. Lucie County Level of Service Report (Fall 2021), FDOT Functional Classification Map, City of Port St. Lucie Transportation Element, and St. Lucie County Streets Data.
 2. Peak Hour Directional Maximum Service Volumes were obtained from the St. Lucie County Traffic Counts and Level of Service Report (Fall 2021) and supplemented by the FDOT Quality/Level of Service Handbook (2020).
 3. Percent project traffic assignment was calculated as the maximum across the segment.
 4. Percent impact was calculated as the maximum PM peak hour directional project traffic divided by the directional service volume.
 5. In accordance with St. Lucie TPO Standardized TIS Methodology and Procedures, the minimum threshold for significance was at least 1% impact where development traffic makes its first connection to the roadway network or 5% impact to the maximum roadway service volume.
 6. In accordance with St. Lucie County Land Development Code, the study area includes all significantly impacted segments and all segments within two miles of the project site.

EXISTING CONDITIONS

Existing roadway and intersection conditions were evaluated as a baseline for comparison with future background and future buildout conditions.

EXISTING ROADWAY SEGMENT ANALYSIS

The study area roadway segments were evaluated for level of service (LOS) and volume-to capacity (V/C) ratios based on the latest available traffic volume data from the St. Lucie TPO Traffic Counts and Level of Service Report (Fall 2021). For segments on which the latest available data was collected prior to 2021, a historical annual growth rate is applied to estimate existing (2021) traffic volumes.

Capacities for the study area roadways are based on the St. Lucie TPO Traffic Counts and Level of Service Report (Fall 2021) and supplemented with the latest generalized LOS capacity tables from the *FDOT Quality/Level of Service Handbook* (2020).

Table 3 summarizes the existing (2021) roadway segment analysis. The analysis found that during the PM peak hour all study area roadway segments operate within their acceptable LOS service capacities except the segments of Midway Road between East Torino Parkway and Selvitz Road.

EXISTING INTERSECTION ANALYSIS

The study area intersections were evaluated for LOS, delay, and V/C ratios utilizing *Synchro 11* software, which implements the methodologies outlined in the *HCM 6*. Turning movement volumes were collected at the study area intersections in June 2020 and September 2021. Turning movement counts are provided in **Appendix C**. Per instruction from St. Lucie County Traffic staff, a 1.23 factor was applied to the turning movement volumes collected in June 2020 to account for atypical traffic conditions related to the ongoing COVID-19 pandemic and associated stay-at-home orders. Turning movement volumes collected in September 2021 were adjusted to the peak season using the peak season conversion factor (PSCF) from the FDOT Florida Traffic Online (FTO). Seasonal factor data is included in **Appendix D**. Intersection volume development worksheets are provided in **Appendix E**. **Figure 5** illustrates the existing turning movements.

The study intersections were evaluated using existing signal timings, which were provided by St. Lucie County in April 2020. Existing signal timing data is provided in **Appendix F**. **Table 4** summarizes the existing (2021) intersection analyses for the PM peak hour conditions at the study area intersections. All six (6) study intersections operate with LOS D or better and V/C ratios less than 1.00 under existing PM peak hour conditions except the westbound left-turn movement, which has a V/C ratio of 1.04 under existing (2021) PM peak hour conditions. *Synchro* outputs are provided in **Appendix G**.

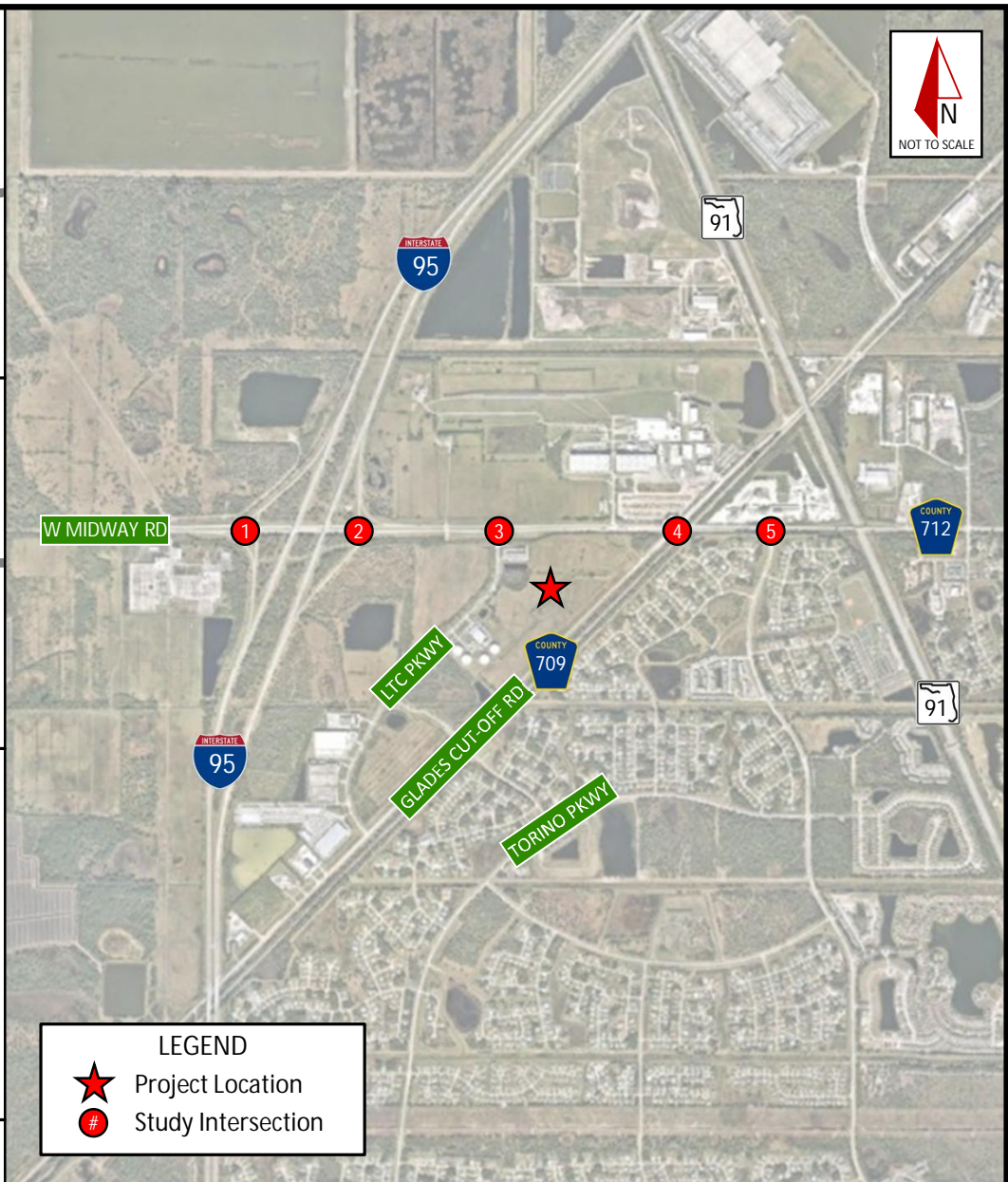
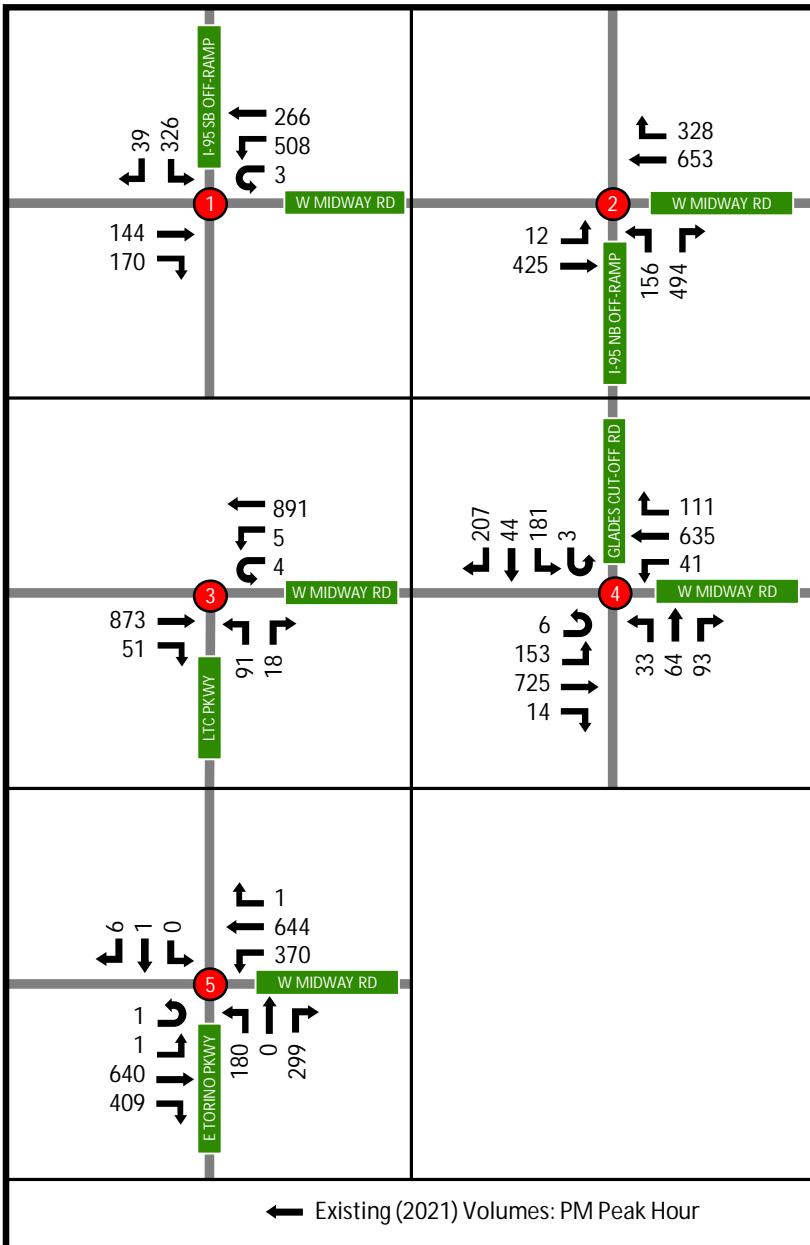


Figure 5: Existing (2021) Turning Movement Volumes
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Table 3: Existing (2021) Roadway Segment Analysis

Roadway From To		Roadway Attributes ¹					Peak Hour Directional Maximum Service Volume ²	Existing (2021) Peak Season Traffic Conditions					
		Functional Classification	Area Type	Roadway Jurisdiction	Adopted LOS	Number of Lanes		PM Peak Hour Volume Data		Applied Growth ⁴	2021 PM PHPD Volume	V/C Ratio	LOS
								PHPD Volume ³	Year				
Midway Road													
McCarty Road	I-95	Principal Arterial	R	St. Lucie County	D	2U	700	519	2020	1.05	545	0.78	C
I-95	Glades Cut-Off Road	Principal Arterial	U	St. Lucie County	E	4D	2,100	996	2020	1.05	1,046	0.50	C
Glades Cut-Off Road	East Torino Parkway	Principal Arterial	U	St. Lucie County	E	4D	2,100	1,087	2021	1.00	1,087	0.52	C
East Torino Parkway	Milner Dr	Principal Arterial	U	St. Lucie County	E	2U	880	1,053	2021	1.00	1,053	1.20	F
Milner Dr	W of Selvitz Rd	Principal Arterial	U	St. Lucie County	E	2U	790	1,053	2021	1.00	1,053	1.33	F
W of Selvitz Rd	Selvitz Road	Principal Arterial	U	St. Lucie County	E	2U	920	1,053	2021	1.00	1,053	1.14	F
LTC Parkway													
Glades Cut-Off Road	Midway Road	Collector	U	Port St. Lucie	D	2U	680	105	2020 ⁵	1.05	110	0.16	C
I-95													
St. Lucie West Boulevard	Midway Road	Interstate	U	FDOT	D	6F	5,500	3,222	2018	1.14	3,673	0.67	C
Midway Road	Okeechobee Road	Interstate	U	FDOT	D	6F	5,500	4,063	2018	1.14	4,632	0.84	D
Glades Cut-Off Road													
Commerce Center Drive	Midway Road	Minor Arterial	U	St. Lucie County	E	2U	920	171	2020	1.05	180	0.20	C
Midway Road	Jenkins Road	Minor Arterial	U	St. Lucie County	E	2U	790	540	2021	1.00	540	0.68	D
Jenkins Road	Selvitz Road	Minor Arterial	U	St. Lucie County	E	2U	830	384	2020	1.05	403	0.49	D
East Torino Parkway													
Cashmere Boulevard	Torino Parkway	Minor Arterial	U	Port St. Lucie	E	2U	830	612	2021	1.00	612	0.74	D
Torino Parkway	Midway Road	Minor Arterial	U	Port St. Lucie	E	2U	880	811	2021	1.00	811	0.92	C
Torino Parkway													
Cashmere Boulevard	California Boulevard	Minor Arterial	U	Port St. Lucie	E	2U	630	362	2021	1.00	362	0.57	D
California Boulevard	East Torino Parkway	Major Collector	U	Port St. Lucie	D	2U	630	253	2021	1.00	253	0.40	C

Notes:

- Roadway attributes were obtained from the latest St. Lucie County Level of Service Report Fall 2021, FDOT Functional Classification Map, and City of Port St. Lucie Transportation Element.
- Peak Hour Directional Maximum Service Volumes were obtained from the St. Lucie County Traffic Counts and Level of Service Report Fall 2021 and supplemented by the FDOT Quality/Level of Service Handbook (2020).
- PM Peak Hour Peak Direction traffic volumes were obtained from the St. Lucie County Traffic Counts and Level of Service Report Fall 2021.
- The growth rate applied is based on an average historical five-year growth rate of approximately 4.5% on study area roadways times the number of years since the traffic count.
- Traffic volumes on the segment of LTC Parkway from Glades Cut-Off Road to Midway Road are based on turning movement volumes collected in June 2020. Per direction from St. Lucie County staff, a 1.23 factor was applied to account for the atypical traffic conditions at the time due to the ongoing COVID-19 pandemic.

Table 4: Existing (2021) Intersection Operations

Intersection	Existing Conditions			
	Approach	LOS	Delay (sec/veh)	Max V/C
PM Peak Hour Traffic				
Midway Rd & I-95 SB On & Off Ramps	Eastbound	C	28.8	0.36 (EBT)
	Westbound	D	51.0	1.04 (WBL)
	Southbound	D	39.2	0.86 (SBL)
	Overall	D	43.3	1.04 (WBL)
Midway Rd & I-95 NB On & Off Ramps	Eastbound	A	6.3	0.25 (EBT)
	Westbound	B	13.8	0.62 (WBT)
	Northbound	C	23.5	0.72 (NBL)
	Overall	B	12.4	0.72 (NBL)
Midway Rd & LTC Pkwy ¹	Westbound Left	B	12.7	0.02 (WBL)
	Northbound	D	25.8	0.39 (NBL)
Midway Rd & Glades Cut-Off Rd	Eastbound	C	29.9	0.59 (EBT/R)
	Westbound	C	35.0	0.70 (WBT/R)
	Northbound	D	41.5	0.27 (NBT)
	Southbound	C	33.9	0.55 (SBL)
	Overall	C	33.3	0.70 (WBT/R)
Midway Rd & E Torino Pkwy	Eastbound	D	34.7	0.85 (EBT)
	Westbound	C	25.3	0.85 (WBL)
	Northbound	D	49.2	0.65 (NBL)
	Southbound	E	70.9	0.09 (SBT/R)
	Overall	C	34.9	0.85 (WBL)

1. LOS, Delay, and V/C are for yield- and stop-controlled approaches only at unsignalized intersection.

FUTURE VOLUME DEVELOPMENT

An annual growth rate was determined to apply to existing roadway and intersection volumes to calculate future year (2023) background traffic volumes.

HISTORICAL TRAFFIC GROWTH

Historical annual average daily traffic (AADT) volumes on the study area roadways were obtained from the last ten years of St. Lucie Traffic Count and Level of Service publications and from FDOT Florida Traffic Online historical AADT reports. The historical data was input into a forecast function to determine a linear growth rate based on five-year and ten-year historical data. The average five-year annual growth rate within the study area is 4.47 percent (4.47%) and the average ten-year annual growth rate is 2.85 percent (2.85%). When weighted by the latest AADT data (year 2019), the five-year average is 4.29 percent (4.29%), and the ten-year average is 2.84 percent (2.84%). The historical AADT volumes are shown in **Table 5**.

PROJECTED POPULATION GROWTH

Future population projections for St. Lucie County were reviewed as a secondary factor in determining the anticipated growth in traffic volumes within the study area. The University of Florida's Bureau of Economic and Business Research (BEBR) publishes low, medium, and high population projections for each County in Florida in five-year increments through year 2045. Based on the population projections for St. Lucie County in year 2025, BEBR estimates a low annual growth rate of 0.21 percent (0.21%), a medium annual growth rate of 1.66 percent (1.66%), and a high annual growth rate of 2.89 percent (2.89%).

APPLIED GROWTH RATE

The background annual growth rate selected for this Traffic Analysis is 3.50 percent (3.50%). The selected annual growth rate is between the historical five-year and ten-year weighted average growth rate and higher than anticipated population growth in St. Lucie County.

ADJACENT PROJECT TRIPS

In addition to annual background traffic growth, the anticipated project trips from the adjacent Speedway gas station/convenience market and Project Midway industrial development are included in the background traffic growth calculations. Excerpts from the Project Midway traffic study (which includes Speedway project trips) are provided in **Appendix H**.

Table 5: Historical Growth Rate Calculations

Roadway From	To	FDOT/TPO Count Station	Historical Annual Average Daily Traffic ¹										5-year Growth Rate ²	10-year Growth Rate ²
			2010	2011	2012	2013	2014	2015	2016	2017	2018	2019		
Midway Road														
McCarty Road	I-95	940732	4,600	4,400	4,600	4,200	4,400	4,400	4,700	5,800	5,000	8,400	6.48%	-1.73%
I-95	Glades Cut-Off Road	945140	14,300	12,400	15,500	14,200	15,900	15,900	15,200	16,500	19,100	21,000	5.46%	1.48%
Glades Cut-Off Road	East Torino Parkway	228	--	12,500	12,285	11,000	16,000	19,000	19,500	18,000	20,000	21,500	1.79%	5.50%
East Torino Parkway	Selvitz Road	948538	13,000	13,000	16,200	15,700	15,900	15,600	15,800	16,000	16,800	16,600	1.97%	2.52%
LTC Parkway														
Glades Cut-Off Road	Midway Road	N/A ³	--	--	--	--	--	--	--	--	--	--	--	--
I-95														
St. Lucie West Boulevard	Midway Road	941904	--	--	56,039	56,504	54,912	55,171	57,884	58,368	68,500	63,486	4.68%	2.41%
Midway Road	Okeechobee Road	941902	--	--	63,416	64,444	65,849	66,916	68,954	70,109	78,500	75,846	3.76%	2.71%
Glades Cut-Off Road														
Commerce Center Drive	Midway Road	940279	2,500	2,300	2,400	2,500	2,700	2,700	2,700	2,800	2,800	2,800	1.39%	2.06%
Midway Road	Jenkins Road	115	--	--	6,583	6,000	6,652	6,795	10,500	9,100	9,011	12,500	5.71%	3.99%
Jenkins Road	Selvitz Road	113	--	--	4,815	4,800	4,913	4,955	7,700	6,176	6,303	6,600	3.13%	5.19%
East Torino Parkway														
Cashmere Boulevard	Torino Parkway	710	--	7,400	7,500	8,000	6,600	6,471	6,086	12,000	11,000	11,500	11.59%	3.56%
Torino Parkway	Midway Road	237	--	9,600	9,781	9,781	11,000	12,500	11,500	13,000	13,000	14,500	2.99%	3.35%
Torino Parkway														
Cashmere Boulevard	California Boulevard	709	--	--	5,600	5,600	5,600	6,800	7,200	7,600	7,400	7,800	2.67%	4.67%
California Boulevard	East Torino Parkway	238	--	--	3,596	3,840	4,083	3,200	3,682	3,817	4,300	4,314	6.46%	1.29%

Notes:

1. Historical AADT volumes were obtained from St. Lucie County Traffic Counts and Level of Service Reports and from FDOT Florida Traffic Online.
2. 5-year and 10-year growth rates are calculated based on a linear trend forecast formula applied to the available historical AADT data.
3. Historical traffic data for LTC Parkway was not available from St. Lucie County or FDOT.
4. The weighted average is weighted by the latest available (2019) AADT volume on the roadway segments.
5. High Population Projection is from the highest of three population projections for St. Lucie County through year 2025 published by the Bureau of Economic and Business Research.

Average:	4.47%	2.85%
Weighted Average ⁴ :	4.29%	2.84%

High Population Projection ⁵ :	2.89%
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Recommended Annual Growth Rate:	3.50%
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COMMITTED IMPROVEMENTS

A review of the St. Lucie TPO 2045 Long Range Transportation Plan (LRTP), the St. Lucie TPO Transportation Improvement Program (TIP), and the FDOT 5-year work program, the following improvements are located within the study area of the development.

- Additional northbound and southbound left-turn lanes at the I-95 & Midway Road interchange (Item Number 439754-1). The improvements have construction funding and is therefore included in the future background scenario.
 - FDOT has Design and Construction funding in FY 2022
- Widening of Midway Road from a 2-lane facility to a 4-lane facility from Glades Cut-Off Road to Jenkins Road.
 - This improvement does not have construction funding and therefore is not included in the future background scenario.
- Widening of Midway Road from a 2-lane facility to a 4-lane facility from Jenkins Road to Selvitz Road (Item Number 231440-5).
 - This improvement has construction funding for FY 2026 and therefore is not included in the future background scenario.

Information pertaining to the I-95 & Midway Road interchange improvements (Item Number 439754-1) are provided within **Appendix I**.

FUTURE (2023) BACKGROUND CONDITIONS

Future roadway and intersection conditions were evaluated after applying two (2) years of 3.50 percent (3.50%) annual growth and anticipated traffic from the adjacent developments to the existing (2021) segment and intersection turning movement volumes.

FUTURE (2023) BACKGROUND ROADWAY SEGMENT ANALYSIS

Future (2023) background traffic volumes on the study area roadway segments were determined by applying the annual growth rate to the existing (2021) peak hour directional volumes and adding project trips from the adjacent Speedway development and Project Midway industrial development. Future (2023) background traffic volumes were then compared to the service capacities for each individual roadway segment based on the St. Lucie TPO Traffic Counts and Level of Service Report (Fall 2021) and supplemented with the latest generalized LOS capacity tables from the *FDOT Quality/Level of Service Handbook (2020)*.

Table 6 summarizes the future (2023) background roadway segment analysis. The analysis found that all study area roadway segments are expected to operate within their LOS service volume except for the segments of Midway Road between East Torino Parkway and Selvitz Road (which were identified under existing conditions), and the segment of East Torino Parkway between Torino Parkway and Midway Road.

Table 6: Future (2023) Background Roadway Segment Analysis

Roadway From To		Roadway Attributes ¹		Peak Hour Directional Maximum Service Volume ²	Existing (2021) PM PHPD Volume	Background Growth		Future (2023) Background Traffic Conditions		
		Adopted LOS	Number of Lanes			Annual Growth	Adjacent Development ³	Future (2023) PM PHPD Volume ⁴	V/C Ratio	LOS
Midway Road										
McCarty Road	I-95	D	2U	700	545	3.50%	7	591	0.84	C
I-95	Glades Cut-Off Road	E	4D	2,100	1,046	3.50%	339	1,460	0.70	C
Glades Cut-Off Road	East Torino Parkway	E	4D	2,100	1,087	3.50%	217	1,381	0.66	C
East Torino Parkway	Milner Dr	E	2U	880	1,053	3.50%	189	1,317	1.50	F
Milner Dr	W of Selvitz Rd	E	2U	790	1,053	3.50%	189	1,317	1.67	F
W of Selvitz Rd	Selvitz Road	E	2U	920	1,053	3.50%	189	1,317	1.43	F
LTC Parkway										
Glades Cut-Off Road	Midway Road	D	4D	1,470	110	3.50%	224	342	0.23	C
I-95										
St. Lucie West Boulevard	Midway Road	D	6F	5,500	3,673	3.50%	173	4,108	0.75	C
Midway Road	Okeechobee Road	D	6F	5,500	4,632	3.50%	173	5,135	0.93	D
Glades Cut-Off Road										
Commerce Center Drive	Midway Road	E	2U	920	180	3.50%	19	212	0.23	C
Midway Road	Jenkins Road	E	2U	790	540	3.50%	39	617	0.78	D
Jenkins Road	Selvitz Road	E	2U	830	403	3.50%	39	471	0.57	D
East Torino Parkway										
Cashmere Boulevard	Torino Parkway	E	2U	830	612	3.50%	25	681	0.82	E
Torino Parkway	Midway Road	E	2U	880	811	3.50%	25	894	1.02	F
Torino Parkway										
Cashmere Boulevard	California Boulevard	E	2U	630	362	3.50%	--	388	0.62	D
California Boulevard	East Torino Parkway	D	2U	630	253	3.50%	--	271	0.43	D

Notes:

1. Roadway attributes were obtained from the latest St. Lucie County Level of Service Report Fall 2021, FDOT Functional Classification Map, and City of Port St. Lucie Transportation Element.
2. Peak Hour Directional Maximum Service Volumes were obtained from the St. Lucie County Traffic Counts and Level of Service Report Fall 2021 and supplemented by the FDOT Quality/Level of Service Handbook (2020).
3. PM Peak Hour trips from the Speedway gas station/convenience market and Project Midway industrial proposed on the adjacent property.
4. Future (2023) PM Peak Hour Peak Direction Volumes are the sum of existing conditions, two years of background growth, and estimated peak direction trips from the proposed Speedway and Project Midway developments.

FUTURE (2023) BACKGROUND INTERSECTION ANALYSIS

Future (2023) background turning movement volumes were calculated by applying the annual 3.50 percent (3.50%) growth rate to existing (2021) peak season turning movement volumes and adding estimated project trips from the proposed Speedway gas station/convenience market and the Project Midway industrial development. Future background intersection volumes are illustrated in **Figure 6** and intersection volume development worksheets are provided in **Appendix E**.

Future (2023) background conditions at the study area intersections were evaluated for LOS and V/C ratios utilizing *Synchro 11* software. **Table 7** summarizes the future (2023) background intersection analyses for PM peak hour conditions at the study area intersections. *Synchro* outputs are provided in **Appendix G**. Note that the committed improvements at the I-95 and Midway Road interchange were assumed for the background analysis and the intersection of Midway Road and LTC Parkway was analyzed as a traffic signal, since the Project Midway traffic study indicated that a traffic signal is warranted under future (2023) background conditions. LTC Parkway was evaluated as a four-lane roadway with two through lanes in each direction since the Project Midway industrial development included widening the existing two-lane roadway to a four-lane roadway.

At the intersection of Midway Road and East Torino Parkway the eastbound through movement and westbound left-turn movement are anticipated to operate with V/C ratios greater than 1.00 and the overall intersection is anticipated to operate with LOS F under future (2023) background PM peak hour conditions. In order to accommodate the future (2023) background volumes, it is assumed that a dedicated right-turn lane will be constructed, the outside lane will be converted to a through lane, and a second eastbound receiving lane will be constructed on Midway Road and East Torino Parkway. This improvement is consistent with the widening of Midway Road listed in FDOT's Five Year Work Program, funded for construction in FY 2025-2026. With the implementation of this improvement, the intersection of Midway Road and East Torino Parkway would be expected to operate with LOS D during the PM peak hour under future (2023) background conditions.

All other intersections are expected to operate with LOS D or better under future (2023) background PM peak hour conditions. **Table 7** summarizes the intersection operations for future (2023) background conditions.

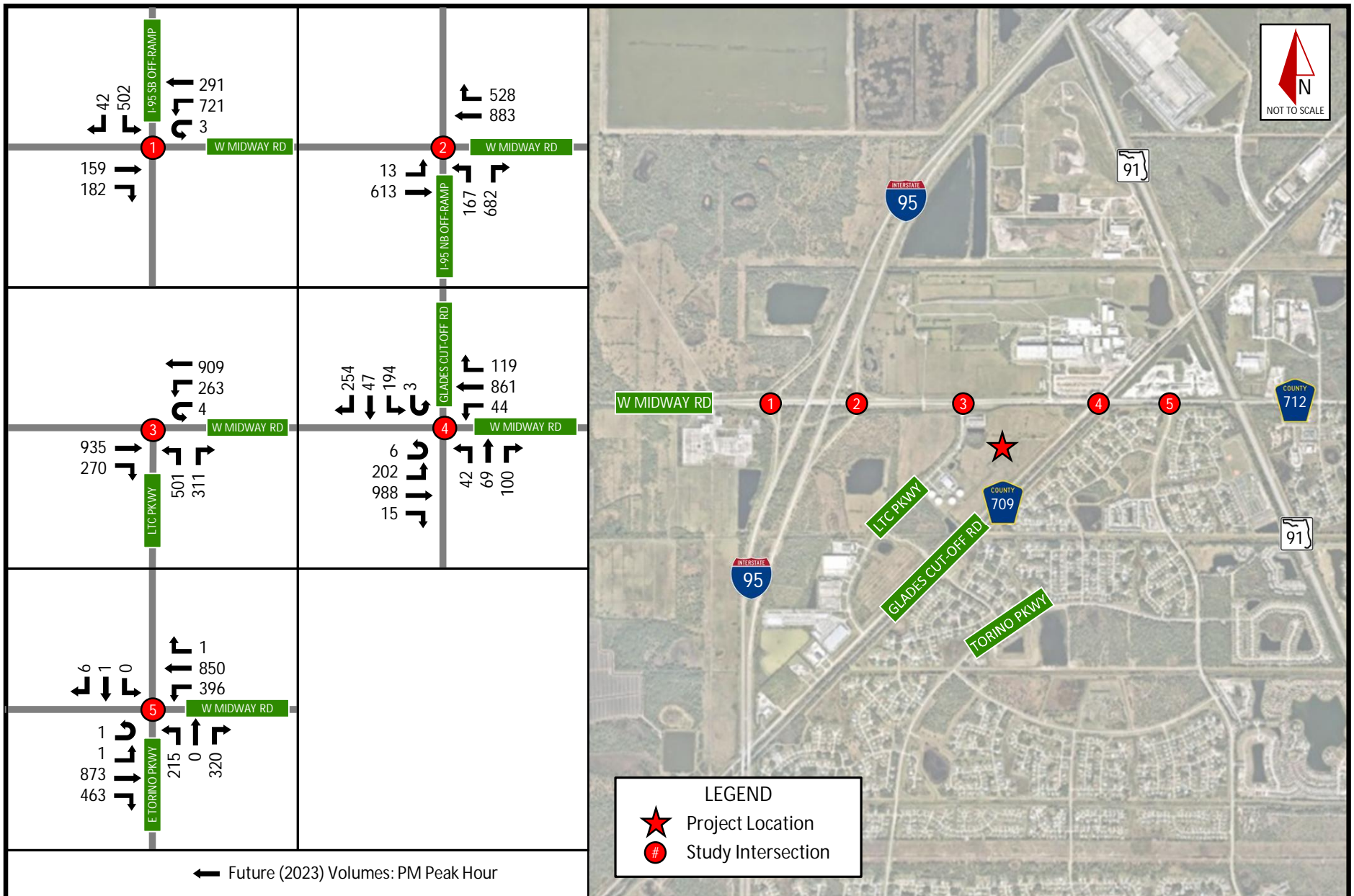


Figure 6: Future (2023) Background Turning Movement Volumes
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Table 7: Future (2023) Background Intersection Analysis

Intersection	Future Background Conditions				w/ Background Improvements			
	Approach	LOS	Delay (sec/veh)	Max V/C	Approach	LOS	Delay (sec/veh)	Max V/C
PM Peak Hour Traffic								
Midway Rd & I-95 SB On & Off Ramps	Eastbound	D	41.6	0.51 (EBT)	-	-	-	-
	Westbound	C	33.0	0.94 (WBL)	-	-	-	-
	Southbound	E	66.1	0.91 (SBL)	-	-	-	-
	Overall	D	44.0	0.94 (WBL)	-	-	-	-
Midway Rd & I-95 NB On & Off Ramps	Eastbound	A	6.1	0.33 (EBT)	-	-	-	-
	Westbound	B	13.9	0.69 (WBT)	-	-	-	-
	Northbound	C	21.8	0.42 (NBL)	-	-	-	-
	Overall	B	11.8	0.69 (WBT)	-	-	-	-
Midway Rd & LTC Pkwy	Eastbound	C	24.1	0.74 (EBT)	-	-	-	-
	Westbound	B	12.6	0.76 (WBL)	-	-	-	-
	Northbound	C	32.4	0.68 (NBL)	-	-	-	-
	Overall	C	22.0	0.76 (WBL)	-	-	-	-
Midway Rd & Glades Cut-Off Rd	Eastbound	C	33.9	0.73 (EBT/R)	-	-	-	-
	Westbound	D	35.8	0.78 (WBT)	-	-	-	-
	Northbound	D	47.9	0.34 (NBT)	-	-	-	-
	Southbound	D	43.3	0.67 (SBL)	-	-	-	-
	Overall	D	37.2	0.78 (WBT)	-	-	-	-
Midway Rd & E Torino Pkwy	Eastbound	F	97.2	1.17 (EBT)	Eastbound	C	33.4	0.72 (EBT)
	Westbound	D	46.2	1.07 (WBL)	Westbound	C	20.6	0.79 (WBL)
	Northbound	D	49.8	0.70 (NBL)	Northbound	D	46.1	0.75 (NBL)
	Southbound	E	72.4	0.09 (SBT/R)	Southbound	E	63.5	0.09 (SBT/R)
	Overall	E	68.7	1.17 (EBT)	Overall	C	30.6	0.79 (WBL)

FUTURE (2023) BUILDOUT CONDITIONS

Future (2023) buildout roadway and intersection conditions were evaluated after adding estimated project trips to the future (2023) background segment and intersection turning movement volumes.

FUTURE (2023) BUILDOUT ROADWAY SEGMENT ANALYSIS

Future (2023) buildout traffic volumes on the study area roadway segments were determined by adding estimated project trips to the future (2023) background traffic volumes. Future (2023) buildout traffic volumes were then compared to the service capacities for each individual roadway segment based on the St. Lucie TPO Traffic Counts and Level of Service Report (Fall 2021) and supplemented with the latest generalized LOS capacity tables from the *FDOT Quality/Level of Service Handbook* (2020).

Table 8 summarizes the future (2023) buildout roadway segment analysis. The analysis found that all study area roadway segments are expected to operate during the PM peak hour within their acceptable LOS service capacities except the segments that are expected to exceed their service capacities under future (2023) background conditions, prior to the addition of project traffic.

Table 8: Future (2023) Buildout Roadway Segment Analysis

Roadway From To		Roadway Attributes ¹		Peak Hour Directional Maximum Service Volume ²	Future (2023) PM PHPD Volume ³	Project Trips	Future (2023) Buildout Traffic Conditions		
		Adopted LOS	Number of Lanes				Buildout PM PHPD Volume ⁴	V/C Ratio	LOS
Midway Road									
McCarty Road	I-95	D	2U	700	591	1	592	0.85	C
I-95	Glades Cut-Off Road	E	4D	2,100	1,460	33	1,493	0.71	C
Glades Cut-Off Road	East Torino Parkway	E	4D	2,100	1,381	14	1,395	0.66	C
East Torino Parkway	Milner Dr	E	2U	880	1,317	13	1,330	1.51	F
Milner Dr	W of Selvitz Rd	E	2U	790	1,317	13	1,330	1.68	F
W of Selvitz Rd	Selvitz Road	E	2U	920	1,317	12	1,329	1.44	F
LTC Parkway									
Glades Cut-Off Road	Midway Road	D	4D	1,470	342	26	368	0.25	C
I-95									
St. Lucie West Boulevard	Midway Road	D	6F	5,500	4,108	10	4,118	0.75	C
Midway Road	Okeechobee Road	D	6F	5,500	5,135	21	5,156	0.94	D
Glades Cut-Off Road									
Commerce Center Drive	Midway Road	E	2U	920	212	1	213	0.23	C
Midway Road	Jenkins Road	E	2U	790	617	4	621	0.79	D
Jenkins Road	Selvitz Road	E	2U	830	471	3	474	0.57	D
East Torino Parkway									
Cashmere Boulevard	Torino Parkway	E	2U	830	681	1	682	0.82	E
Torino Parkway	Midway Road	E	2U	880	894	1	895	1.02	F
Torino Parkway									
Cashmere Boulevard	California Boulevard	E	2U	630	388	0	388	0.62	D
California Boulevard	East Torino Parkway	D	2U	630	271	0	271	0.43	D

Notes:

1. Roadway attributes were obtained from the latest St. Lucie County Level of Service Report Fall 2021, FDOT Functional Classification Map, and City of Port St. Lucie Transportation Element.
2. Peak Hour Directional Maximum Service Volumes were obtained from the St. Lucie County Traffic Counts and Level of Service Report Fall 2021 and supplemented by the FDOT Quality/Level of Service Handbook (2020).
3. Future (2023) PM Peak Hour Peak Direction Volumes are the sum of existing conditions, two years of background growth, and estimated peak direction trips from the proposed Speedway development adjacent to the subject property and the proposed Midway Industrial development.
4. Buildout (2023) PM Peak Hour Peak Direction Volumes are the sum of future (2023) background volumes and peak direction project trips.

FUTURE (2023) BUILDOUT INTERSECTION ANALYSIS

Future (2023) buildout turning movement volumes were calculated by adding estimated project trips to future (2023) background turning movement volumes. Future buildout intersection volumes are illustrated in **Figure 7** and **Figure 8** and intersection volume development worksheets are provided in **Appendix E**.

Future (2023) buildout conditions at the study area intersections were evaluated for LOS, delay, and V/C ratios utilizing *Synchro 11* software. Future (2023) background intersection geometries were utilized for the future (2023) buildout analysis.

Table 9 summarizes the future (2023) buildout intersection analysis for the PM peak hour conditions at the study area intersections. *Synchro* outputs are provided in **Appendix G**.

Assuming the intersection improvements that are needed under future (2023) background conditions, all study area intersections are expected to operate with LOS D or better and all movements are expected to operate with V/C ratios less than 1.00 under the future (2023) buildout PM peak hour conditions.

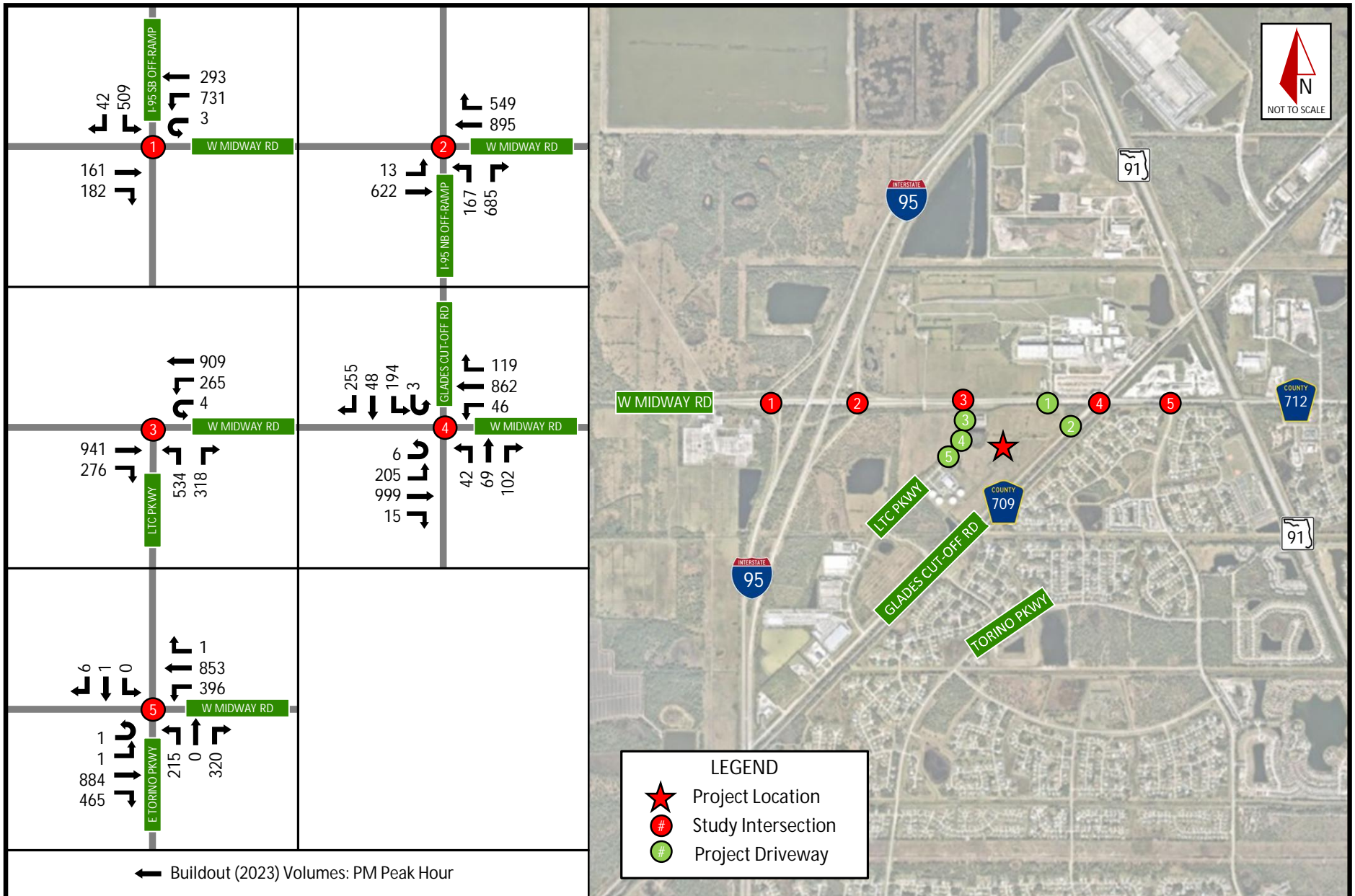


Figure 7: Future (2023) Buildout Turning Movement Volumes
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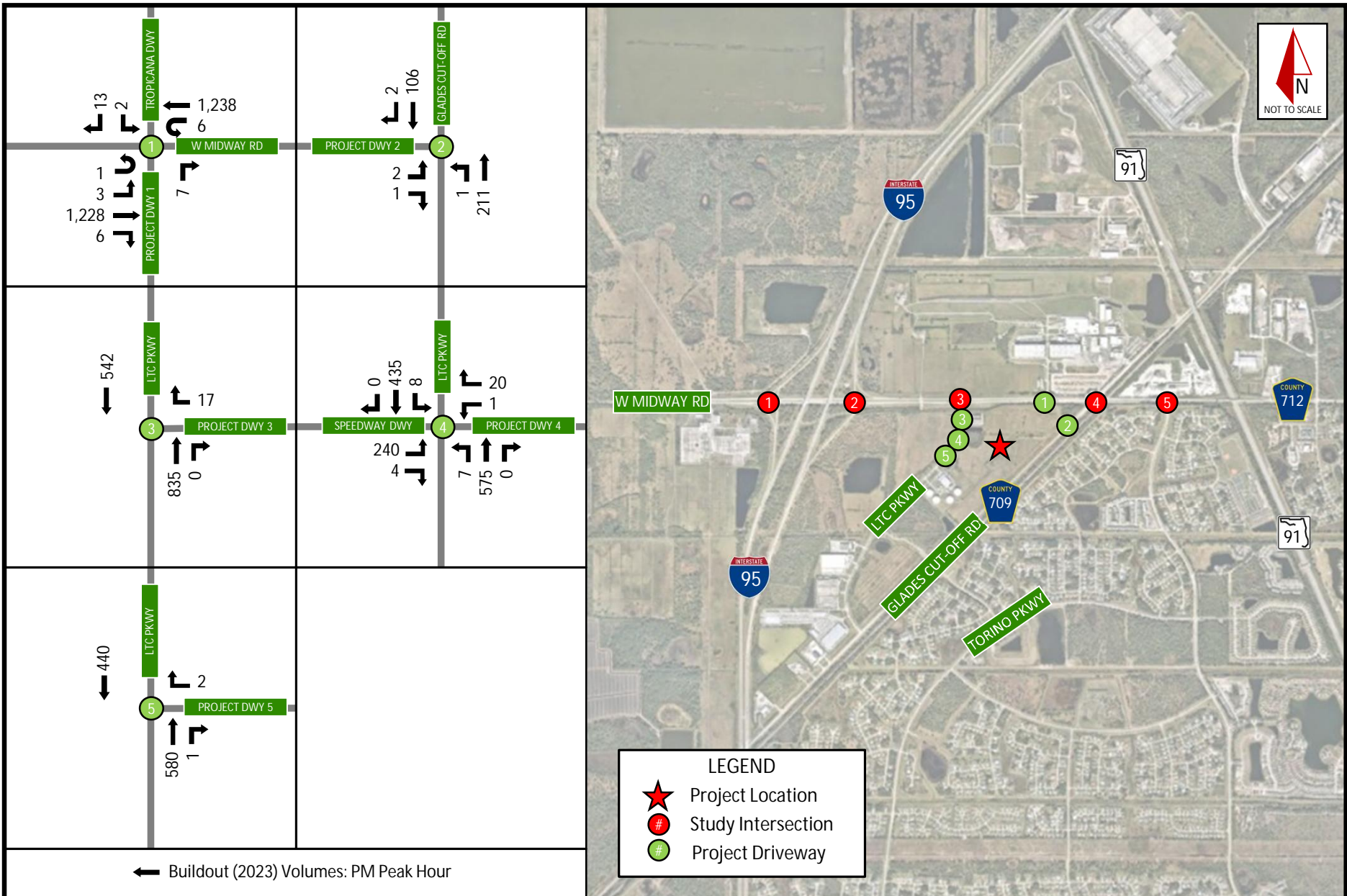


Figure 8: Future (2023) Buildout Turning Movement Volumes (cont.)
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Table 9: Future (2023) Buildout Intersection Analysis

Intersection	Buildout Conditions			
	Approach	LOS	Delay (sec/veh)	Max V/C
PM Peak Hour Traffic				
Midway Rd & I-95 SB On & Off Ramps	Eastbound	D	42.0	0.51 (EBT)
	Westbound	C	34.5	0.95 (WBL)
	Southbound	E	68.0	0.92 (SBL)
	Overall	D	45.4	0.95 (WBL)
Midway Rd & I-95 NB On & Off Ramps	Eastbound	A	6.1	0.33 (EBT)
	Westbound	B	13.9	0.70 (WBT)
	Northbound	C	22.0	0.42 (NBL)
	Overall	B	11.8	0.70 (WBT)
Midway Rd & LTC Pkwy	Eastbound	C	25.0	0.75 (EBT)
	Westbound	B	13.8	0.78 (WBL)
	Northbound	C	32.8	0.70 (NBL)
	Overall	C	23.0	0.78 (WBL)
Midway Rd & Glades Cut-Off Rd	Eastbound	C	34.1	0.73 (EBT/R)
	Westbound	D	35.8	0.78 (WBT/R)
	Northbound	D	48.1	0.34 (NBT)
	Southbound	D	43.5	0.67 (SBL)
	Overall	D	37.3	0.78 (WBT/R)
Midway Rd & E Torino Pkwy	Eastbound	C	34.6	0.72 (EBR)
	Westbound	C	20.7	0.80 (WBL)
	Northbound	D	46.9	0.76 (NBL)
	Southbound	E	64.1	0.09 (SBT/R)
	Overall	C	31.2	0.80 (WBL)

Table 10 summarizes the future (2023) buildout intersection analyses for the PM peak hour conditions at the project driveways on Midway Road, LTC Parkway, and Glades Cut-Off Road. *Synchro* outputs are provided in **Appendix G**.

All egress movements from project driveways are expected to operate with LOS B or better and all movements at the project driveways are expected to operate with V/C ratios less than 1.00 under future (2023) buildout PM peak hour conditions.

Table 10: Future (2023) Buildout Driveway Analysis

Intersection		Future Buildout Conditions		
		LOS	Delay (sec/veh)	Max V/C
PM Peak Hour Traffic				
Midway Road & Project Driveway 1 ¹	Northbound Right	B	14.2	0.02
	Southbound ²	D	31.0	0.08
Glades Cut-Off Road & Project Driveway 2 ¹	Eastbound	B	10.2	0.01
	Northbound Left	A	7.5	0.00
LTC Pkwy & Project Driveway 3 ¹	Westbound Right	B	12.2	0.04
LTC Pkwy & Project Driveway 4 ¹	Eastbound ³	E	36.8	0.73
	Westbound	B	11.4	0.04
	Southbound Left	A	9.0	0.01
LTC Pkwy & Project Driveway 5 ¹	Westbound Right	B	10.6	0.00

Notes:

1. LOS, Delay, and V/C are for yield- and stop-controlled approaches only at unsignalized intersection.
2. Southbound approach is a driveway for Tropicana.
3. Eastbound approach is a full access driveway for the adjacent Speedway gas station development.

TURN LANE ANALYSIS

Turn lanes at the study area intersections were evaluated to determine if sufficient deceleration and storage lengths are provided to accommodate future (2023) buildout traffic volumes. The total turn lane length should accommodate the minimum deceleration required in the 2020 FDOT Design Manual, Exhibit 212-1 and the expected 50th percentile queue. Additionally, existing storage lengths were compared to the expected 95th percentile queue length to ensure that the longest peak hour queues could be accommodated within the turn lanes. The summary of the queue length evaluation is provided in **Table 10**.

Based on the future (2023) buildout analysis, the westbound left-turn 95th percentile queues at the intersection of Midway Road and the I-95 southbound ramps are expected to exceed the existing turn lane length; however, the turn lane length was recommended to be lengthened to 700 feet per the Project Midway industrial development traffic study, this would also accommodate any expected queuing from the warehousing facility, therefore additional lengthening is not recommended. An excerpt from the Project Midway industrial development is provided in **Appendix H**.

The eastbound right-turn lane on Midway Road at East Torino Parkway, which is a recommended improvement under future (2023) background conditions before the addition of project traffic, is recommended to be at least 550 feet in length to accommodate future (2023) buildout traffic volumes.

The northbound right-turn 95th percentile queues on LTC Parkway at Midway Road are expected to exceed the existing northbound right-turn lane length, but with the modification of the northbound approach to feature dual northbound left-turn lanes and an exclusive right-turn lane, additional lengthening of the right-turn lane is not recommended.

The 95th percentile queues for the southbound right-turn on Glades Cut-Off Road at Midway Road, the westbound left-turn on Midway Road at East Torino Parkway, and the northbound left-turn on East Torino Parkway at Midway Road are expected to exceed their respective turn lane lengths as well. However, the queues under future (2023) background conditions are expected to exceed the turn lane capacity before the addition of project traffic, so it is not recommended that the proposed warehousing facility be required to increase the turn lane lengths to accommodate the background deficiency.

Table 11: Future (2023) Buildout Conditions Turn Lane Analysis

Intersection/Turn Lane	Existing Length (ft)	Posted Speed	Required Deceleration (Ft) ¹	50th Percentile Queue Length (ft) ²	95th Percentile Queue Length (ft) ²	Existing Turn Lane Sufficient (Y/N) ³	Turn Lane Extension (ft)
I-95 & SB Ramps Westbound Left-turn Lane ⁴	470	45	185	475	650	N	190
I-95 & NB Ramps Eastbound Left-turn Lane	410	45	185	25	25	Y	-
Midway Rd & LTC Pkwy Eastbound Right-Turn Lane	550	45	185	175	400	Y	-
Westbound Left-turn Lane	470	45	185	175	275	Y	-
Northbound Right-turn Lane ⁴	200	30	145	250	375	N	195
Midway Rd & Glades Cut-Off Rd Eastbound Left-Turn Lane	650	50	240	75	150	Y	-
Westbound Left-Turn Lane	395	50	240	50	75	Y	-
Northbound Left-Turn Lane	375	50	240	50	50	Y	-
Northbound Right-Turn Lane	350	50	240	75	150	Y	-
Southbound Left-Turn Lane	440	50	240	150	225	Y	-
Southbound Right-Turn Lane ⁴	300	50	240	275	400	N	215
Midway Rd & E Torino Pkwy Eastbound Left-Turn Lane	325	45	185	0	25	Y	-
Eastbound Right-Turn Lane	N/A ⁵	45	185	375	525	-	525
Westbound Left-Turn Lane ⁴	300	45	185	375	525	N	260
Northbound Left-Turn Lane ⁴	250	40	155	175	275	N	80

Notes:

1. Required deceleration length based on the 2020 FDOT Design Manual, Exhibit 212-1.
2. Anticipated queue lengths are based on the 50th and 95th percentile queues during the future (2023) buildout PM peak hour as reported by Synchro analysis.
3. Existing storage lengths were determined to be sufficient if the turn lane could accommodate the higher of (1) the sum of the required deceleration length and 50th percentile queue length or (2) the 95th percentile queue length.
4. Noted turn lanes have insufficient capacity to accommodate forecasted turning movement volumes under future (2023) background conditions.
5. Eastbound right-turn lane is a recommended improvement under future (2023) background conditions, before the addition of project traffic.

SITE ACCESS ANALYSIS

Access to the site is proposed via a right-in/right-out (RIRO) driveway on Midway Road, two (2) right-in/right-out driveways and one (1) full access connection on LTC Parkway, and one (1) full access connection on Glades Cut-Off Road. Per the LTC Ranch Planned Unit Development that governs access conditions along LTC Parkway, all driveways must be at least 150 feet apart along LTC Parkway. Overall, only 18 entering vehicles are anticipated during the PM peak hour at the proposed development. As such, ingress turn lanes are not warranted at any of the proposed project driveways. National Cooperative Highway Research Project (NCHRP) Report 457 worksheets are provided in **Appendix J**.

MIDWAY ROAD AT PROPOSED RIRO DRIVEWAY 1

Approximately six (6) eastbound ingress right turns are anticipated to enter the project driveway on Midway Road under PM peak hour conditions. Based on NCHRP Report 457, an ingress right-turn lane is not warranted on Midway Road.

GLADES CUT-OFF ROAD AT PROPOSED FULL ACCESS DRIVEWAY 2

Due to low ingress turning movement volumes at the proposed full-access driveway on Glades Cut-Off Road, neither a dedicated ingress right-turn nor dedicated ingress left-turn lane is recommended. However, this driveway is critical for site circulation and for ingress from the north and east. Per discussion with St. Lucie County, this driveway may ultimately provide cross-access with adjacent developments on Glades Cut-Off Road when the road is widened to a four-lane divided section.

LTC PARKWAY AT PROPOSED RIRO DRIVEWAY 3

No northbound right-turn ingress movements are expected to utilize this driveway during the PM peak hour; therefore, a dedicated ingress right-turn lane is not recommended.

LTC PARKWAY AT PROPOSED FULL ACCESS DRIVEWAY 4

Approximately eight (8) southbound ingress left turns are anticipated to enter the full-access project driveway on LTC Parkway under PM peak hour conditions. Based on NCHRP Report 457, an ingress left-turn lane is not warranted on LTC Parkway.

No northbound right-turn ingress movements are expected to utilize this driveway during the PM peak hour; therefore, a dedicated ingress right-turn lane is not recommended.

LTC PARKWAY AT PROPOSED RIRO DRIVEWAY 5

Only one (1) northbound ingress right turn is anticipated to enter the southernmost RIRO driveway on LTC Parkway under PM peak hour conditions. Based on NCHRP Report 457, an ingress right-turn lane is not recommended on LTC Parkway.

CONCLUSION

This traffic impact analysis was prepared to support a proposed warehousing facility located on the southside of Midway Road, east of LTC Parkway, and west of Glades Cut-Off Road in the City of Port St. Lucie, Florida. The proposed development will be comprised of two (2) warehouse buildings: Building 100 consisting of 162,000 square feet of warehousing and Building 200 consisting of 192,500 square feet of warehousing. The development is planned for buildout by year 2023.

The proposed warehousing facility is anticipated to generate approximately 652 daily trips, 61 AM peak hour trips (47 entering, 14 exiting), and 68 PM peak hour trips (18 entering, 50 exiting). The estimated project distribution was determined utilizing the area's adopted transportation model and hand-adjusted based on engineering judgment and knowledge of the surrounding transportation network.

In accordance with the St. Lucie TPO TIS Methodologies and Procedures document, all major roadways within two (2) miles of the proposed development were included in the study area, as well as five (5) intersections nearest the project site:

- Midway Road & I-95 SB Ramp (signalized)
- Midway Road & I-95 NB Ramp (signalized)
- Midway Road & LTC Parkway (unsignalized)
- Midway Road & Glades Cut-Off Road (signalized)
- Midway Road & East Torino Parkway (signalized)

The existing (2021) conditions analysis found three (3) segments currently have volumes exceeding their peak hour directional capacities:

- Midway Road from East Torino Parkway to Milner Road
- Midway Road from Milner Road to west of Selvitz Road
- Midway Road from west of Selvitz Road to Selvitz Road

The existing (2021) conditions intersection analysis found all study intersections to be performing at LOS D or better during the PM peak hour.

Future (2023) background traffic volumes were forecasted on study area roadway segments and at study area intersections by applying a 3.50 percent (3.50%) annual growth rate determined by comparing five-year and ten-year traffic growth to forecasted population growth in St. Lucie County. Additionally, project trips from the Speedway convenience market/gas station and the Project Midway industrial development were included in background traffic calculations.

Interchange improvements at the I-95 and Midway Road interchange were included in the future (2023) background conditions analyses, including the addition of dual left-turn lanes on both the southbound I-95 off-ramp and the northbound I-95 off-ramp. A traffic signal at Midway Road and LTC Parkway is assumed as a background improvement since it will be constructed with the Project Midway development.

The future (2023) background conditions analysis found that the only additional roadway segment within the study area anticipated to exceed its peak hour directional capacity with the inclusion of background traffic growth is the segment of East Torino Parkway from Torino Parkway to Midway Road.

The future background conditions intersection analysis found all intersections, but the intersection of Midway Road with East Torino Parkway, are expected to perform with LOS D or better with the inclusion of background traffic growth and committed improvements to the I-95 and Midway Road interchange and signalization of the intersection of Midway Road and LTC Parkway. The eastbound through movement and westbound left-turn movement on Midway Road at East Torino Parkway are expected to have a V/C ratio over 1.00. An additional eastbound through lane is assumed on Midway Road at East Torino Parkway to improve intersection operations under future (2023) background conditions before the addition of project traffic. This improvement is consistent with the widening of Midway Road listed in FDOT's Five Year Work Program, funded for construction in FY 2025-2026.

Future (2023) buildout traffic volumes were calculated by adding anticipated project traffic to the future (2023) background traffic volumes on study area roadway segments and at study area intersections.

The future (2023) buildout conditions analysis found that no additional roadway segments within the study area are anticipated to exceed their peak hour directional capacities with the inclusion of background traffic growth. The future (2023) buildout conditions intersection analysis found that all study area intersections are expected to operate at LOS D or better and all egress movements from the proposed project driveways are expected to operate at LOS B or better under PM peak hour conditions.

Existing turn lane lengths at the study area intersections were evaluated against the anticipated queue lengths under future (2023) buildout conditions. Most existing turn lane lengths provide adequate storage to accommodate future (2023) buildout traffic volumes. Turn lane lengths recommended in the previously submitted TIA for Project Midway accommodate future (2023) buildout volumes.

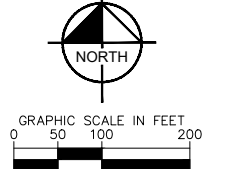
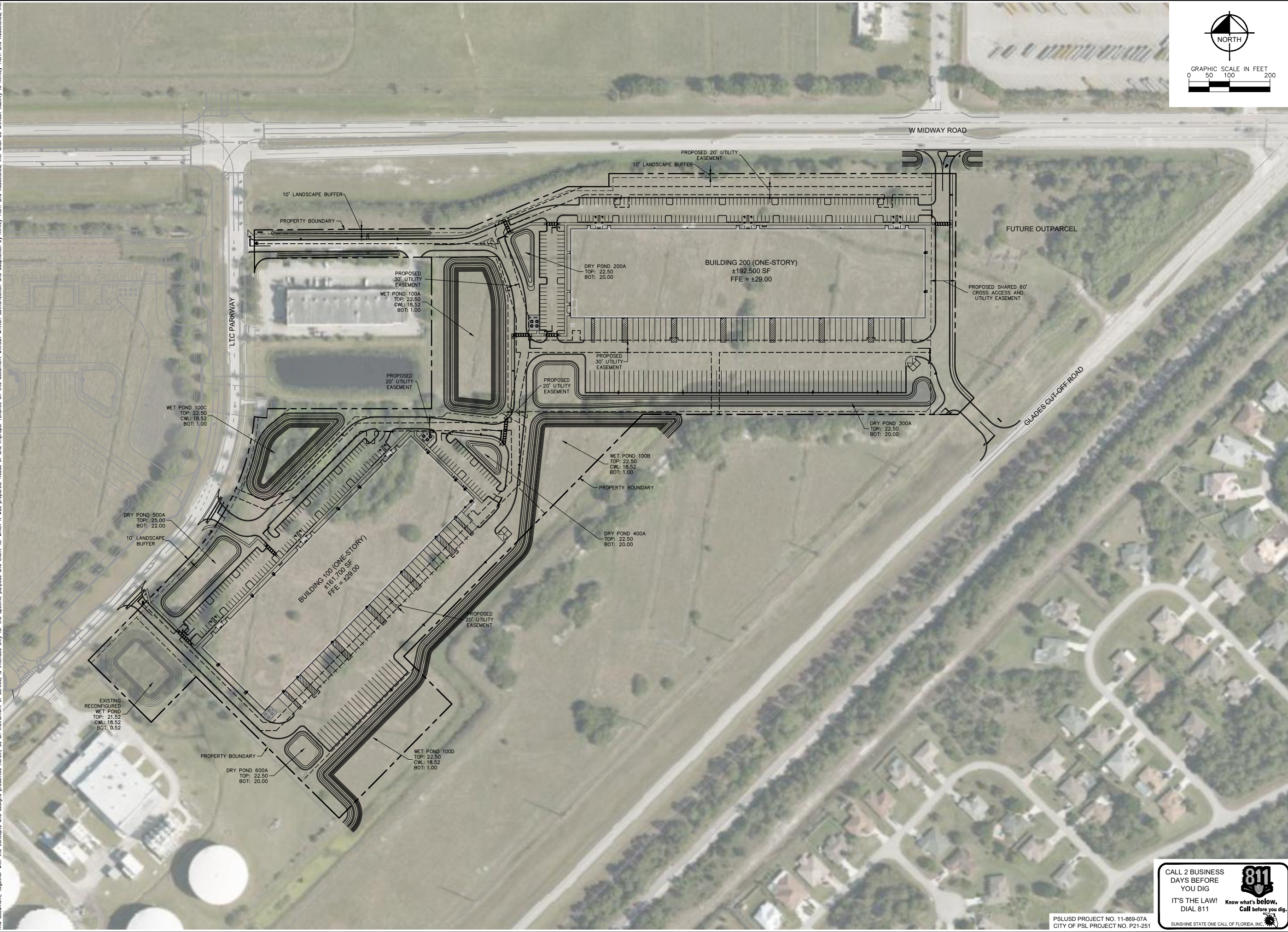
Several additional turn lanes were found to have insufficient capacity to accommodate future (2023) background traffic conditions. Since these deficiencies occur prior to the addition of project traffic, it is not recommended that the proposed warehousing facility be required to lengthen the turn lanes.

The site access analysis completed for the proposed project driveways indicates that ingress project traffic will not warrant dedicated ingress turn lanes at any of the proposed project driveways.

APPENDICES

APPENDIX A: Conceptual Site Plan

Plotted By: Haggerty, Jordan - Sheet Set: PROJECT GLADES - Layout: C300 AERIAL SITE PLAN - January 11, 2022 - 04:39:41pm - K:\BCD-Civil\46265028 - Glades Cut-Off Rd Industrial\CADD\PlanSheets\C300 AERIAL SITE PLAN.dwg
 This document, together with the concepts and designs presented herein, is an instrument of service, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance on this document without written authorization and adaptation by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc.




No.	REVISIONS	DATE	BY

Kimley»Horn
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 1615 S. CONGRESS AVE., SUITE 201,
 DELRAY BEACH, FL 33445
 PHONE: 561-330-2345 FAX: 561-963-8175
 WWW.KIMLEY-HORN.COM REGISTRY NO. 696

LICENSED PROFESSIONAL	JORDAN L. HAGGERTY, P.E.
KHA PROJECT	046265028
DATE	OCT 2021
SCALE	AS SHOWN
DESIGNED BY	JLH
DRAWN BY	CM
CHECKED BY	JLH
DATE	
FL LICENSE NUMBER	80511

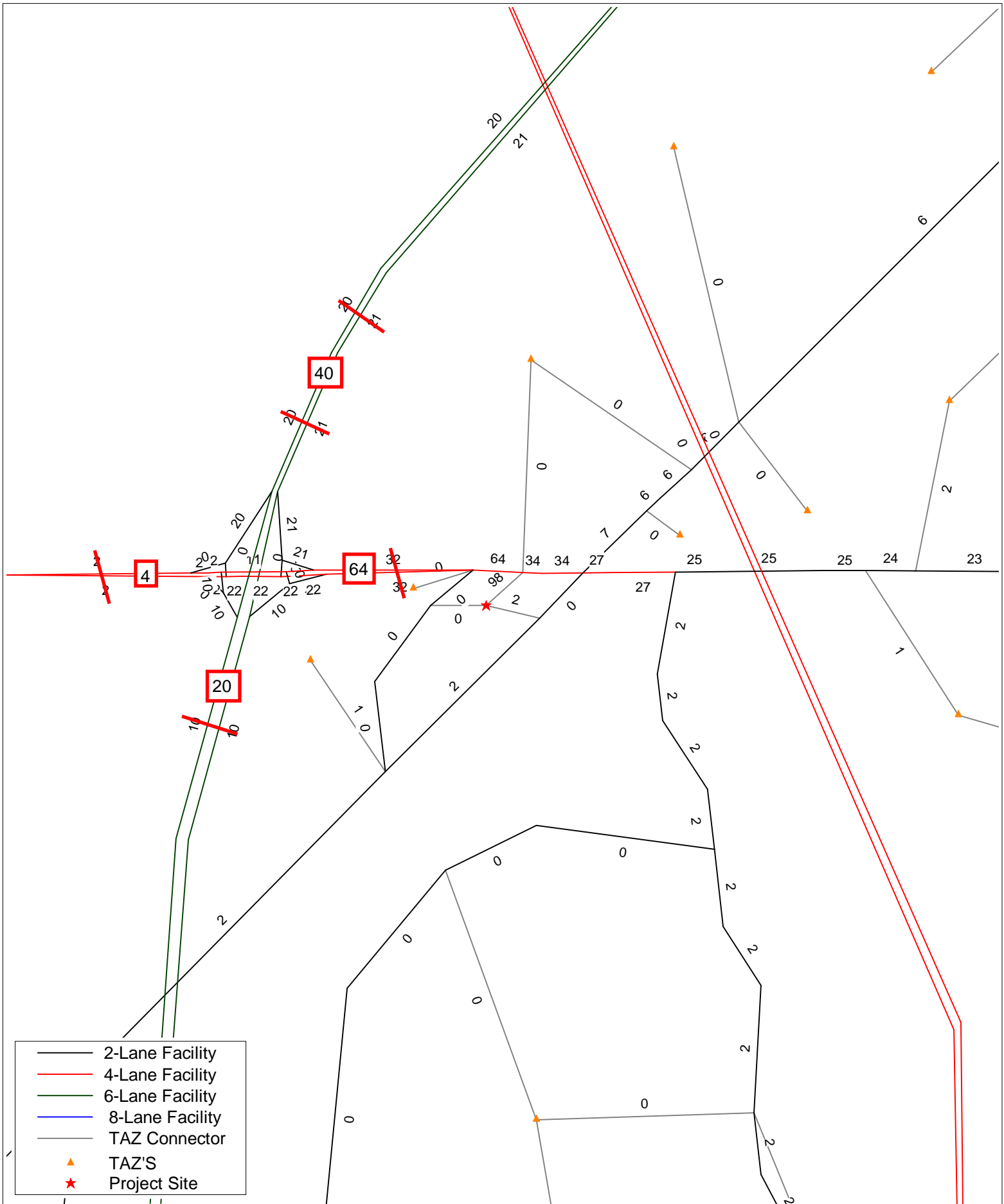
AERIAL SITE PLAN
 PROJECT GLADES
 PREPARED FOR
 SEEFRIED INDUSTRIAL
 PROPERTIES, INC.

PORT ST. LUCIE, FL
 SHEET NUMBER
C300

CALL 2 BUSINESS
 DAYS BEFORE
 YOU DIG
 IT'S THE LAW!
 DIAL 811

 Know what's below,
 Call before you dig.

PSLUSD PROJECT NO. 11-869-07A
 CITY OF PSL PROJECT NO. P21-251

**APPENDIX B:
TCRPM Model Output with Hand
Adjustments**



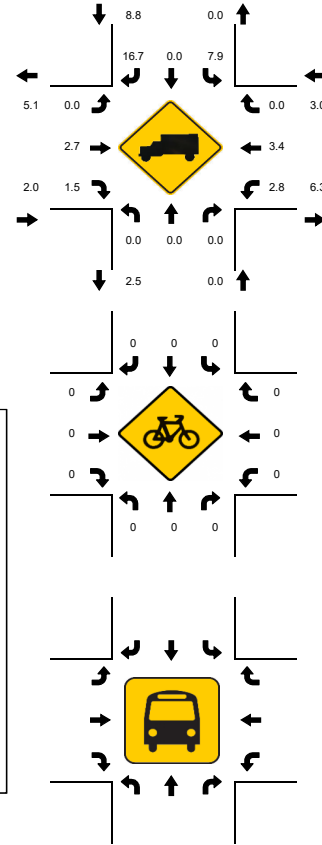
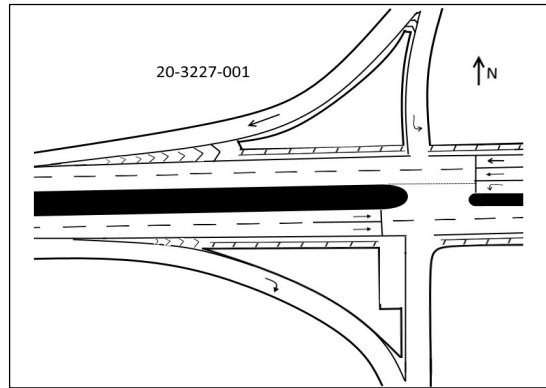
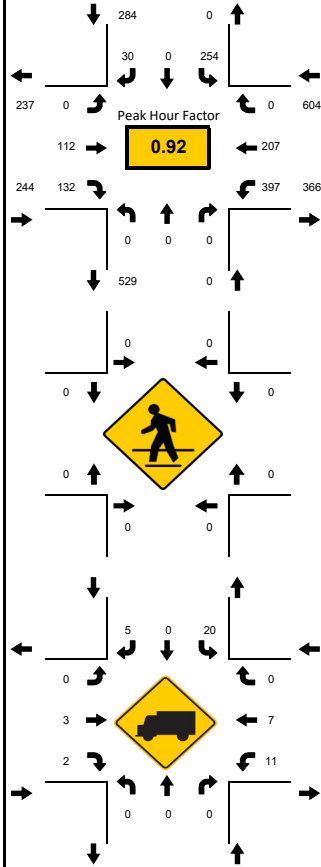
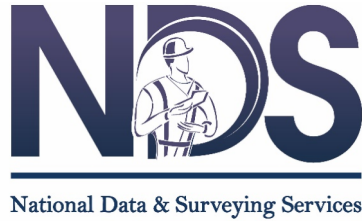
**GLADES CUT-OFF ROAD SPECIALTY WAREHOUSE
TRAFFIC DISTRIBUTION (PERCENTAGES)
10/5/2021**

APPENDIX C: Turning Movement Counts

LOCATION: I-95 SB On & Off Ramps & Midway Rd
 CITY/STATE: Port St. Lucie, FL

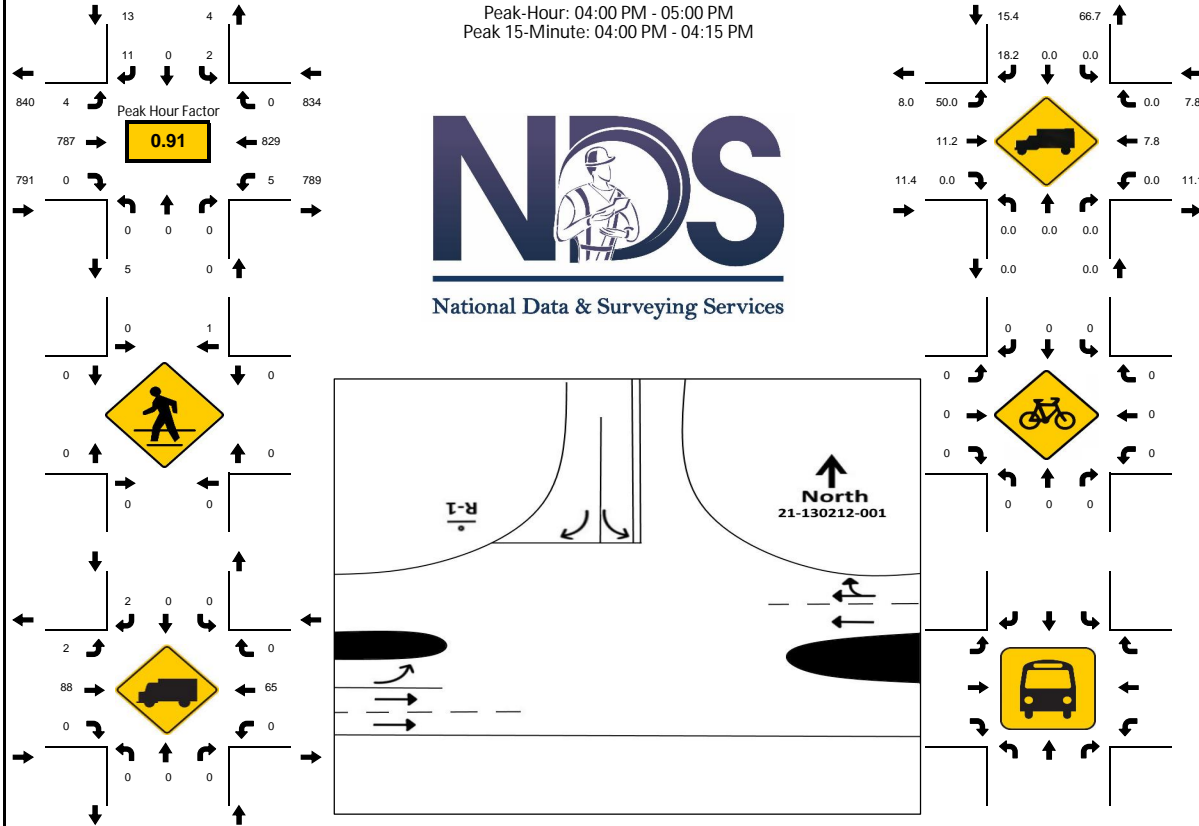
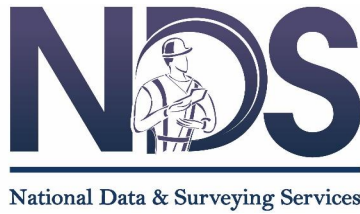
PROJECT ID: 20-03227-001
 DATE: 06/25/2020

Peak-Hour: 05:00 PM - 06:00 PM
 Peak 15-Minute: 05:30 PM - 05:45 PM



15-Min Count Period Beginning At	I-95 SB On & Off Ramps Northbound					I-95 SB On & Off Ramps Southbound					Midway Rd Eastbound					Midway Rd Westbound					Total	Hourly Total
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*		
04:00 PM	0	0	0	0	0	53	0	3	0	0	0	26	19	0	0	96	50	0	0	0	247	1011
04:15 PM	0	0	0	0	0	40	0	4	0	0	0	18	21	0	0	102	43	0	0	0	228	1057
04:30 PM	0	0	0	0	0	55	0	5	0	0	0	27	35	0	0	101	61	0	2	0	286	1104
04:45 PM	0	0	0	0	0	53	0	5	0	0	0	21	25	0	0	93	53	0	0	0	250	1125
05:00 PM	0	0	0	0	0	59	0	6	0	0	0	27	32	0	0	107	62	0	0	0	293	1132
05:15 PM	0	0	0	0	0	67	0	8	0	0	0	22	22	0	0	119	37	0	0	0	275	839
05:30 PM	0	0	0	0	0	71	0	11	0	0	0	35	37	0	0	86	65	0	2	0	307	564
05:45 PM	0	0	0	0	0	57	0	5	0	0	0	28	41	0	0	83	43	0	0	0	257	257
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*		
All Vehicles	0	0	0	0	0	284	0	44	0	0	0	140	164	0	0	476	260	0	8	0	1376	
Heavy Trucks	0	0	0	0	0	28	0	16	0	0	0	8	8	0	0	24	12	0	0	0	96	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

Peak-Hour: 04:00 PM - 05:00 PM
 Peak 15-Minute: 04:00 PM - 04:15 PM

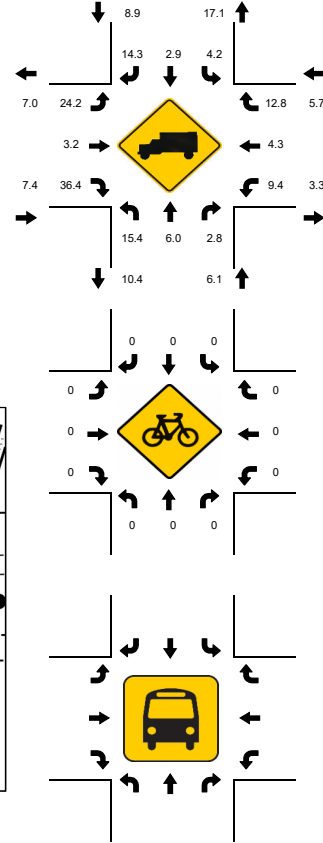
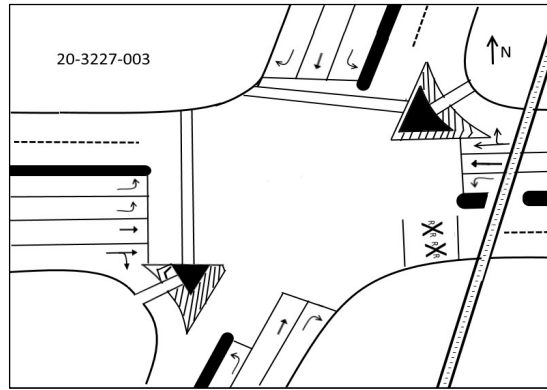
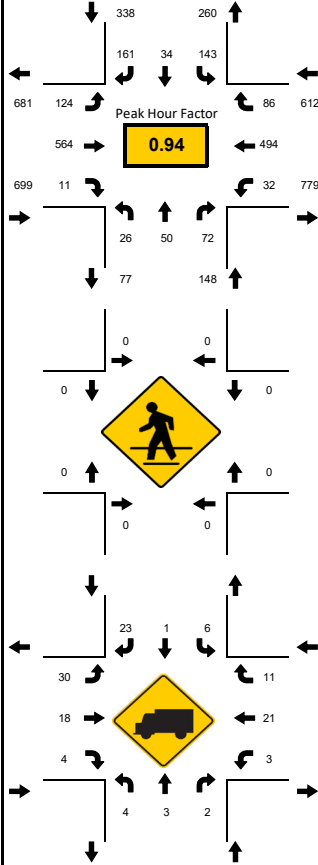
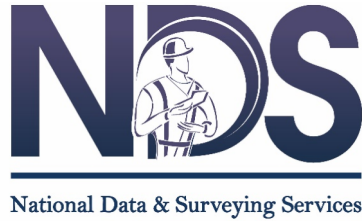


15-Min Count Period Beginning At	Tropicana Products Inc Dwy Northbound					Tropicana Products Inc Dwy Southbound					CR 712/W Midway Rd Eastbound					CR 712/W Midway Rd Westbound					Total	Hourly Total
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*		
04:00 PM	0	0	0	0	0	0	0	2	0	0	1	199	0	1	0	0	243	0	3	0	449	1638
04:15 PM	0	0	0	0	0	1	0	5	0	0	1	212	0	0	0	0	220	0	1	0	440	1611
04:30 PM	0	0	0	0	0	1	0	2	0	0	0	184	0	0	0	0	186	0	0	0	373	1576
04:45 PM	0	0	0	0	0	0	0	2	0	0	1	192	0	0	0	0	180	0	1	0	376	1568
05:00 PM	0	0	0	0	0	0	0	1	0	0	1	192	0	1	0	0	226	0	1	0	422	1531
05:15 PM	0	0	0	0	0	0	0	2	0	0	1	201	0	0	0	0	201	0	0	0	405	1109
05:30 PM	0	0	0	0	0	0	0	2	0	0	2	193	0	0	0	0	167	0	1	0	365	704
05:45 PM	0	0	0	0	0	0	0	3	0	0	0	180	0	0	0	0	154	0	2	0	339	339
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
All Vehicles	0	0	0	0	0	4	0	20	0	0	4	848	0	4	0	0	972	0	12	0	1864	
Heavy Trucks	0	0	0	0	0	0	0	8	0	0	4	100	0	0	0	0	88	0	0	0	200	
Pedestrians	0	0	0	0	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

LOCATION: Glades Cut Off Rd & Midway Rd
 CITY/STATE: Port St. Lucie, FL

PROJECT ID: 20-03227-003
 DATE: 06/25/2020

Peak-Hour: 04:00 PM - 05:00 PM
 Peak 15-Minute: 04:15 PM - 04:30 PM

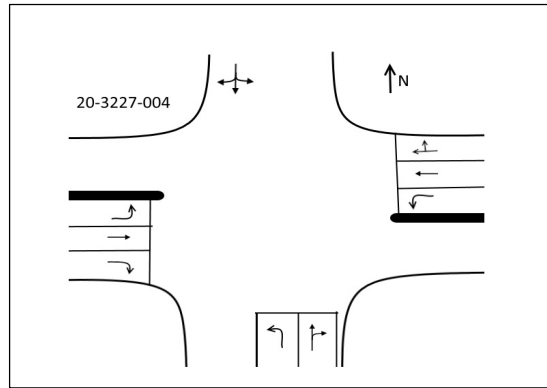
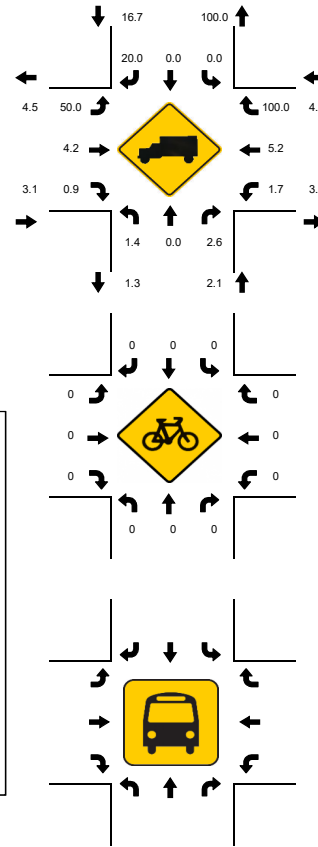
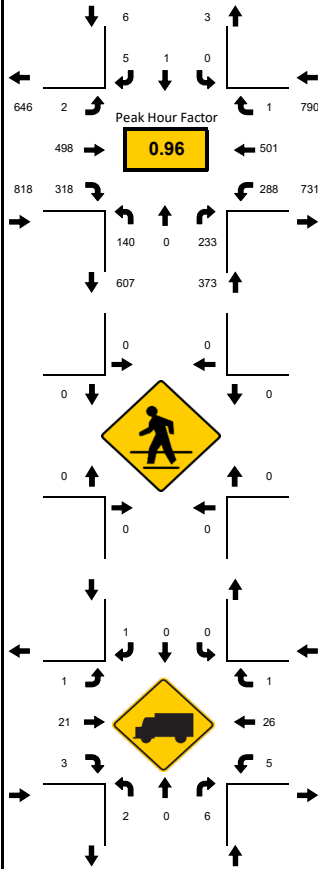
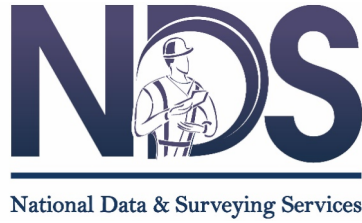


15-Min Count Period Beginning At	Glades Cut Off Rd Northbound					Glades Cut Off Rd Southbound					Midway Rd Eastbound					Midway Rd Westbound					Total	Hourly Total
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*		
04:00 PM	5	13	16	0	10	44	8	59	0	13	37	134	4	0	0	11	114	29	0	0	474	1797
04:15 PM	10	7	17	0	11	44	12	50	2	15	47	137	1	3	0	5	110	32	0	0	477	1769
04:30 PM	7	16	22	0	15	33	4	20	0	1	22	144	3	1	0	6	144	10	0	0	432	1748
04:45 PM	4	14	17	0	10	20	10	32	0	11	13	149	3	1	0	10	126	15	0	0	414	1757
05:00 PM	3	10	24	0	16	32	5	39	0	17	19	145	2	1	0	8	143	15	0	0	446	1711
05:15 PM	5	10	24	0	17	31	11	28	0	10	8	153	5	4	0	16	147	14	0	0	456	1265
05:30 PM	3	10	19	0	17	42	8	25	0	14	30	157	1	2	0	11	122	11	0	0	441	809
05:45 PM	5	10	11	0	9	33	7	15	2	7	24	129	0	2	0	11	108	11	0	0	368	368
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*		
All Vehicles	40	64	88	0	60	176	48	236	8	60	188	596	16	12	0	44	576	128	0	0	2220	
Heavy Trucks	12	4	4			16	4	32			52	24	12			8	36	16			220	
Pedestrians	0					0					0					0					0	
Bicycles	0	0	0			0	0	0			0	0	0			0	0	0			0	
Railroad																						
Stopped Buses																						

LOCATION: East Torino Pkwy & Midway Rd
 CITY/STATE: Port St. Lucie, FL

PROJECT ID: 20-03227-004
 DATE: 06/25/2020

Peak-Hour: 04:45 PM - 05:45 PM
 Peak 15-Minute: 05:15 PM - 05:30 PM



15-Min Count Period Beginning At	East Torino Pkwy Northbound					East Torino Pkwy Southbound					Midway Rd Eastbound				Midway Rd Westbound				Total	Hourly Total		
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt			U	R*
04:00 PM	33	0	39	0	9	0	0	4	0	1	0	139	64	0	33	75	122	0	0	0	476	1872
04:15 PM	35	0	55	0	18	0	0	0	0	0	1	139	57	0	29	55	103	0	0	0	445	1907
04:30 PM	40	0	62	0	15	0	2	1	0	0	1	118	70	0	43	70	120	0	0	0	484	1982
04:45 PM	36	0	57	0	7	0	0	2	0	0	0	129	63	1	30	59	120	0	0	0	467	1987
05:00 PM	37	0	63	0	12	0	1	2	0	1	1	123	85	0	38	65	134	0	0	0	511	1921
05:15 PM	38	0	62	0	12	0	0	1	0	1	0	118	85	0	40	89	127	0	0	0	520	1410
05:30 PM	29	0	51	0	15	0	0	0	0	0	0	128	85	0	33	75	120	1	0	1	489	890
05:45 PM	28	0	47	0	14	0	0	1	0	1	0	97	82	0	44	52	94	0	0	0	401	401
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound				Westbound				Total			
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt		U	R*	
All Vehicles	152	0	252	0	60	0	4	8	0	4	4	516	340	4	160	356	536	4	0	4	2176	
Heavy Trucks	8	0	12			0	0	4			4	40	12			12	40	4			136	
Pedestrians	0					0		0			0					0					0	
Bicycles	0	0	0			0	0	0			0	0	0			0	0	0			0	
Railroad																						
Stopped Buses																						

**APPENDIX D:
FDOT Florida Traffic Online (FTO) Data**

2019 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL
 CATEGORY: 9401 CEN.-W OF US1 TO I95

MOCF: 0.94

WEEK	DATES	SF	PSCF
1	01/01/2019 - 01/05/2019	1.01	1.07
2	01/06/2019 - 01/12/2019	0.99	1.05
3	01/13/2019 - 01/19/2019	0.96	1.02
* 4	01/20/2019 - 01/26/2019	0.95	1.01
* 5	01/27/2019 - 02/02/2019	0.94	1.00
* 6	02/03/2019 - 02/09/2019	0.93	0.99
* 7	02/10/2019 - 02/16/2019	0.92	0.98
* 8	02/17/2019 - 02/23/2019	0.92	0.98
* 9	02/24/2019 - 03/02/2019	0.93	0.99
*10	03/03/2019 - 03/09/2019	0.93	0.99
*11	03/10/2019 - 03/16/2019	0.93	0.99
*12	03/17/2019 - 03/23/2019	0.94	1.00
*13	03/24/2019 - 03/30/2019	0.94	1.00
*14	03/31/2019 - 04/06/2019	0.95	1.01
*15	04/07/2019 - 04/13/2019	0.95	1.01
*16	04/14/2019 - 04/20/2019	0.96	1.02
17	04/21/2019 - 04/27/2019	0.97	1.03
18	04/28/2019 - 05/04/2019	0.98	1.04
19	05/05/2019 - 05/11/2019	0.99	1.05
20	05/12/2019 - 05/18/2019	1.00	1.06
21	05/19/2019 - 05/25/2019	1.01	1.07
22	05/26/2019 - 06/01/2019	1.02	1.09
23	06/02/2019 - 06/08/2019	1.02	1.09
24	06/09/2019 - 06/15/2019	1.03	1.10
25	06/16/2019 - 06/22/2019	1.04	1.11
26	06/23/2019 - 06/29/2019	1.05	1.12
27	06/30/2019 - 07/06/2019	1.05	1.12
28	07/07/2019 - 07/13/2019	1.06	1.13
29	07/14/2019 - 07/20/2019	1.07	1.14
30	07/21/2019 - 07/27/2019	1.06	1.13
31	07/28/2019 - 08/03/2019	1.06	1.13
32	08/04/2019 - 08/10/2019	1.05	1.12
33	08/11/2019 - 08/17/2019	1.04	1.11
34	08/18/2019 - 08/24/2019	1.05	1.12
35	08/25/2019 - 08/31/2019	1.07	1.14
36	09/01/2019 - 09/07/2019	1.08	1.15
37	09/08/2019 - 09/14/2019	1.10	1.17
38	09/15/2019 - 09/21/2019	1.11	1.18
39	09/22/2019 - 09/28/2019	1.09	1.16
40	09/29/2019 - 10/05/2019	1.07	1.14
41	10/06/2019 - 10/12/2019	1.04	1.11
42	10/13/2019 - 10/19/2019	1.02	1.09
43	10/20/2019 - 10/26/2019	1.02	1.09
44	10/27/2019 - 11/02/2019	1.02	1.09
45	11/03/2019 - 11/09/2019	1.01	1.07
46	11/10/2019 - 11/16/2019	1.01	1.07
47	11/17/2019 - 11/23/2019	1.01	1.07
48	11/24/2019 - 11/30/2019	1.01	1.07
49	12/01/2019 - 12/07/2019	1.01	1.07
50	12/08/2019 - 12/14/2019	1.01	1.07
51	12/15/2019 - 12/21/2019	1.01	1.07
52	12/22/2019 - 12/28/2019	0.99	1.05
53	12/29/2019 - 12/31/2019	0.96	1.02

* PEAK SEASON

14-FEB-2020 15:39:28

830UPD

4_9401_PKSEASON.TXT

Traffic Counts and Level of Service Report 2021

Roadway Name	Location	STATION ID	AADT	Last Count Year	Pk Hr Service Capacity	AM Pk Hr Pk Dir			PM Pk Hr Pk Dir		
						Volume	LOS	V/C	Volume	LOS	V/C
COUNTRY CLUB DR	ST LUCIE WEST BLVD to CALIFORNIA BLVD	725	8,304	2019	1,710	535	C	0.313	490	C	0.287
CROSSTOWN PKWY	COMMERCE CENTER DR to I-95	650	15,500	2021	3,170	738	C	0.233	789	C	0.249
CROSSTOWN PKWY	I-95 to CALIFORNIA BLVD	651	29,500	2021	3,170	1,667	C	0.526	1,874	C	0.591
CROSSTOWN PKWY	CALIFORNIA BLVD to CASHMERE BLVD	652	30,500	2021	3,170	1,719	C	0.542	1,616	C	0.51
CROSSTOWN PKWY	CASHMERE BLVD to CAMEO BLVD	653	34,000	2021	3,170	1,840	C	0.58	1,768	C	0.558
CROSSTOWN PKWY	CAMEO BLVD to BAYSHORE BLVD	654	40,500	2021	3,170	2,014	C	0.635	2,111	C	0.666
CROSSTOWN PKWY	BAYSHORE BLVD to AIROSO BLVD	655	30,000	2021	3,170	1,560	C	0.492	1,531	C	0.483
CROSSTOWN PKWY	AIROSO BLVD to SANDIA DR	656	17,500	2021	3,170	847	C	0.267	908	C	0.286
CROSSTOWN PKWY	SANDIA DR to MANTH LN	657	20,500	2021	3,170	1,047	C	0.33	1,027	C	0.324
CROSSTOWN PKWY	FLORESTA DR to US 1	66	33,000	2021	3,170	2,191	C	0.691	1,994	C	0.629
CROSSROADS PKWY	OKEECHOBEE RD to KINGS HWY	649	2,333	2017	790	117	C	0.148	117	C	0.148
DARWIN BLVD	BECKER RD to PAAR DR	235	7,400	2021	630	658	F	1.044	609	D	0.967
DARWIN BLVD	PAAR DR to TULIP BLVD	235	7,400	2021	920	658	C	0.715	609	C	0.662
DARWIN BLVD	TULIP BLVD to PORT ST LUCIE BLVD	659	11,500	2021	920	542	C	0.589	537	C	0.584
DEL RIO BLVD	PORT ST LUCIE BLVD to CALIFORNIA BLVD	311	10,293	2019	920	804	C	0.874	725	C	0.788
DEL RIO BLVD	CALIFORNIA BLVD to CASHMERE BLVD	660	8,002	2019	880	488	C	0.555	484	C	0.55
DEL RIO BLVD	CASHMERE BLVD to CALIFORNIA BLVD	661	4,900	2021	880	261	C	0.297	264	C	0.3
DELAWARE AVE	HARTMAN RD to 33RD ST	662	1,617	2016	600	251	C	0.418	202	C	0.337
DELAWARE AVE	33RD ST to 25TH ST	500	2,891	2017	1,710	192	C	0.112	220	C	0.129
DELAWARE AVE	25TH ST to OKEECHOBEE RD	948526	2,000	2020	1,220	92	C	0.075	92	C	0.075
DELAWARE AVE	OKEECHOBEE RD to 13TH ST	663	11,462	2020	790	628	D	0.795	584	D	0.739
DELAWARE AVE	13TH ST to 10TH ST	664	7,700	2021	750	418	D	0.557	399	D	0.532
DELAWARE AVE	10TH ST to 7TH ST	664	7,700	2021	600	418	D	0.697	399	D	0.665
DELAWARE AVE	7TH ST to US 1	665	7,188	2020	750	390	D	0.52	401	D	0.535
EAST TORINO PKWY	CASHMERE BLVD to TORINO PKWY	710	10,500	2021	830	619	C	0.746	612	C	0.737

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Traffic Counts and Level of Service Report 2021

Roadway Name	Location	STATION ID	AADT	Last Count Year	Pk Hr Service Capacity	AM Pk Hr Pk Dir			PM Pk Hr Pk Dir		
						Volume	LOS	V/C	Volume	LOS	V/C
EAST TORINO PKWY	TORINO PKWY to MIDWAY RD	237	14,500	2021	880	940	F	1.068	811	C	0.922
EASY ST	US 1 to BUCHANAN DR	106	5,400	2021	750	299	C	0.399	379	D	0.505
EASY ST	BUCHANAN DR to YUCCA DR	106	5,400	2021	540	299	D	0.554	379	D	0.702
EDWARDS RD	JENKINS RD to MCNEIL RD	174	11,500	2021	630	529	C	0.84	533	C	0.846
EDWARDS RD	MCNEIL RD to SELVITZ RD	174	11,500	2021	700	529	C	0.756	533	C	0.761
EDWARDS RD	SELVITZ RD to 25TH ST	110	14,500	2021	880	729	C	0.828	741	C	0.842
EDWARDS RD	25TH ST to SUNRISE BLVD	108	16,000	2021	1,700	778	D	0.458	778	D	0.458
EDWARDS RD	SUNRISE BLVD to OLEANDER AVE	502	15,401	2019	1,700	764	D	0.449	745	D	0.438
EDWARDS RD	OLEANDER AVE to US 1	173	9,616	2019	1,700	529	C	0.311	462	C	0.272
FARMER'S MARKET RD	OLEANDER AVE to US 1	112	1,848	2019	750	128	C	0.171	125	C	0.167
FLORESTA DR	OAKLYN ST to PORT ST LUCIE BLVD	317	17,572	2019	920	1,216	F	1.322	929	F	1.01
FLORESTA DR	THORNHILL DR to CROSSTOWN PKWY	315	15,459	2019	880	1,002	F	1.139	913	F	1.038
FLORESTA DR	PORT ST LUCIE BLVD to THORNHILL DR	315	15,459	2019	880	1,002	F	1.139	913	F	1.038
FLORESTA DR	CROSSTOWN PKWY to PRIMA VISTA BLVD	109	11,000	2021	920	609	C	0.662	559	C	0.608
FLORESTA DR	PRIMA VISTA BLVD to AIROSO BLVD	107	9,000	2021	920	497	C	0.54	549	C	0.597
FLORESTA DR	SELVITZ RD to BAYSHORE BLVD	313	4,400	2021	630	316	C	0.502	336	C	0.533
FLORESTA DR	AIROSO BLVD to SELVITZ RD	313	4,400	2021	880	316	C	0.359	336	C	0.382
FT PIERCE BLVD	INDRIO RD to EMERSON AVE	226	3,613	2019	580	271	D	0.467	277	D	0.478
GARDENIA AVE	OLEANDER AVE to US 1	666	2,867	2017	750	191	C	0.255	204	C	0.272
GATLIN BLVD	W OF I-95 to E OF I-95	945075	48,500	2020	3,170	3,352	F	1.057	2,732	C	0.862
GATLIN BLVD	E OF I-95 to SAVAGE BLVD	945075	48,500	2020	3,170	3,352	F	1.057	2,732	C	0.862
GATLIN BLVD	SAVAGE BLVD to ROSSER BLVD	945075	48,500	2020	3,170	3,352	F	1.057	2,732	C	0.862
GATLIN BLVD	ROSSER BLVD to SAVONA BLVD	945075	48,500	2020	3,170	3,352	F	1.057	2,732	C	0.862
GATLIN BLVD	SAVONA BLVD to PORT ST LUCIE BLVD	945075	48,500	2020	3,170	3,352	F	1.057	2,732	C	0.862
GEORGIA AVE	25TH ST to OKEECHOBEE RD	667	4,778	2020	600	295	C	0.492	266	C	0.443

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Traffic Counts and Level of Service Report 2021

Roadway Name	Location	STATION ID	AADT	Last Count Year	Pk Hr Service Capacity	AM Pk Hr Pk Dir			PM Pk Hr Pk Dir		
						Volume	LOS	V/C	Volume	LOS	V/C
GEORGIA AVE	OKEECHOBEE RD to 17TH ST	667	4,778	2020	750	295	C	0.393	266	C	0.355
GEORGIA AVE	17TH ST to 13TH ST	508	4,723	2019	600	263	C	0.438	267	C	0.445
GEORGIA AVE	13TH ST to 7TH ST	506	2,165	2019	600	133	C	0.222	136	C	0.227
GEORGIA AVE	7TH ST to US 1	504	1,939	2019	600	122	C	0.203	135	C	0.225
GILSON RD	MARTIN C.L. to BECKER RD	111	10,500	2021	710	880	F	1.239	910	F	1.282
GILSON RD	BECKER RD to LAKERIDGE DR	111	10,500	2021	540	880	F	1.63	910	F	1.685
GLADES CUT-OFF RD	RANGE LINE RD to RESERVE BLVD	668	2,967	2017	1,070	209	B	0.195	264	B	0.247
GLADES CUT-OFF RD	RESERVE BLVD to COMMERCE CENTER DR	119	3,634	2016	1,070	336	B	0.314	336	B	0.314
GLADES CUT-OFF RD	CARLTON RD to RANGE LINE RD	668	2,967	2017	390	209	B	0.536	264	C	0.677
GLADES CUT-OFF RD	COMMERCE CENTER DR to MIDWAY RD	940279	3,400	2020	920	187	C	0.203	171	C	0.186
GLADES CUT-OFF RD	MIDWAY RD to JENKINS RD	115	8,500	2021	790	510	D	0.646	540	D	0.684
GLADES CUT-OFF RD	JENKINS RD to SELVITZ RD	113	6,575	2020	830	368	C	0.443	384	C	0.463
GRAHAM RD	KINGS HWY to JENKINS RD	669	3,833	2017	630	262	C	0.416	250	C	0.397
GREEN RIVER PKWY	MARTIN C.L. to CHARLESTON DR	319	5,700	2021	1,070	395	C	0.369	359	B	0.336
GREEN RIVER PKWY	CHARLESTON DR to MELALEUCA BLVD	319	5,700	2021	1,070	395	C	0.369	359	B	0.336
GREEN RIVER PKWY	MELALEUCA BLVD to WALTON RD	319	5,700	2021	1,070	395	C	0.369	359	B	0.336
HARTMAN RD	OKEECHOBEE RD to PETERSON RD	670	6,000	2021	750	286	C	0.381	280	C	0.373
HARTMAN RD	PETERSON RD to DELAWARE AVE	670	6,000	2021	540	286	D	0.53	280	D	0.519
HARTMAN RD	DELAWARE AVE to ORANGE AVE	670	6,000	2021	790	286	C	0.362	280	C	0.354
HEADER CANAL RD	OKEECHOBEE RD to ORANGE AVE	121	570	2019	670	47	B	0.07	57	B	0.085
HILLMOOR DR	US 1 to LENNARD RD	671	7,057	2019	790	366	C	0.463	465	D	0.589
I-95	GATLIN BLVD to ST LUCIE WEST BLVD	941901	80,500	2020	5,500	4,122	C	0.749	3,723	C	0.677
I-95	ST LUCIE WEST BLVD to MIDWAY RD	941904	66,431	2018	5,500	3,736	C	0.679	3,222	B	0.586
I-95	MIDWAY RD to OKEECHOBEE RD	941902	79,206	2018	5,500	5,004	D	0.91	4,063	C	0.739
I-95	OKEECHOBEE RD to ORANGE AVE	940260	60,386	2020	7,320	2,804	B	0.383	2,804	B	0.383

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						Volume	LOS	V/C	Volume	LOS	V/C
MCCARTY RD	WILLIAMS RD to MIDWAY RD	680	383	2017	540	33	C	0.061	36	C	0.067
MCCARTY RD	MIDWAY RD to OKEECHOBEE RD	681	391	2020	540	34	C	0.063	34	C	0.063
MCNEIL RD	OKEECHOBEE RD to KIRBY LOOP RD	682	6,480	2020	790	396	D	0.501	395	D	0.5
MCNEIL RD	KIRBY LOOP RD to EDWARDS RD	682	6,480	2020	540	396	D	0.733	395	D	0.731
MELALEUCA BLVD	LENNARD RD to GREEN RIVER PKWY	683	11,000	2021	920	647	C	0.703	617	C	0.671
MIDWAY RD	EAST TORINO PKWY to MILNER DR	134	20,500	2021	880	1,001	F	1.137	1,053	F	1.197
MIDWAY RD	MILNER DR to W OF SELVITZ RD	134	20,500	2021	790	1,001	F	1.267	1,053	F	1.333
MIDWAY RD	OKEECHOBEE RD to SHINN RD	940732	8,400	2020	760	408	C	0.537	519	C	0.683
MIDWAY RD	SHINN RD to MCCARTY RD	940732	8,400	2020	630	408	C	0.648	519	C	0.824
MIDWAY RD	MCCARTY RD to I-95	940732	8,400	2020	700	408	C	0.583	519	C	0.741
MIDWAY RD	I-95 to GLADES CUT-OFF RD	945140	19,400	2020	2,100	899	C	0.428	996	C	0.474
MIDWAY RD	GLADES CUT-OFF RD to EAST TORINO PKWY	228	20,000	2021	2,100	1,056	C	0.503	1,087	C	0.518
MIDWAY RD	W OF SELVITZ RD to SELVITZ RD	134	20,500	2021	920	1,001	F	1.088	1,053	F	1.145
MIDWAY RD	SELVITZ RD to CHRISTENSEN RD	132	16,500	2021	920	793	C	0.862	756	C	0.822
MIDWAY RD	CHRISTENSEN RD to 25TH ST	132	16,500	2021	840	793	E	0.944	756	D	0.9
MIDWAY RD	25TH ST to SUNRISE BLVD	130	18,855	2016	840	1,029	F	1.225	945	F	1.125
MIDWAY RD	SUNRISE BLVD to OLEANDER AVE	130	18,855	2016	840	1,029	F	1.225	945	F	1.125
MIDWAY RD	OLEANDER AVE to US 1	242	15,197	2016	840	802	E	0.955	794	E	0.945
MIDWAY RD	US 1 to WALLACE ST	940023	3,600	2020	840	196	C	0.233	216	C	0.257
MIDWAY RD	WALLACE ST to WEATHERBEE RD	940023	3,600	2020	920	196	C	0.213	216	C	0.235
MIDWAY RD	WEATHERBEE RD to INDIAN RIVER DR	940023	3,600	2020	630	196	C	0.311	216	C	0.343
MORNINGSIDE BLVD	WESTMORELAND BLVD to PORT ST LUCIE BLVD	333	2,577	2017	920	155	C	0.168	148	C	0.161
MORNINGSIDE BLVD	PORT ST LUCIE BLVD to LYGATE DR	331	3,910	2020	880	311	C	0.353	330	C	0.375
NEBRASKA AVE	25TH ST to 13TH ST	684	3,733	2017	1,710	232	C	0.136	196	C	0.115
OAKRIDGE DR	MOUNTWELL ST to OAKLYN ST	621	6,623	2019	700	412	C	0.589	358	C	0.511

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Roadway Name	Location	STATION ID	AADT	Last Count Year	Pk Hr Service Capacity	AM Pk Hr Pk Dir			PM Pk Hr Pk Dir		
						Volume	LOS	V/C	Volume	LOS	V/C
ST LUCIE WEST BLVD	CASHMERE BLVD to BAYSHORE BLVD	316	45,500	2021	3,170	1,995	C	0.629	2,125	C	0.67
SUNRISE BLVD	MIDWAY RD to BELL AVE	155	3,500	2021	540	211	C	0.391	223	C	0.413
SUNRISE BLVD	BELL AVE to EDWARDS RD	153	3,793	2016	750	251	C	0.335	284	C	0.379
SUNRISE BLVD	EDWARDS RD to CORTEZ BLVD	511	6,590	2020	600	584	D	0.973	465	D	0.775
SUNRISE BLVD	CORTEZ BLVD to VIRGINIA AVE	511	6,590	2020	750	584	D	0.779	465	D	0.62
SUNRISE BLVD	VIRGINIA AVE to OLEANDER AVE	509	5,073	2020	750	399	D	0.532	393	D	0.524
SUNRISE BLVD	OLEANDER AVE to 7TH ST	708	3,967	2017	1,540	247	C	0.16	287	C	0.186
SUNRISE BLVD	7TH ST to US 1	708	3,967	2017	1,710	247	C	0.144	287	C	0.168
TIFFANY AVE	US 1 to HILLMOOR DR	322	16,941	2019	2,100	966	C	0.46	973	C	0.463
TIFFANY AVE	HILLMOOR DR to VILLAGE GREEN DR	322	16,941	2019	2,100	966	C	0.46	973	C	0.463
TIFFANY AVE	VILLAGE GREEN DR to LENNARD RD	320	4,200	2021	2,100	204	C	0.097	198	C	0.094
TORINO PKWY	CASHMERE BLVD to CALIFORNIA BLVD	709	6,200	2021	630	397	C	0.63	362	C	0.575
TORINO PKWY	CALIFORNIA BLVD to EAST TORINO PKWY	238	4,700	2021	630	310	C	0.492	253	C	0.402
TRADITION PKWY	COMMUNITY BLVD to VILLAGE PKWY	711	6,200	2021	1,710	649	C	0.38	629	C	0.368
TRADITION PKWY	VILLAGE PKWY to W OF I-95	712	33,500	2021	3,170	1,691	C	0.533	1,833	C	0.578
TULIP BLVD	DARWIN BLVD to PORT ST LUCIE BLVD	713	8,471	2019	790	541	D	0.685	471	D	0.596
TULIP BLVD	PORT ST LUCIE BLVD to PAAR DR	714	8,800	2021	790	492	D	0.623	526	D	0.666
TULIP BLVD	PAAR DR to DARWIN BLVD	714	8,800	2021	790	492	D	0.623	526	D	0.666
TURNPIKE FEEDER RD	TURNPIKE FEEDER RD SB RAMP to US 1	940078	4,971	2015	660	447	C	0.677	447	C	0.677
TURNPIKE FEEDER RD	INDIAN PINES BLVD to TURNPIKE FEEDER RD SB R...	940269	11,200	2020	870	700	C	0.805	642	C	0.738
TURNPIKE FEEDER RD	INDRIO RD to INDIAN PINES BLVD	940745	12,400	2020	870	649	C	0.746	682	C	0.784
US 1	MARTIN C.L. to LENNARD RD	945071	58,500	2020	4,240	2,664	C	0.628	3,132	C	0.739
US 1	LENNARD RD to PORT ST LUCIE BLVD	945071	58,500	2020	4,040	2,664	C	0.659	3,132	C	0.775
US 1	PORT ST LUCIE BLVD to JENNINGS RD	945070	30,000	2020	3,020	1,409	C	0.467	1,495	C	0.495
US 1	JENNINGS RD to TIFFANY AVE	945070	30,000	2020	3,020	1,409	C	0.467	1,495	C	0.495

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TABLE 7

Generalized **Peak Hour Directional** Volumes for Florida's

Urbanized Areas

January 2020

INTERRUPTED FLOW FACILITIES						UNINTERRUPTED FLOW FACILITIES					
STATE SIGNALIZED ARTERIALS						FREEWAYS					
Class I (40 mph or higher posted speed limit)						Core Urbanized					
Lanes	Median	B	C	D	E	Lanes	B	C	D	E	
1	Undivided	*	830	880	**	2	2,230	3,100	3,740	4,080	
2	Divided	*	1,910	2,000	**	3	3,280	4,570	5,620	6,130	
3	Divided	*	2,940	3,020	**	4	4,310	6,030	7,490	8,170	
4	Divided	*	3,970	4,040	**	5	5,390	7,430	9,370	10,220	
						6	6,380	8,990	11,510	12,760	
Class II (35 mph or slower posted speed limit)						Urbanized					
Lanes	Median	B	C	D	E	Lanes	B	C	D	E	
1	Undivided	*	370	750	800	2	2,270	3,100	3,890	4,230	
2	Divided	*	730	1,630	1,700	3	3,410	4,650	5,780	6,340	
3	Divided	*	1,170	2,520	2,560	4	4,550	6,200	7,680	8,460	
4	Divided	*	1,610	3,390	3,420	5	5,690	7,760	9,520	10,570	
Non-State Signalized Roadway Adjustments (Alter corresponding state volumes by the indicated percent.)						Freeway Adjustments					
Non-State Signalized Roadways - 10%						Auxiliary Lane + 1,000 Ramp Metering + 5%					
Median & Turn Lane Adjustments						UNINTERRUPTED FLOW HIGHWAYS					
Lanes	Median	Exclusive Left Lanes	Exclusive Right Lanes	Adjustment Factors		Lanes	Median	B	C	D	E
1	Divided	Yes	No	+5%		1	Undivided	580	890	1,200	1,610
1	Undivided	No	No	-20%		2	Divided	1,800	2,600	3,280	3,730
Multi	Undivided	Yes	No	-5%		3	Divided	2,700	3,900	4,920	5,600
Multi	Undivided	No	No	-25%							
-	-	-	Yes	+ 5%		Uninterrupted Flow Highway Adjustments					
One-Way Facility Adjustment Multiply the corresponding directional volumes in this table by 1.2						Lanes	Median	Exclusive left lanes	Adjustment factors		
						1	Divided	Yes	+5%		
						Multi	Undivided	Yes	-5%		
						Multi	Undivided	No	-25%		
BICYCLE MODE² (Multiply vehicle volumes shown below by number of directional roadway lanes to determine two-way maximum service volumes.)						¹ Values shown are presented as peak hour directional volumes for levels of service and are for the automobile/truck modes unless specifically stated. This table does not constitute a standard and should be used only for general planning applications. The computer models from which this table is derived should be used for more specific planning applications. The table and deriving computer models should not be used for corridor or intersection design, where more refined techniques exist. Calculations are based on planning applications of the HCM and the Transit Capacity and Quality of Service Manual.					
Paved Shoulder/Bicycle Lane Coverage						² Level of service for the bicycle and pedestrian modes in this table is based on number of vehicles, not number of bicyclists or pedestrians using the facility.					
		B	C	D	E	³ Buses per hour shown are only for the peak hour in the single direction of the higher traffic flow.					
0-49%		*	150	390	1,000	* Cannot be achieved using table input value defaults.					
50-84%		110	340	1,000	>1,000	** Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached. For the bicycle mode, the level of service letter grade (including F) is not achievable because there is no maximum vehicle volume threshold using table input value defaults.					
85-100%		470	1,000	>1,000	**	<i>Source:</i> Florida Department of Transportation Systems Implementation Office https://www.fdot.gov/planning/systems/					
PEDESTRIAN MODE² (Multiply vehicle volumes shown below by number of directional roadway lanes to determine two-way maximum service volumes.)											
Sidewalk Coverage		B	C	D	E						
0-49%		*	*	140	480						
50-84%		*	80	440	800						
85-100%		200	540	880	>1,000						
BUS MODE (Scheduled Fixed Route)³ (Buses in peak hour in peak direction)											
Sidewalk Coverage		B	C	D	E						
0-84%		> 5	≥ 4	≥ 3	≥ 2						
85-100%		> 4	≥ 3	≥ 2	≥ 1						

APPENDIX E: Intersection Volume Development

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & I-95 SB Ramp

COUNT DATE: June 25, 2020

PM PEAK HOUR FACTOR: 0.92

"PM EXISTING TRAFFIC"	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	0	112	132	2	395	207	0	0	0	0	254	0	30
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	0	144	170	3	508	266	0	0	0	0	326	0	39

"PM BACKGROUND TRAFFIC"	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station		1			47	1					47		
Project Midway Industrial		4			130	5					106		
TOTAL "VESTED" TRAFFIC	0	5	0		177	6	0	0	0	0	153	0	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	10	12	0	36	19	0	0	0	0	23	0	3

PM NON-PROJECT TRAFFIC	0	159	182	3	721	291	0	0	0	0	502	0	42
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering		4.0%									40.0%		
	Exiting					20.0%	4.0%							

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering		4.0%									40.0%		
	Exiting					20.0%	4.0%							

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New		1			5	1					3		
Bldg 200 Project Trips	Net New		1			5	1					4		
PM TOTAL PROJECT TRAFFIC		0	2	0	0	10	2	0	0	0	0	7	0	0

PM TOTAL TRAFFIC	0	161	182	3	731	293	0	0	0	0	509	0	42
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & I-95 NB Ramp

COUNT DATE: June 25, 2020

PM PEAK HOUR FACTOR: 0.95

"PM EXISTING TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	9	331	0	0	508	255	121	0	384	0	0	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	12	425	0	0	653	328	156	0	494	0	0	0

"PM BACKGROUND TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station		49			49	47			47			
Project Midway Industrial		109			134	130			106			
TOTAL "VESTED" TRAFFIC	0	158	0	0	183	177	0	0	153	0	0	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	1	30	0	0	47	23	11	0	35	0	0	0

PM NON-PROJECT TRAFFIC	13	613	0	0	883	528	167	0	682	0	0	0
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering		44.0%							20.0%			
	Exiting				24.0%	40.0%							

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering		44.0%							20.0%			
	Exiting				24.0%	40.0%							

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New		4			6	9			1			
Bldg 200 Project Trips	Net New		5			6	12			2			
PM TOTAL PROJECT TRAFFIC		0	9	0	0	12	21	0	0	3	0	0	0

PM TOTAL TRAFFIC	13	622	0	0	895	549	167	0	685	0	0	0
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & LTC Parkway
COUNT DATE: June 25, 2020
PM PEAK HOUR FACTOR: 0.96

"PM EXISTING TRAFFIC"	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	0	679	40	3	4	693	0	71	0	14	0	0	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	0	873	51	4	5	891	0	91	0	18	0	0	0

"PM BACKGROUND TRAFFIC"	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station					102	-45		140		100			
Project Midway Industrial			215		156			264		192			
TOTAL "VESTED" TRAFFIC	0	0	215	0	258	-45	0	404	0	292	0	0	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	62	4	0	0	63	0	6	0	1	0	0	0

PM NON-PROJECT TRAFFIC	0	935	270	4	263	909	0	501	0	311	0	0	0
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering			64.0%		34.0%								
	Exiting								64.0%		34.0%			

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering		60.0%	4.0%										
	Exiting								64.0%					

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New			5		2			15		7			
Bldg 200 Project Trips	Net New		6	1					18					
PM TOTAL PROJECT TRAFFIC		0	6	6	0	2	0	0	33	0	7	0	0	0

PM TOTAL TRAFFIC	0	941	276	4	265	909	0	534	0	318	0	0	0
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & Glades Cut-Off Road
 COUNT DATE: June 25, 2020
 PM PEAK HOUR FACTOR: 0.94

"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
PM Raw Turning Movements	5	119	564	11	32	494	86	26	50	72	2	141	34	161
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	6	153	725	14	41	635	111	33	64	93	3	181	44	207

"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Speedway Gas Station		9	48			48		7						9
Project Midway Industrial		29	163			133								23
TOTAL "VESTED" TRAFFIC	0	38	211	0	0	181	0	7	0	0	0	0	0	32

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	11	52	1	3	45	8	2	5	7	0	13	3	15

PM NON-PROJECT TRAFFIC	6	202	988	15	44	861	119	42	69	100	3	194	47	254
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"BLDG 100 PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE														
Net New Distribution	Entering						27.0%								7.0%
	Exiting		7.0%	27.0%											

"BLDG 200 PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE														
Net New Distribution	Entering					27.0%								7.0%	
	Exiting		7.0%	21.0%							6.0%				

"PM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE														
Bldg 100 Project Trips	Net New		1	6			1								1
Bldg 200 Project Trips	Net New		2	5		2					2			1	
PM TOTAL PROJECT TRAFFIC		0	3	11	0	2	1	0	0	0	2	0	0	1	1

PM TOTAL TRAFFIC	6	205	999	15	46	862	119	42	69	102	3	194	48	255
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & East Torino Parkway
COUNT DATE: June 25, 2020
PM PEAK HOUR FACTOR: 0.96

"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	1	1	498	318	288	501	1	140	0	233	0	1	5
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	1	1	640	409	370	644	1	180	0	299	0	1	6

"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station			43	6		43		6					
Project Midway Industrial			144	19		117		16					
TOTAL "VESTED" TRAFFIC		0	187	25	0	160	0	22	0	0	0	0	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	0	46	29	26	46	0	13	0	21	0	0	0

PM NON-PROJECT TRAFFIC	1	1	873	463	396	850	1	215	0	320	0	1	6
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering						25.0%		2.0%					
	Exiting			25.0%	2.0%									

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering						25.0%		2.0%					
	Exiting			25.0%	2.0%									

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New			5	1		1		0					
Bldg 200 Project Trips	Net New			6	1		2		0					
PM TOTAL PROJECT TRAFFIC		0	0	11	2	0	3	0	0	0	0	0	0	0

PM TOTAL TRAFFIC	1	1	884	465	396	853	1	215	0	320	0	1	6
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & Tropicana Dwy & Project Dwy 1
 COUNT DATE: September 30, 2021
 PM PEAK HOUR FACTOR: 0.91

"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	1	3	787	0	5	0	829	0	0	0	0	2	0	11
Peak Season Correction Factor	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14	1.14

PM EXISTING CONDITIONS	1	3	897	0	6	0	945	0	0	0	0	2	0	13
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"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station			100				102							
Project Midway Industrial			192				156							
TOTAL "VESTED" TRAFFIC		0	292	0		0	258	0	0	0	0	0	0	0

Years To Buildout	1	1	1	1	1	1	1	1	1	1	1	1	1	1
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	0	31	0	0	0	33	0	0	0	0	0	0	0

PM NON-PROJECT TRAFFIC	1	3	1,220	0	6	0	1,236	0	0	0	0	2	0	13
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering							34.0%							
	Exiting			34.0%											

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering				60.0%										
	Exiting											28.0%			

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New			7				2							
Bldg 200 Project Trips	Net New				6							7			
PM TOTAL PROJECT TRAFFIC		0	0	7	6	0	0	2	0	0	0	7	0	0	0

PM TOTAL TRAFFIC	1	3	1,227	6	6	0	1,238	0	0	0	7	2	0	13
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Glades Cut-Off Road & Project Dwy 2

COUNT DATE: June 25, 2020

PM PEAK HOUR FACTOR: 0.94

"PM EXISTING TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	0	0	0	0	0	0	0	148	0	0	77	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	0	0	0	0	0	0	0	190	0	0	99	0

"PM BACKGROUND TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station								7			0	
Project Midway Industrial								0			0	
TOTAL "VESTED" TRAFFIC	0	0	0	0	0	0	0	7	0	0	0	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	0	0	0	0	0	0	14	0	0	7	0

PM NON-PROJECT TRAFFIC	0	0	0	0	0	0	0	211	0	0	106	0
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering												
	Exiting												

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering							2.0%					34.0%
	Exiting	6.0%		2.0%									

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New												
Bldg 200 Project Trips	Net New	2		1				1					3
PM TOTAL PROJECT TRAFFIC		2	0	1	0	0	0	1	0	0	0	0	3

PM TOTAL TRAFFIC	2	0	1	0	0	0	0	1	211	0	0	106	3
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: LTC Parkway & Project Dwy 3
COUNT DATE: June 25, 2020
PM PEAK HOUR FACTOR: 0.96

"PM EXISTING TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	0	0	0	0	0	0	0	85	0	0	44	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	0	0	0	0	0	0	0	109	0	0	57	0

"PM BACKGROUND TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station								240			102	
Project Midway Industrial								456			371	
TOTAL "VESTED" TRAFFIC	0	0	0	0	0	0	0	696	0	0	473	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	0	0	0	0	0	0	8	0	0	4	0

PM NON-PROJECT TRAFFIC	0	0	0	0	0	0	0	813	0	0	534	0
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering											98.0%	
	Exiting							98.0%					

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering											4.0%	
	Exiting						64.0%						

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New								22			7	
Bldg 200 Project Trips	Net New						18					1	
PM TOTAL PROJECT TRAFFIC		0	0	0	0	0	18	0	22	0	0	8	0

PM TOTAL TRAFFIC	0	0	0	0	0	0	18	0	835	0	0	542	0
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: LTC Parkway & Project Dwy 4
COUNT DATE: June 25, 2020
PM PEAK HOUR FACTOR: 0.96

"PM EXISTING TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	0	0	0	0	0	0	0	85	0	0	44	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	0	0	0	0	0	0	0	109	0	0	57	0

"PM BACKGROUND TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station	240		4				7				3	
Project Midway Industrial								456			371	
TOTAL "VESTED" TRAFFIC	240	0	4	0	0	0	7	456	0	0	374	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	0	0	0	0	0	0	8	0	0	4	0

PM NON-PROJECT TRAFFIC	240	0	4	0	0	0	7	573	0	0	435	0
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering										98.0%		
	Exiting				2.0%		90.0%		8.0%				

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering										4.0%		
	Exiting												

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New				1		20		2		7		
Bldg 200 Project Trips	Net New										1		
PM TOTAL PROJECT TRAFFIC		0	0	0	1	0	20	0	2	0	8	0	0

PM TOTAL TRAFFIC	240	0	4	1	0	20	7	575	0	8	435	0
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: LTC Parkway & Project Dwy 5
COUNT DATE: June 25, 2020
PM PEAK HOUR FACTOR: 0.96

"PM EXISTING TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
PM Raw Turning Movements	0	0	0	0	0	0	0	85	0	0	44	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
Growth to 2021	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%	4.50%
PM EXISTING CONDITIONS	0	0	0	0	0	0	0	109	0	0	57	0

"PM BACKGROUND TRAFFIC"	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Speedway Gas Station								7			7	
Project Midway Industrial								456			371	
TOTAL "VESTED" TRAFFIC	0	0	0	0	0	0	0	463	0	0	378	0

Years To Buildout	2	2	2	2	2	2	2	2	2	2	2	2
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH	0	0	0	0	0	0	0	8	0	0	4	0

PM NON-PROJECT TRAFFIC	0	0	0	0	0	0	0	580	0	0	439	0
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"BLDG 100 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering									2.0%			
	Exiting						8.0%					2.0%	

"BLDG 200 PROJECT DISTRIBUTION"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Net New Distribution	Entering												
	Exiting												

"PM PROJECT TRAFFIC"

LAND USE	TYPE	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Bldg 100 Project Trips	Net New						2			1		1	
Bldg 200 Project Trips	Net New												
PM TOTAL PROJECT TRAFFIC		0	0	0	0	0	2	0	0	1	0	1	0

PM TOTAL TRAFFIC	0	0	0	0	0	0	2	0	580	1	0	440	0
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**APPENDIX F:
Signal Timing**

St. Lucie County



MOVING TRAFFIC FORWARD

00024 - MIDWAY RD @ I-95 SB RAMP - - Econolite Type - Cobalt

Controller Timing Plan (MM) 2-1

Plan 1 - ""

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction	N-L	WB-T	E-L	W-T	WB-L	EB-T	W-L	SB	N	N	N	N	N	N	N	N
Min Green	0	8	0	0	7	8	0	6	5	5	5	5	5	5	5	5
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	0	0	0	0	0	0	0	0	10	0	10	0	10	0	10
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	0	0	0	0	0	0	0	0	16	0	16	0	16	0	16
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	0.0	3.0	0.0	0.0	5.0	3.0	0.0	3.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	0	45	0	0	30	45	0	25	35	35	35	35	35	35	35	35
Max2	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	0.0	5.0	0.0	0.0	5.0	5.0	0.0	4.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Red Clear	0.0	3.0	0.0	0.0	3.0	3.0	0.0	3.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Q in use

St. Lucie County



MOVING TRAFFIC FORWARD

00027 - MIDWAY RD @ TORINO PKWY - - Econolite Type - Cobalt

Controller Timing Plan (MM) 2-1

Plan 1 - ""

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction	E-L	W-T	S-L	N-T	W-L	E-T	N-L	S-T	N	N	N	N	N	N	N	N
Min Green	7	7	7	7	7	7	7	7	5	5	5	5	5	5	5	5
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	0	0	0	0	0	0	0	0	10	0	10	0	10	0	10
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	0	0	0	0	0	0	0	0	16	0	16	0	16	0	16
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	5.0	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	15	60	15	40	35	60	40	10	35	35	35	35	35	35	35	35
Max2	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40	40
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Red Clear	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Ø in use

St. Lucie County



NB/SB and EB/WB timings are assumed to be flipped

00022 - MIDWAY RD @ GLADES CUT OFF - - Econolite Type - Cobalt

Controller Timing Plan (MM) 2-1

Plan 1 - ""

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction	N-L	S-T	E-L	W-T	S-L	N-T	W-L	E-T	N	N	N	N	N	N	N	N
Min Green	15	10	7	7	10	10	7	7	5	5	5	5	5	5	5	5
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	7	0	0	0	0	0	7	0	0	0	10	0	10	0	10
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	20	0	0	0	0	0	20	0	0	0	16	0	16	0	16
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	25	60	20	25	25	60	10	25	35	35	35	35	35	35	35	35
Max2	0	0	0	0	0	0	0	0	40	40	40	40	40	40	40	40
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.0	5.0	4.0	4.0	5.0	5.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Red Clear	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	0.0	2.0	0.0	2.0	0.0	0.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

St. Lucie County



MOVING TRAFFIC FORWARD

00023 - MIDWAY RD @ I-95 NB RAMP - - Econolite Type - Cobalt

Controller Timing Plan (MM) 2-1

Plan 1 - ""

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction	EBL	WB	N	NB	N	N	N	N	N	N	N	N	N	N	N	N
Min Green	7	15	5	7	5	15	5	5	5	5	5	5	5	5	5	5
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	0	0	0	0	0	0	0	0	10	0	10	0	10	0	10
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	0	0	0	0	0	0	0	0	16	0	16	0	16	0	16
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	18	45	0	25	0	45	0	25	35	35	35	35	35	35	35	35
Max2	0	0	0	0	0	0	0	0	40	40	40	40	40	40	40	40
Max3	28	55	0	35	0	55	0	35	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.0	5.0	4.0	5.0	4.0	5.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Red Clear	3.0	3.0	1.0	3.0	1.0	3.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Ø in use

APPENDIX G: Synchro Outputs

Existing (2021) Conditions

Lanes, Volumes, Timings
1: Midway Rd & I-95 SB off-ramp

Glades Cut Off Road Industrial
Existing Conditions, PM Peak



Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑			↑↑						↑	↑
Traffic Volume (vph)	0	144	170	3	508	266	0	0	0	0	326	0	39
Future Volume (vph)	0	144	170	3	508	266	0	0	0	0	326	0	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		500		400		0	0		0	0		400
Storage Lanes	0		1		1		0	0		0	0		1
Taper Length (ft)	25				25			25			25		
Right Turn on Red			Yes				Yes			Yes			Yes
Link Speed (mph)		45				45			45			45	
Link Distance (ft)		1345				1562			1124			796	
Travel Time (s)		20.4				23.7			17.0			12.1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%	3%	0%	0%	0%	9%	9%	9%
Shared Lane Traffic (%)													
Turn Type		NA	Perm	Prot	Prot	NA					Perm	NA	Perm
Protected Phases		6		5	5	2						8	
Permitted Phases			6									8	8
Detector Phase		6	6	5	5	2					8	8	8
Switch Phase													
Minimum Initial (s)		8.0	8.0	7.0	7.0	8.0					6.0	6.0	6.0
Minimum Split (s)		16.0	16.0	15.0	15.0	16.0					13.0	13.0	13.0
Total Split (s)		45.0	45.0	30.0	30.0	75.0					25.0	25.0	25.0
Total Split (%)		45.0%	45.0%	30.0%	30.0%	75.0%					25.0%	25.0%	25.0%
Yellow Time (s)		5.0	5.0	5.0	5.0	5.0					4.0	4.0	4.0
All-Red Time (s)		3.0	3.0	3.0	3.0	3.0					3.0	3.0	3.0
Lost Time Adjust (s)		0.0	0.0		0.0	0.0						0.0	0.0
Total Lost Time (s)		8.0	8.0		8.0	8.0						7.0	7.0
Lead/Lag		Lag	Lag	Lead	Lead								
Lead-Lag Optimize?		Yes	Yes	Yes	Yes								
Recall Mode		Min	Min	None	None	Min					None	None	None

Intersection Summary

Area Type: Other

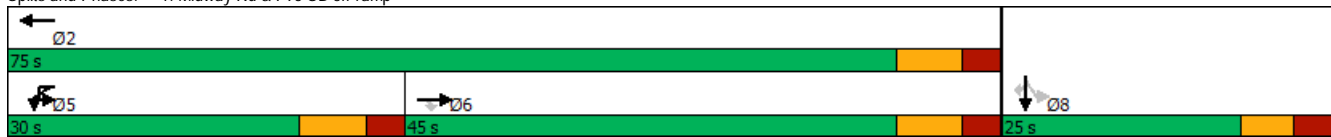
Cycle Length: 100

Actuated Cycle Length: 71.9

Natural Cycle: 75














Control Type: Actuated-Uncoordinated

Splits and Phases: 1: Midway Rd & I-95 SB off-ramp



HCM Signalized Intersection Capacity Analysis
 1: Midway Rd & I-95 SB off-ramp

Glades Cut Off Road Industrial
 Existing Conditions, PM Peak

														
Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↑↑	↑			↑↑						↑	↑	
Traffic Volume (vph)	0	144	170	3	508	266	0	0	0	0	326	0	39	
Future Volume (vph)	0	144	170	3	508	266	0	0	0	0	326	0	39	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		8.0	8.0		8.0	8.0						7.0	7.0	
Lane Util. Factor		0.95	1.00		1.00	0.95						1.00	1.00	
Frt		1.00	0.85		1.00	1.00						1.00	0.85	
Flt Protected		1.00	1.00		0.95	1.00						0.95	1.00	
Satd. Flow (prot)		3539	1583		1753	3505						1656	1482	
Flt Permitted		1.00	1.00		0.95	1.00						0.95	1.00	
Satd. Flow (perm)		3539	1583		1753	3505						1656	1482	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	157	185	3	552	289	0	0	0	0	354	0	42	
RTOR Reduction (vph)	0	0	162	0	0	0	0	0	0	0	0	0	31	
Lane Group Flow (vph)	0	157	23	0	555	289	0	0	0	0	0	354	11	
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%	3%	0%	0%	0%	9%	9%	9%	
Turn Type		NA	Perm	Prot	Prot	NA					Perm	NA	Perm	
Protected Phases		6		5	5	2						8		
Permitted Phases			6								8		8	
Actuated Green, G (s)		8.9	8.9		22.0	38.9						18.0	18.0	
Effective Green, g (s)		8.9	8.9		22.0	38.9						18.0	18.0	
Actuated g/C Ratio		0.12	0.12		0.31	0.54						0.25	0.25	
Clearance Time (s)		8.0	8.0		8.0	8.0						7.0	7.0	
Vehicle Extension (s)		3.0	3.0		5.0	3.0						3.0	3.0	
Lane Grp Cap (vph)		438	195		536	1896						414	371	
v/s Ratio Prot		c0.04			c0.32	0.08								
v/s Ratio Perm			0.01									0.21	0.01	
v/c Ratio		0.36	0.12		1.04	0.15						0.86	0.03	
Uniform Delay, d1		28.9	28.0		25.0	8.3						25.7	20.3	
Progression Factor		1.00	1.00		1.00	1.00						1.00	1.00	
Incremental Delay, d2		0.5	0.3		48.3	0.0						15.7	0.0	
Delay (s)		29.4	28.3		73.3	8.3						41.4	20.4	
Level of Service		C	C		E	A						D	C	
Approach Delay (s)		28.8				51.0			0.0			39.2		
Approach LOS		C				D			A			D		
Intersection Summary														
HCM 2000 Control Delay			43.3		HCM 2000 Level of Service								D	
HCM 2000 Volume to Capacity ratio			0.85											
Actuated Cycle Length (s)			71.9		Sum of lost time (s)							23.0		
Intersection Capacity Utilization			76.1%		ICU Level of Service							D		
Analysis Period (min)			15											
c Critical Lane Group														

Lanes, Volumes, Timings
2: I-95 NB off-ramp & Midway Rd

Glades Cut Off Road Industrial
Existing Conditions, PM Peak



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗			↖	↗		↖	↗			
Traffic Volume (vph)	12	425	0	0	653	328	156	0	494	0	0	0
Future Volume (vph)	12	425	0	0	653	328	156	0	494	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	0		350	0		300	0		0
Storage Lanes	1		0	0		1	0		1	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1562			2097			847			779	
Travel Time (s)		23.7			31.8			12.8			11.8	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	6%	6%	6%	7%	7%	7%	7%	7%	7%	0%	0%	0%
Shared Lane Traffic (%)												
Turn Type	pm+pt	NA			NA	Perm	Perm	NA	Perm			
Protected Phases	1	6			2			4				
Permitted Phases	6					2	4		4			
Detector Phase	1	6			2	2	4	4	4			
Switch Phase												
Minimum Initial (s)	7.0	15.0			15.0	15.0	7.0	7.0	7.0			
Minimum Split (s)	15.0	23.0			23.0	23.0	15.0	15.0	15.0			
Total Split (s)	18.0	63.0			45.0	45.0	25.0	25.0	25.0			
Total Split (%)	20.5%	71.6%			51.1%	51.1%	28.4%	28.4%	28.4%			
Yellow Time (s)	5.0	5.0			5.0	5.0	5.0	5.0	5.0			
All-Red Time (s)	3.0	3.0			3.0	3.0	3.0	3.0	3.0			
Lost Time Adjust (s)	0.0	0.0			0.0	0.0		0.0	0.0			
Total Lost Time (s)	8.0	8.0			8.0	8.0		8.0	8.0			
Lead/Lag	Lead				Lag	Lag						
Lead-Lag Optimize?	Yes				Yes	Yes						
Recall Mode	None	Min			Min	Min	None	None	None			

Intersection Summary

Area Type: Other
 Cycle Length: 88
 Actuated Cycle Length: 50.9
 Natural Cycle: 60
 Control Type: Actuated-Uncoordinated

Splits and Phases: 2: I-95 NB off-ramp & Midway Rd

Ø1 18 s	Ø2 45 s	Ø4 25 s
Ø6 63 s		

HCM 6th Signalized Intersection Summary
 2: I-95 NB off-ramp & Midway Rd

Glades Cut Off Road Industrial
 Existing Conditions, PM Peak










Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↑↑			↑↑	↔		↔	↔				
Traffic Volume (veh/h)	12	425	0	0	653	328	156	0	494	0	0	0	
Future Volume (veh/h)	12	425	0	0	653	328	156	0	494	0	0	0	
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0	
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00				
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Work Zone On Approach	No												
Adj Sat Flow, veh/h/ln	1811	1811	0	0	1796	1796	1796	1796	1796				
Adj Flow Rate, veh/h	13	447	0	0	687	0	164	0	0				
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95				
Percent Heavy Veh, %	6	6	0	0	7	7	7	7	7				
Cap, veh/h	309	1792	0	0	1108		228	0					
Arrive On Green	0.02	0.52	0.00	0.00	0.32	0.00	0.13	0.00	0.00				
Sat Flow, veh/h	1725	3532	0	0	3503	1522	1711	0	1522				
Grp Volume(v), veh/h	13	447	0	0	687	0	164	0	0				
Grp Sat Flow(s),veh/h/ln	1725	1721	0	0	1706	1522	1711	0	1522				
Q Serve(g_s), s	0.2	3.3	0.0	0.0	7.9	0.0	4.2	0.0	0.0				
Cycle Q Clear(g_c), s	0.2	3.3	0.0	0.0	7.9	0.0	4.2	0.0	0.0				
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00				
Lane Grp Cap(c), veh/h	309	1792	0	0	1108		228	0					
V/C Ratio(X)	0.04	0.25	0.00	0.00	0.62		0.72	0.00					
Avail Cap(c_a), veh/h	642	4094	0	0	2732		629	0					
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Upstream Filter(I)	1.00	1.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00				
Uniform Delay (d), s/veh	9.7	6.1	0.0	0.0	13.2	0.0	19.2	0.0	0.0				
Incr Delay (d2), s/veh	0.1	0.1	0.0	0.0	0.6	0.0	4.3	0.0	0.0				
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
%ile BackOfQ(95%),veh/ln	0.1	1.2	0.0	0.0	4.1	0.0	2.9	0.0	0.0				
Unsig. Movement Delay, s/veh													
LnGrp Delay(d),s/veh	9.8	6.2	0.0	0.0	13.8	0.0	23.5	0.0	0.0				
LnGrp LOS	A	A	A	A	B		C	A					
Approach Vol, veh/h	460						687		A	164			A
Approach Delay, s/veh	6.3						13.8			23.5			
Approach LOS	A						B			C			
Timer - Assigned Phs	1	2	4		6								
Phs Duration (G+Y+Rc), s	9.1	23.0	14.1		32.1								
Change Period (Y+Rc), s	8.0	8.0	8.0		8.0								
Max Green Setting (Gmax), s	10.0	37.0	17.0		55.0								
Max Q Clear Time (g_c+I1), s	2.2	9.9	6.2		5.3								
Green Ext Time (p_c), s	0.0	4.6	0.5		2.9								

Intersection Summary												
HCM 6th Ctrl Delay	12.4											
HCM 6th LOS	B											

Notes
 Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lanes, Volumes, Timings
3: LTC Pkwy & Midway Rd

Glades Cut Off Road Industrial
Existing Conditions, PM Peak

							
Lane Group	EBT	EBR	WBU	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↓	↑↑	↓	↑
Traffic Volume (vph)	873	51	4	5	891	91	18
Future Volume (vph)	873	51	4	5	891	91	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		550		400		0	175
Storage Lanes		1		1		1	1
Taper Length (ft)				25		25	
Link Speed (mph)	35				45	30	
Link Distance (ft)	2097				1746	980	
Travel Time (s)	40.9				26.5	22.3	
Peak Hour Factor	0.96	0.96	0.92	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	8%	8%	7%	7%	7%	7%	7%
Shared Lane Traffic (%)							
Sign Control	Free				Free	Stop	

Intersection Summary

Area Type: Other
Control Type: Unsignalized

Intersection							
Int Delay, s/veh	1.5						
Movement	EBT	EBR	WBU	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↓	↑↑	↓	↑
Traffic Vol, veh/h	873	51	4	5	891	91	18
Future Vol, veh/h	873	51	4	5	891	91	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	-	None	-	None
Storage Length	-	550	-	400	-	0	175
Veh in Median Storage, #	0	-	-	-	0	1	-
Grade, %	0	-	-	-	0	0	-
Peak Hour Factor	96	96	92	96	96	96	96
Heavy Vehicles, %	8	8	7	7	7	7	7
Mvmt Flow	909	53	4	5	928	95	19
Major/Minor	Major1	Major2	Major2		Minor1	Minor1	
Conflicting Flow All	0	0	909	962	0	1391	455
Stage 1	-	-	-	-	-	909	-
Stage 2	-	-	-	-	-	482	-
Critical Hdwy	-	-	6.54	4.24	-	6.94	7.04
Critical Hdwy Stg 1	-	-	-	-	-	5.94	-
Critical Hdwy Stg 2	-	-	-	-	-	5.94	-
Follow-up Hdwy	-	-	2.57	2.27	-	3.57	3.37
Pot Cap-1 Maneuver	-	-	365	681	-	127	539
Stage 1	-	-	-	-	-	342	-
Stage 2	-	-	-	-	-	573	-
Platoon blocked, %	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	479	479	-	124	539
Mov Cap-2 Maneuver	-	-	-	-	-	246	-
Stage 1	-	-	-	-	-	342	-
Stage 2	-	-	-	-	-	561	-
Approach	EB	WB	WB		NB	NB	
HCM Control Delay, s	0	0.1	0.1		25.8	25.8	
HCM LOS			D			D	
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT	WBT
Capacity (veh/h)	246	539	-	-	479	-	-
HCM Lane V/C Ratio	0.385	0.035	-	-	0.02	-	-
HCM Control Delay (s)	28.5	11.9	-	-	12.7	-	-
HCM Lane LOS	D	B	-	-	B	-	-
HCM 95th %tile Q(veh)	1.7	0.1	-	-	0.1	-	-

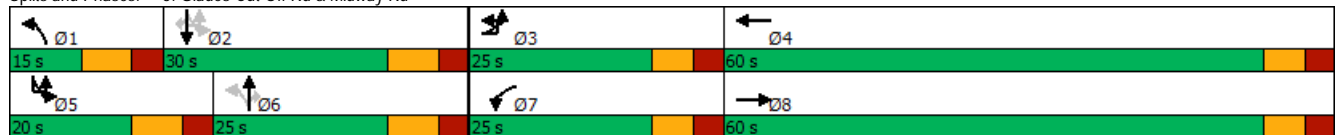
Lanes, Volumes, Timings
5: Glades Cut-Off Rd & Midway Rd

Glades Cut Off Road Industrial
Existing Conditions, PM Peak

Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Lane Configurations		↔↔	↕↕		↔	↕↕		↔	↕	↕		↔	↕	↕
Traffic Volume (vph)	6	153	725	14	41	635	111	33	64	93	3	181	44	207
Future Volume (vph)	6	153	725	14	41	635	111	33	64	93	3	181	44	207
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		625		0	400		0	325		325		400		275
Storage Lanes		2		0	1		0	1		1		1		1
Taper Length (ft)		25			25			25				25		
Right Turn on Red				Yes			Yes			Yes				Yes
Link Speed (mph)			50			50			50					50
Link Distance (ft)			894			1488			1663					1361
Travel Time (s)			12.2			20.3			22.7					18.6
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	7%	7%	7%	6%	6%	6%	6%	6%	6%	2%	9%	9%	9%
Shared Lane Traffic (%)														
Turn Type	Prot	Prot	NA		Prot	NA		pm+pt	NA	Perm	pm+pt	pm+pt	NA	Perm
Protected Phases	3	3	8		7	4		1	6		5	5	2	
Permitted Phases								6		6	2	2		2
Detector Phase	3	3	8		7	4		1	6	6	5	5	2	2
Switch Phase														
Minimum Initial (s)	15.0	15.0	7.0		15.0	7.0		7.0	10.0	10.0	7.0	7.0	10.0	10.0
Minimum Split (s)	22.0	22.0	14.0		22.0	14.0		15.0	18.0	18.0	18.0	18.0	18.0	18.0
Total Split (s)	25.0	25.0	60.0		25.0	60.0		15.0	25.0	25.0	20.0	20.0	30.0	30.0
Total Split (%)	19.2%	19.2%	46.2%		19.2%	46.2%		11.5%	19.2%	19.2%	15.4%	15.4%	23.1%	23.1%
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)		0.0	0.0		0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0
Lead/Lag	Lead	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	Min		None	Min		None	None	None	None	None	None	None

Intersection Summary	
Area Type:	Other
Cycle Length:	130
Actuated Cycle Length:	103.4
Natural Cycle:	80
Control Type:	Actuated-Uncoordinated

Splits and Phases: 5: Glades Cut-Off Rd & Midway Rd



HCM Signalized Intersection Capacity Analysis
5: Glades Cut-Off Rd & Midway Rd

Glades Cut Off Road Industrial
Existing Conditions, PM Peak

Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Lane Configurations														
Traffic Volume (vph)	6	153	725	14	41	635	111	33	64	93	3	181	44	207
Future Volume (vph)	6	153	725	14	41	635	111	33	64	93	3	181	44	207
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0
Lane Util. Factor		0.97	0.95		1.00	0.95		1.00	1.00	1.00		1.00	1.00	1.00
Frt		1.00	1.00		1.00	0.98		1.00	1.00	0.85		1.00	1.00	0.85
Flt Protected		0.95	1.00		0.95	1.00		0.95	1.00	1.00		0.95	1.00	1.00
Satd. Flow (prot)		3278	3364		1703	3330		1703	1792	1524		1658	1743	1482
Flt Permitted		0.95	1.00		0.95	1.00		0.73	1.00	1.00		0.47	1.00	1.00
Satd. Flow (perm)		3278	3364		1703	3330		1302	1792	1524		818	1743	1482
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	6	163	771	15	44	676	118	35	68	99	3	193	47	220
RTOR Reduction (vph)	0	0	1	0	0	12	0	0	85	0	0	0	0	173
Lane Group Flow (vph)	0	169	785	0	44	782	0	35	68	14	0	196	47	47
Heavy Vehicles (%)	2%	7%	7%	7%	6%	6%	6%	6%	6%	6%	2%	9%	9%	9%
Turn Type	Prot	Prot	NA		Prot	NA		pm+pt	NA	Perm	pm+pt	pm+pt	NA	Perm
Protected Phases	3	3	8		7	4		1	6		5	5	2	
Permitted Phases								6		6	2	2		2
Actuated Green, G (s)		15.4	43.8		8.5	36.9		19.4	15.4	15.4		35.5	23.5	23.5
Effective Green, g (s)		15.4	43.8		8.5	36.9		19.4	15.4	15.4		35.5	23.5	23.5
Actuated g/C Ratio		0.14	0.40		0.08	0.34		0.18	0.14	0.14		0.32	0.21	0.21
Clearance Time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0
Vehicle Extension (s)		5.0	5.0		5.0	5.0		5.0	5.0	5.0		5.0	5.0	5.0
Lane Grp Cap (vph)		459	1341		131	1119		244	251	213		357	373	317
v/s Ratio Prot		c0.05	c0.23		0.03	c0.23		0.01	0.04			c0.06	0.03	
v/s Ratio Perm								0.02		0.01		c0.12		0.03
v/c Ratio		0.37	0.59		0.34	0.70		0.14	0.27	0.07		0.55	0.13	0.15
Uniform Delay, d1		42.8	25.9		48.0	31.6		38.0	42.2	41.0		28.9	34.9	35.0
Progression Factor		1.00	1.00		1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2		1.0	1.0		3.2	2.4		0.6	1.2	0.3		3.0	0.3	0.5
Delay (s)		43.8	26.9		51.1	34.1		38.6	43.4	41.2		31.9	35.2	35.5
Level of Service		D	C		D	C		D	D	D		C	D	D
Approach Delay (s)			29.9			35.0			41.5					33.9
Approach LOS			C			C			D					C
Intersection Summary														
HCM 2000 Control Delay			33.3			HCM 2000 Level of Service								C
HCM 2000 Volume to Capacity ratio			0.64											
Actuated Cycle Length (s)			109.8			Sum of lost time (s)			30.0					
Intersection Capacity Utilization			77.2%			ICU Level of Service			D					
Analysis Period (min)			15											
c Critical Lane Group														

Lanes, Volumes, Timings
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Existing Conditions, PM Peak



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↑	↗	↖	↕	↕	↖	↕	↗	↖	↕	↗
Traffic Volume (vph)	1	1	640	409	370	644	1	180	0	299	0	1	6
Future Volume (vph)	1	1	640	409	370	644	1	180	0	299	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		325		0	300		300	250		0	100		0
Storage Lanes		1		1	1		1	1		0	1		0
Taper Length (ft)		25			25			25			25		
Right Turn on Red				Yes			Yes			Yes			Yes
Link Speed (mph)			45			45			30			30	
Link Distance (ft)			1488			1388			510			298	
Travel Time (s)			22.5			21.0			11.6			6.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%
Shared Lane Traffic (%)													
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	1	1	6		5	2		7	4		3	8	
Permitted Phases	6	6		6	2			4			8		
Detector Phase	1	1	6	6	5	2		7	4		3	8	
Switch Phase													
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0		15.0	15.0		15.0	15.0	
Total Split (s)	15.0	15.0	60.0	60.0	35.0	80.0		40.0	40.0		15.0	15.0	
Total Split (%)	10.0%	10.0%	40.0%	40.0%	23.3%	53.3%		26.7%	26.7%		10.0%	10.0%	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0		8.0	8.0	
Lead/Lag	Lead	Lead	Lag	Lag	Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min		None	None		None	None	

Intersection Summary

Area Type: Other

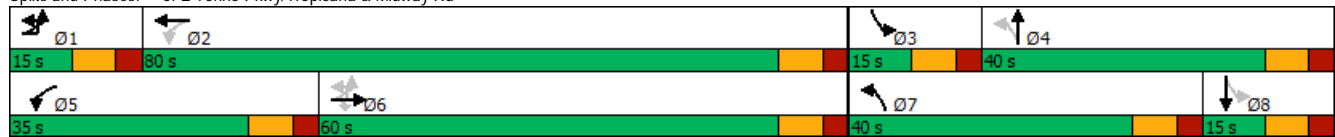
Cycle Length: 150

Actuated Cycle Length: 124.7

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Splits and Phases: 6: E Torino Pkwy/Tropicana & Midway Rd



HCM Signalized Intersection Capacity Analysis
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Existing Conditions, PM Peak



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↑	↗	↖	↕		↖	↕		↗	↔	
Traffic Volume (vph)	1	1	640	409	370	644	1	180	0	299	0	1	6
Future Volume (vph)	1	1	640	409	370	644	1	180	0	299	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0	
Lane Util. Factor		1.00	1.00	1.00	1.00	0.95		1.00	1.00			1.00	
Frt		1.00	1.00	0.85	1.00	1.00		1.00	0.85			1.00	0.87
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00	
Satd. Flow (prot)		1761	1845	1568	1736	3470		1770	1583			1415	
Flt Permitted		0.40	1.00	1.00	0.13	1.00		0.43	1.00			1.00	
Satd. Flow (perm)		734	1845	1568	231	3470		810	1583			1415	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	1	1	667	426	385	671	1	188	0	311	0	1	6
RTOR Reduction (vph)	0	0	0	154	0	0	0	0	249	0	0	6	0
Lane Group Flow (vph)	0	2	667	272	385	672	0	188	62	0	0	1	0
Heavy Vehicles (%)	2%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	1	1	6		5	2		7	4		3	8	
Permitted Phases	6	6		6	2			4			8		
Actuated Green, G (s)		60.2	59.0	59.0	94.2	85.0		27.6	27.6			1.2	
Effective Green, g (s)		60.2	59.0	59.0	94.2	85.0		27.6	27.6			1.2	
Actuated g/C Ratio		0.44	0.43	0.43	0.68	0.62		0.20	0.20			0.01	
Clearance Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0	
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0			3.0	
Lane Grp Cap (vph)		329	789	671	454	2140		290	317			12	
v/s Ratio Prot		0.00	0.36		c0.17	0.19		c0.09	0.04			0.00	
v/s Ratio Perm		0.00		0.17	c0.41			c0.04					
v/c Ratio		0.01	0.85	0.41	0.85	0.31		0.65	0.20			0.09	
Uniform Delay, d1		21.9	35.3	27.3	33.8	12.5		49.4	45.9			67.8	
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Incremental Delay, d2		0.0	8.3	0.4	13.7	0.1		4.9	0.3			3.1	
Delay (s)		21.9	43.6	27.7	47.5	12.6		54.3	46.2			70.9	
Level of Service		C	D	C	D	B		D	D			E	
Approach Delay (s)			37.4			25.3			49.2			70.9	
Approach LOS			D			C			D			E	
Intersection Summary													
HCM 2000 Control Delay			34.9			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio			0.87										
Actuated Cycle Length (s)			137.8			Sum of lost time (s)			32.0				
Intersection Capacity Utilization			92.7%			ICU Level of Service			F				
Analysis Period (min)			15										
c Critical Lane Group													

Future (2023) Background Conditions

Lanes, Volumes, Timings
1: Midway Rd & I-95 SB off-ramp

Glades Cut Off Road Industrial
Background Conditions, PM Peak



Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑		↓	↑↑					↑	↓	↑
Traffic Volume (vph)	0	159	182	3	721	291	0	0	0	0	502	0	42
Future Volume (vph)	0	159	182	3	721	291	0	0	0	0	502	0	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		500		400		0	0		0	0		400
Storage Lanes	0		1		1		0	0		0	1		1
Taper Length (ft)	25				25			25			25		
Right Turn on Red			Yes				Yes			Yes			Yes
Link Speed (mph)		45				45			45			45	
Link Distance (ft)		1345				1562			1124			796	
Travel Time (s)		20.4				23.7			17.0			12.1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%	3%	0%	0%	0%	9%	9%	9%
Shared Lane Traffic (%)											50%		
Turn Type		NA	Perm	Prot	Prot	NA					Perm	NA	Perm
Protected Phases		6		5	5	2						8	
Permitted Phases			6								8		8
Detector Phase		6	6	5	5	2					8	8	8
Switch Phase													
Minimum Initial (s)		8.0	8.0	7.0	7.0	8.0					6.0	6.0	6.0
Minimum Split (s)		16.0	16.0	15.0	15.0	16.0					13.0	13.0	13.0
Total Split (s)		18.0	18.0	56.0	56.0	74.0					26.0	26.0	26.0
Total Split (%)		18.0%	18.0%	56.0%	56.0%	74.0%					26.0%	26.0%	26.0%
Yellow Time (s)		5.0	5.0	5.0	5.0	5.0					4.0	4.0	4.0
All-Red Time (s)		3.0	3.0	3.0	3.0	3.0					3.0	3.0	3.0
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	0.0	0.0
Total Lost Time (s)		8.0	8.0		8.0	8.0					7.0	7.0	7.0
Lead/Lag		Lag	Lag	Lead	Lead								
Lead-Lag Optimize?		Yes	Yes	Yes	Yes								
Recall Mode		Min	Min	None	None	Min					None	None	None

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	97.2
Natural Cycle:	90
Control Type:	Actuated-Uncoordinated

Splits and Phases: 1: Midway Rd & I-95 SB off-ramp



HCM Signalized Intersection Capacity Analysis
1: Midway Rd & I-95 SB off-ramp

Glades Cut Off Road Industrial
Background Conditions, PM Peak



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↑↑	↑		↓	↑↑					↓	↓	↑	
Traffic Volume (vph)	0	159	182	3	721	291	0	0	0	0	502	0	42	
Future Volume (vph)	0	159	182	3	721	291	0	0	0	0	502	0	42	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		8.0	8.0		8.0	8.0					7.0	7.0	7.0	
Lane Util. Factor		0.95	1.00		1.00	0.95					0.95	0.95	1.00	
Frt		1.00	0.85		1.00	1.00					1.00	1.00	0.85	
Flt Protected		1.00	1.00		0.95	1.00					0.95	0.95	1.00	
Satd. Flow (prot)		3539	1583		1752	3505					1573	1573	1482	
Flt Permitted		1.00	1.00		0.95	1.00					0.95	0.95	1.00	
Satd. Flow (perm)		3539	1583		1752	3505					1573	1573	1482	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	173	198	3	784	316	0	0	0	0	546	0	46	
RTOR Reduction (vph)	0	0	179	0	0	0	0	0	0	0	0	0	37	
Lane Group Flow (vph)	0	173	19	0	787	316	0	0	0	0	273	273	9	
Heavy Vehicles (%)		2%	2%		2%	3%	3%	3%	0%	0%	9%	9%	9%	
Turn Type		NA	Perm	Prot	Prot	NA					Perm	NA	Perm	
Protected Phases		6		5	5	2						8		
Permitted Phases			6								8		8	
Actuated Green, G (s)		9.4	9.4		46.2	63.6					18.5	18.5	18.5	
Effective Green, g (s)		9.4	9.4		46.2	63.6					18.5	18.5	18.5	
Actuated g/C Ratio		0.10	0.10		0.48	0.65					0.19	0.19	0.19	
Clearance Time (s)		8.0	8.0		8.0	8.0					7.0	7.0	7.0	
Vehicle Extension (s)		3.0	3.0		5.0	3.0					3.0	3.0	3.0	
Lane Grp Cap (vph)		342	153		833	2295					299	299	282	
v/s Ratio Prot		c0.05			c0.45	0.09								
v/s Ratio Perm			0.01								c0.17	0.17	0.01	
v/c Ratio		0.51	0.13		0.94	0.14					0.91	0.91	0.03	
Uniform Delay, d1		41.6	40.1		24.2	6.4					38.5	38.5	32.0	
Progression Factor		1.00	1.00		1.00	1.00					1.00	1.00	1.00	
Incremental Delay, d2		1.2	0.4		19.5	0.0					30.4	30.4	0.0	
Delay (s)		42.8	40.5		43.7	6.4					68.9	68.9	32.0	
Level of Service		D	D		D	A					E	E	C	
Approach Delay (s)		41.6				33.0			0.0			66.1		
Approach LOS		D				C			A			E		
Intersection Summary														
HCM 2000 Control Delay			44.0		HCM 2000 Level of Service							D		
HCM 2000 Volume to Capacity ratio			0.88											
Actuated Cycle Length (s)			97.1		Sum of lost time (s)						23.0			
Intersection Capacity Utilization			84.5%		ICU Level of Service						E			
Analysis Period (min)			15											
c Critical Lane Group														

Lanes, Volumes, Timings
2: I-95 NB off-ramp & Midway Rd

Glades Cut Off Road Industrial
Background Conditions, PM Peak



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑			↑↑	↗	↘	↖	↗			
Traffic Volume (vph)	13	613	0	0	883	528	167	0	682	0	0	0
Future Volume (vph)	13	613	0	0	883	528	167	0	682	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	0		350	0		300	0		0
Storage Lanes	1		0	0		1	1		1	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1562			2097			847			779	
Travel Time (s)		23.7			31.8			12.8			11.8	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	6%	6%	6%	7%	7%	7%	7%	7%	7%	0%	0%	0%
Shared Lane Traffic (%)												
Turn Type	pm+pt	NA			NA	Perm	Perm	NA	Perm			
Protected Phases	1	6			2			4				
Permitted Phases	6					2	4		4			
Detector Phase	1	6			2	2	4	4	4			
Switch Phase												
Minimum Initial (s)	7.0	15.0			15.0	15.0	7.0	7.0	7.0			
Minimum Split (s)	15.0	23.0			23.0	23.0	15.0	15.0	15.0			
Total Split (s)	27.0	77.0			50.0	50.0	23.0	23.0	23.0			
Total Split (%)	27.0%	77.0%			50.0%	50.0%	23.0%	23.0%	23.0%			
Yellow Time (s)	5.0	5.0			5.0	5.0	5.0	5.0	5.0			
All-Red Time (s)	3.0	3.0			3.0	3.0	3.0	3.0	3.0			
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0	0.0			
Total Lost Time (s)	8.0	8.0			8.0	8.0	8.0	8.0	8.0			
Lead/Lag	Lead				Lag	Lag						
Lead-Lag Optimize?	Yes				Yes	Yes						
Recall Mode	None	Min			Min	Min	None	None	None			

Intersection Summary

Area Type: Other
 Cycle Length: 100
 Actuated Cycle Length: 61.8
 Natural Cycle: 90
 Control Type: Actuated-Uncoordinated

Splits and Phases: 2: I-95 NB off-ramp & Midway Rd



HCM 6th Signalized Intersection Summary
 2: I-95 NB off-ramp & Midway Rd

Glades Cut Off Road Industrial
 Background Conditions, PM Peak



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↕			↕	↕	↔	↔	↕			
Traffic Volume (veh/h)	13	613	0	0	883	528	167	0	682	0	0	0
Future Volume (veh/h)	13	613	0	0	883	528	167	0	682	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1811	1811	0	0	1796	1796	1796	1796	1796			
Adj Flow Rate, veh/h	14	645	0	0	929	0	176	0	0			
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95			
Percent Heavy Veh, %	6	6	0	0	7	7	7	7	7			
Cap, veh/h	277	1964	0	0	1342		422	0				
Arrive On Green	0.02	0.57	0.00	0.00	0.39	0.00	0.12	0.00	0.00			
Sat Flow, veh/h	1725	3532	0	0	3503	1522	3421	0	1522			
Grp Volume(v), veh/h	14	645	0	0	929	0	176	0	0			
Grp Sat Flow(s),veh/h/ln	1725	1721	0	0	1706	1522	1711	0	1522			
Q Serve(g_s), s	0.2	5.2	0.0	0.0	11.9	0.0	2.5	0.0	0.0			
Cycle Q Clear(g_c), s	0.2	5.2	0.0	0.0	11.9	0.0	2.5	0.0	0.0			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	277	1964	0	0	1342		422	0				
V/C Ratio(X)	0.05	0.33	0.00	0.00	0.69		0.42	0.00				
Avail Cap(c_a), veh/h	861	4538	0	0	2740		981	0				
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	1.00	1.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00			
Uniform Delay (d), s/veh	9.7	5.9	0.0	0.0	13.2	0.0	21.2	0.0	0.0			
Incr Delay (d2), s/veh	0.1	0.1	0.0	0.0	0.6	0.0	0.7	0.0	0.0			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	0.1	2.0	0.0	0.0	6.3	0.0	1.6	0.0	0.0			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	9.8	6.0	0.0	0.0	13.9	0.0	21.8	0.0	0.0			
LnGrp LOS	A	A	A	A	B		C	A				
Approach Vol, veh/h		659			929	A		176	A			
Approach Delay, s/veh		6.1			13.9			21.8				
Approach LOS		A			B			C				
Timer - Assigned Phs	1	2		4		6						
Phs Duration (G+Y+Rc), s	9.3	28.6		14.5		37.9						
Change Period (Y+Rc), s	8.0	8.0		8.0		8.0						
Max Green Setting (Gmax), s	19.0	42.0		15.0		69.0						
Max Q Clear Time (g_c+I1), s	2.2	13.9		4.5		7.2						
Green Ext Time (p_c), s	0.0	6.7		0.4		4.5						

Intersection Summary

HCM 6th Ctrl Delay	11.8
HCM 6th LOS	B

Notes

User approved volume balancing among the lanes for turning movement.
 Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lanes, Volumes, Timings
3: LTC Pkwy & Midway Rd

Glades Cut Off Road Industrial
Background Conditions, PM Peak

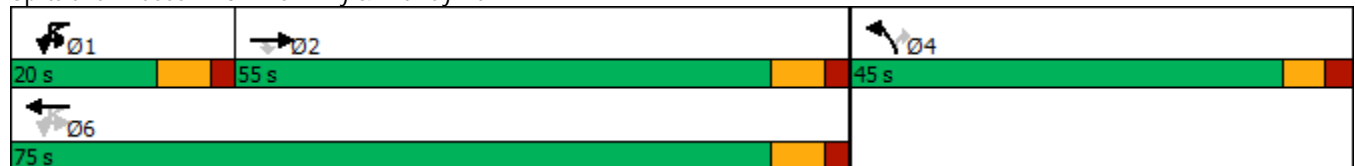


Lane Group	EBT	EBR	WBU	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↓	↑↑	↑↓	↑
Traffic Volume (vph)	935	270	4	263	909	501	311
Future Volume (vph)	935	270	4	263	909	501	311
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		550		400		0	175
Storage Lanes		1		1		2	1
Taper Length (ft)				25		25	
Right Turn on Red		Yes					Yes
Link Speed (mph)	35				45	30	
Link Distance (ft)	2097				1746	617	
Travel Time (s)	40.9				26.5	14.0	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	8%	8%	7%	7%	7%	7%	7%
Shared Lane Traffic (%)							
Turn Type	NA	Perm	pm+pt	pm+pt	NA	Prot	Perm
Protected Phases	2		1	1	6	4	
Permitted Phases		2	6	6			4
Detector Phase	2	2	1	1	6	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	25.2	25.2	11.9	11.9	25.2	11.4	11.4
Total Split (s)	55.0	55.0	20.0	20.0	75.0	45.0	45.0
Total Split (%)	45.8%	45.8%	16.7%	16.7%	62.5%	37.5%	37.5%
Yellow Time (s)	4.8	4.8	4.8	4.8	4.8	3.7	3.7
All-Red Time (s)	2.4	2.4	2.1	2.1	2.4	2.7	2.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2		6.9	7.2	6.4	6.4
Lead/Lag	Lag	Lag	Lead	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes	Yes			
Recall Mode	Min	Min	None	None	Min	None	None

Intersection Summary

Area Type: Other
 Cycle Length: 120
 Actuated Cycle Length: 90.9
 Natural Cycle: 70
 Control Type: Actuated-Uncoordinated

Splits and Phases: 3: LTC Pkwy & Midway Rd



HCM Signalized Intersection Capacity Analysis

3: LTC Pkwy & Midway Rd

Glades Cut Off Road Industrial
Background Conditions, PM Peak



Movement	EBT	EBR	WBU	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↓	↑↑	↑↓	↑
Traffic Volume (vph)	935	270	4	263	909	501	311
Future Volume (vph)	935	270	4	263	909	501	311
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.2	7.2		6.9	7.2	6.4	6.4
Lane Util. Factor	0.95	1.00		1.00	0.95	0.97	1.00
Frt	1.00	0.85		1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00		0.95	1.00	0.95	1.00
Satd. Flow (prot)	3343	1495		1687	3374	3273	1509
Flt Permitted	1.00	1.00		0.14	1.00	0.95	1.00
Satd. Flow (perm)	3343	1495		257	3374	3273	1509
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	974	281	4	274	947	522	324
RTOR Reduction (vph)	0	171	0	0	0	0	199
Lane Group Flow (vph)	974	110	0	278	947	522	125
Heavy Vehicles (%)	8%	8%	7%	7%	7%	7%	7%
Turn Type	NA	Perm	pm+pt	pm+pt	NA	Prot	Perm
Protected Phases	2		1	1	6	4	
Permitted Phases		2	6	6			4
Actuated Green, G (s)	35.6	35.6		55.8	55.8	21.2	21.2
Effective Green, g (s)	35.6	35.6		55.8	55.8	21.2	21.2
Actuated g/C Ratio	0.39	0.39		0.62	0.62	0.23	0.23
Clearance Time (s)	7.2	7.2		6.9	7.2	6.4	6.4
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1313	587		368	2078	765	353
v/s Ratio Prot	0.29			c0.11	0.28	c0.16	
v/s Ratio Perm		0.07		c0.35			0.08
v/c Ratio	0.74	0.19		0.76	0.46	0.68	0.35
Uniform Delay, d1	23.6	18.0		14.9	9.3	31.6	29.0
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2	2.3	0.2		8.6	0.2	2.5	0.6
Delay (s)	25.9	18.2		23.5	9.5	34.2	29.6
Level of Service	C	B		C	A	C	C
Approach Delay (s)	24.1				12.6	32.4	
Approach LOS	C				B	C	

Intersection Summary

HCM 2000 Control Delay	22.0	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.77		
Actuated Cycle Length (s)	90.6	Sum of lost time (s)	20.5
Intersection Capacity Utilization	77.0%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

Lanes, Volumes, Timings
5: Glades Cut-Off Rd & Midway Rd

Glades Cut Off Road Industrial
Background Conditions, PM Peak

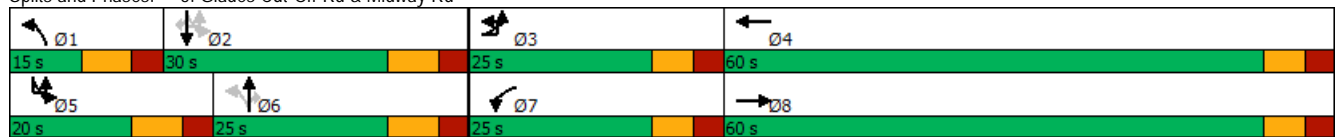


Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Lane Configurations		↔↔	↕↕		↔	↕↕		↔	↕	↕↕		↔↔	↕	↕↕
Traffic Volume (vph)	6	202	988	15	44	861	119	42	69	100	3	194	47	254
Future Volume (vph)	6	202	988	15	44	861	119	42	69	100	3	194	47	254
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		625		0	400		0	325		325		400		275
Storage Lanes		2		0	1		0	1		1		1		1
Taper Length (ft)		25			25			25				25		
Right Turn on Red				Yes			Yes			Yes				Yes
Link Speed (mph)			50			50			50				50	
Link Distance (ft)			894			1488			909				1361	
Travel Time (s)			12.2			20.3			12.4				18.6	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	7%	7%	7%	7%	6%	6%	6%	6%	6%	6%	9%	9%	9%	9%
Shared Lane Traffic (%)														
Turn Type	Prot	Prot	NA		Prot	NA		pm+pt	NA	Perm	pm+pt	pm+pt	NA	Perm
Protected Phases	3	3	8		7	4		1	6		5	5	2	
Permitted Phases								6		6	2	2		2
Detector Phase	3	3	8		7	4		1	6	6	5	5	2	2
Switch Phase														
Minimum Initial (s)	15.0	15.0	7.0		15.0	7.0		7.0	10.0	10.0	7.0	7.0	10.0	10.0
Minimum Split (s)	22.0	22.0	25.0		22.0	14.0		15.0	18.0	18.0	18.0	18.0	18.0	18.0
Total Split (s)	25.0	25.0	60.0		25.0	60.0		15.0	25.0	25.0	20.0	20.0	30.0	30.0
Total Split (%)	19.2%	19.2%	46.2%		19.2%	46.2%		11.5%	19.2%	19.2%	15.4%	15.4%	23.1%	23.1%
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)		0.0	0.0		0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0
Lead/Lag	Lead	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	Min		None	Min		None	None	None	None	None	None	None

Intersection Summary

Area Type:	Other
Cycle Length:	130
Actuated Cycle Length:	117.2
Natural Cycle:	85
Control Type:	Actuated-Uncoordinated

Splits and Phases: 5: Glades Cut-Off Rd & Midway Rd



HCM Signalized Intersection Capacity Analysis
5: Glades Cut-Off Rd & Midway Rd

Glades Cut Off Road Industrial
Background Conditions, PM Peak

Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations															
Traffic Volume (vph)	6	202	988	15	44	861	119	42	69	100	3	194	47	254	
Future Volume (vph)	6	202	988	15	44	861	119	42	69	100	3	194	47	254	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0	
Lane Util. Factor		0.97	0.95		1.00	0.95		1.00	1.00	1.00		1.00	1.00	1.00	
Frt		1.00	1.00		1.00	0.98		1.00	1.00	0.85		1.00	1.00	0.85	
Flt Protected		0.95	1.00		0.95	1.00		0.95	1.00	1.00		0.95	1.00	1.00	
Satd. Flow (prot)		3273	3366		1703	3343		1703	1792	1524		1656	1743	1482	
Flt Permitted		0.95	1.00		0.95	1.00		0.72	1.00	1.00		0.48	1.00	1.00	
Satd. Flow (perm)		3273	3366		1703	3343		1298	1792	1524		842	1743	1482	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	
Adj. Flow (vph)	6	215	1051	16	47	916	127	45	73	106	3	206	50	270	
RTOR Reduction (vph)	0	0	1	0	0	8	0	0	0	93	0	0	0	223	
Lane Group Flow (vph)	0	221	1066	0	47	1035	0	45	73	13	0	209	50	47	
Heavy Vehicles (%)	7%	7%	7%	7%	6%	6%	6%	6%	6%	6%	9%	9%	9%	9%	
Turn Type	Prot	Prot	NA		Prot	NA		pm+pt	NA	Perm	pm+pt	pm+pt	NA	Perm	
Protected Phases	3	3	8		7	4		1	6		5	5	2		
Permitted Phases								6		6	2	2		2	
Actuated Green, G (s)		16.1	52.3		11.7	47.9		19.7	14.3	14.3		33.1	21.0	21.0	
Effective Green, g (s)		16.1	52.3		11.7	47.9		19.7	14.3	14.3		33.1	21.0	21.0	
Actuated g/C Ratio		0.13	0.43		0.10	0.40		0.16	0.12	0.12		0.27	0.17	0.17	
Clearance Time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0	
Vehicle Extension (s)		5.0	5.0		5.0	5.0		5.0	5.0	5.0		5.0	5.0	5.0	
Lane Grp Cap (vph)		437	1462		165	1329		230	212	181		313	304	258	
v/s Ratio Prot		c0.07	c0.32		0.03	c0.31		0.01	0.04			c0.07	0.03		
v/s Ratio Perm								0.02		0.01		c0.12		0.03	
v/c Ratio		0.51	0.73		0.28	0.78		0.20	0.34	0.07		0.67	0.16	0.18	
Uniform Delay, d1		48.5	28.2		50.5	31.6		43.3	48.7	47.1		36.7	42.2	42.4	
Progression Factor		1.00	1.00		1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Incremental Delay, d2		1.9	2.3		2.0	3.4		0.9	2.0	0.3		7.0	0.5	0.7	
Delay (s)		50.4	30.5		52.5	35.1		44.1	50.8	47.5		43.7	42.8	43.1	
Level of Service		D	C		D	D		D	D	D		D	D	D	
Approach Delay (s)			33.9			35.8			47.9					43.3	
Approach LOS			C			D			D					D	
Intersection Summary															
HCM 2000 Control Delay			37.2											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio			0.75												
Actuated Cycle Length (s)			120.4							30.0					
Intersection Capacity Utilization			86.7%											ICU Level of Service	E
Analysis Period (min)			15												
c Critical Lane Group															

Lanes, Volumes, Timings
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Background Conditions, PM Peak



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↘	↑	↗	↘	↗	↗	↘	↗		↘	↗	
Traffic Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6
Future Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		325		300	300		300	250		0	100		0
Storage Lanes		1		1	1		1	1		0	1		0
Taper Length (ft)		25			25			25			25		
Right Turn on Red				Yes			Yes			Yes			Yes
Link Speed (mph)			45			45			30			30	
Link Distance (ft)			1488			1388			510			298	
Travel Time (s)			22.5			21.0			11.6			6.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%
Shared Lane Traffic (%)													
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	1	1	6		5	2		7	4		3	8	
Permitted Phases	6	6		6	2			4			8		
Detector Phase	1	1	6	6	5	2		7	4		3	8	
Switch Phase													
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0		15.0	15.0		15.0	15.0	
Total Split (s)	15.0	15.0	60.0	60.0	35.0	80.0		40.0	40.0		15.0	15.0	
Total Split (%)	10.0%	10.0%	40.0%	40.0%	23.3%	53.3%		26.7%	26.7%		10.0%	10.0%	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0		8.0	8.0	
Lead/Lag	Lead	Lead	Lag	Lag	Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min		None	None		None	None	

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	127.6
Natural Cycle:	150
Control Type:	Actuated-Uncoordinated

Splits and Phases: 6: E Torino Pkwy/Tropicana & Midway Rd



HCM Signalized Intersection Capacity Analysis
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Background Conditions, PM Peak



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations														
Traffic Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6	
Future Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6	
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0		
Lane Util. Factor		1.00	1.00	1.00	1.00	0.95		1.00	1.00			1.00		
Frt		1.00	1.00	0.85	1.00	1.00		1.00	0.85			0.87		
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00		
Satd. Flow (prot)		1752	1845	1568	1736	3471		1770	1583			1415		
Flt Permitted		0.32	1.00	1.00	0.06	1.00		0.43	1.00			1.00		
Satd. Flow (perm)		592	1845	1568	109	3471		810	1583			1415		
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	
Adj. Flow (vph)	1	1	909	482	412	885	1	224	0	333	0	1	6	
RTOR Reduction (vph)	0	0	0	156	0	0	0	0	261	0	0	6	0	
Lane Group Flow (vph)	0	2	909	326	413	886	0	224	72	0	0	1	0	
Heavy Vehicles (%)	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%	
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA		
Protected Phases	1	1	6		5	2		7	4		3	8		
Permitted Phases	6	6		6	2			4			8			
Actuated Green, G (s)		60.3	59.1	59.1	94.3	85.1		30.5	30.5			1.2		
Effective Green, g (s)		60.3	59.1	59.1	94.3	85.1		30.5	30.5			1.2		
Actuated g/C Ratio		0.43	0.42	0.42	0.67	0.60		0.22	0.22			0.01		
Clearance Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0		
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0			3.0		
Lane Grp Cap (vph)		263	774	658	387	2097		320	342			12		
v/s Ratio Prot		0.00	c0.49		c0.21	0.26		c0.11	0.05			0.00		
v/s Ratio Perm		0.00		0.21	0.51			c0.05						
v/c Ratio		0.01	1.17	0.50	1.07	0.42		0.70	0.21			0.09		
Uniform Delay, d1		23.0	40.9	29.9	48.5	14.8		49.6	45.3			69.3		
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00		
Incremental Delay, d2		0.0	91.9	0.6	64.7	0.1		6.6	0.3			3.1		
Delay (s)		23.1	132.7	30.5	113.2	14.9		56.1	45.6			72.4		
Level of Service		C	F	C	F	B		E	D			E		
Approach Delay (s)			97.2			46.2			49.8			72.4		
Approach LOS			F			D			D			E		
Intersection Summary														
HCM 2000 Control Delay			68.7										HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio			1.08											
Actuated Cycle Length (s)			140.8										Sum of lost time (s)	32.0
Intersection Capacity Utilization			107.7%										ICU Level of Service	G
Analysis Period (min)			15											
c Critical Lane Group														

**Future (2023) Background Conditions with
Improvements**

Lanes, Volumes, Timings
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Background Conditions w/ Improvements, PM Peak



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↕	↗	↖	↕		↖	↕		↗	↕	
Traffic Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6
Future Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		325		300	300		300	250		0	100		0
Storage Lanes		1		1	1		1	1		0	1		0
Taper Length (ft)		25			25			25			25		
Right Turn on Red				Yes			Yes			Yes			Yes
Link Speed (mph)			45			45			30			30	
Link Distance (ft)			1488			1388			510			298	
Travel Time (s)			22.5			21.0			11.6			6.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%
Shared Lane Traffic (%)													
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	1	1	6		5	2		7	4		3	8	
Permitted Phases	6	6		6	2			4			8		
Detector Phase	1	1	6	6	5	2		7	4		3	8	
Switch Phase													
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0		15.0	15.0		15.0	15.0	
Total Split (s)	15.0	15.0	70.0	70.0	40.0	95.0		25.0	25.0		15.0	15.0	
Total Split (%)	10.0%	10.0%	46.7%	46.7%	26.7%	63.3%		16.7%	16.7%		10.0%	10.0%	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0		8.0	8.0	
Lead/Lag	Lead	Lead	Lag	Lag	Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min		None	None		None	None	

Intersection Summary

Area Type: Other

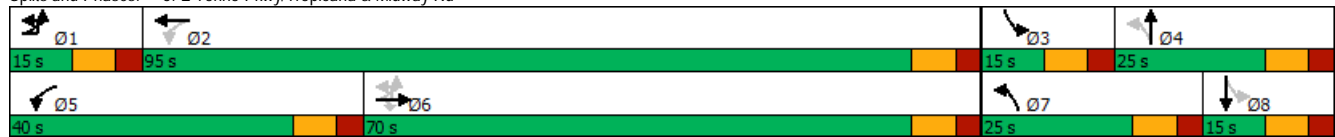
Cycle Length: 150

Actuated Cycle Length: 109.8

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Splits and Phases: 6: E Torino Pkwy/Tropicana & Midway Rd



HCM Signalized Intersection Capacity Analysis
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Background Conditions w/ Improvements, PM Peak



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↕	↕	↕	↕	↕	↕	↕	↕	↕	↕	↕
Traffic Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6
Future Volume (vph)	1	1	873	463	396	850	1	215	0	320	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0	
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95		1.00	1.00			1.00	
Frt		1.00	1.00	0.85	1.00	1.00		1.00	0.85			0.87	
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00	
Satd. Flow (prot)		1752	3505	1568	1736	3471		1770	1583			1415	
Flt Permitted		0.32	1.00	1.00	0.14	1.00		0.44	1.00			1.00	
Satd. Flow (perm)		592	3505	1568	261	3471		819	1583			1415	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	1	1	909	482	412	885	1	224	0	333	0	1	6
RTOR Reduction (vph)	0	0	0	309	0	0	0	0	264	0	0	6	0
Lane Group Flow (vph)	0	2	909	173	413	886	0	224	69	0	0	1	0
Heavy Vehicles (%)	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	1	1	6	5	2	2		7	4		3	8	
Permitted Phases	6	6		6	2			4			8		
Actuated Green, G (s)		45.3	44.2	44.2	81.2	72.1		25.6	25.6			1.1	
Effective Green, g (s)		45.3	44.2	44.2	81.2	72.1		25.6	25.6			1.1	
Actuated g/C Ratio		0.37	0.36	0.36	0.66	0.59		0.21	0.21			0.01	
Clearance Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0	
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0			3.0	
Lane Grp Cap (vph)		228	1261	564	520	2037		298	330			12	
v/s Ratio Prot		0.00	0.26		c0.19	0.26		c0.10	0.04			0.00	
v/s Ratio Perm		0.00		0.11	c0.34			c0.06					
w/c Ratio		0.01	0.72	0.31	0.79	0.43		0.75	0.21			0.09	
Uniform Delay, d1		24.5	34.0	28.3	26.2	14.1		44.2	40.2			60.4	
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Incremental Delay, d2		0.0	2.1	0.3	8.2	0.1		10.2	0.3			3.2	
Delay (s)		24.5	36.0	28.6	34.4	14.2		54.4	40.6			63.5	
Level of Service		C	D	C	C	B		D	D			E	
Approach Delay (s)			33.4			20.6			46.1			63.5	
Approach LOS			C			C			D			E	
Intersection Summary													
HCM 2000 Control Delay			30.6			HCM 2000 Level of Service			C				
HCM 2000 Volume to Capacity ratio			0.86										
Actuated Cycle Length (s)			122.8			Sum of lost time (s)			32.0				
Intersection Capacity Utilization			85.9%			ICU Level of Service			E				
Analysis Period (min)			15										
c Critical Lane Group													

Future (2023) Buildout Conditions

Lanes, Volumes, Timings
1: Midway Rd & I-95 SB off-ramp

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Lane Group	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↑↑	↑		↓	↑↑					↓	↓	↑
Traffic Volume (vph)	0	161	182	3	731	293	0	0	0	0	509	0	42
Future Volume (vph)	0	161	182	3	731	293	0	0	0	0	509	0	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		500		400		0	0		0	400		0
Storage Lanes	0		1		1		0	0		0	1		1
Taper Length (ft)	25				25			25			25		
Right Turn on Red			Yes				Yes			Yes			Yes
Link Speed (mph)		45				45			45			45	
Link Distance (ft)		1345				1562			1124			796	
Travel Time (s)		20.4				23.7			17.0			12.1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	3%	3%	3%	3%	0%	0%	0%	9%	9%	9%
Shared Lane Traffic (%)											50%		
Turn Type		NA	Perm	Prot	Prot	NA					Perm	NA	Perm
Protected Phases		6		5	5	2						8	
Permitted Phases			6								8		8
Detector Phase		6	6	5	5	2					8	8	8
Switch Phase													
Minimum Initial (s)		8.0	8.0	7.0	7.0	8.0					6.0	6.0	6.0
Minimum Split (s)		16.0	16.0	15.0	15.0	16.0					13.0	13.0	13.0
Total Split (s)		18.0	18.0	56.0	56.0	74.0					26.0	26.0	26.0
Total Split (%)		18.0%	18.0%	56.0%	56.0%	74.0%					26.0%	26.0%	26.0%
Yellow Time (s)		5.0	5.0	5.0	5.0	5.0					4.0	4.0	4.0
All-Red Time (s)		3.0	3.0	3.0	3.0	3.0					3.0	3.0	3.0
Lost Time Adjust (s)		0.0	0.0		0.0	0.0					0.0	0.0	0.0
Total Lost Time (s)		8.0	8.0		8.0	8.0					7.0	7.0	7.0
Lead/Lag		Lag	Lag	Lead	Lead								
Lead-Lag Optimize?		Yes	Yes	Yes	Yes								
Recall Mode		Min	Min	None	None	Min					None	None	None

Intersection Summary

Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	97.9
Natural Cycle:	90
Control Type:	Actuated-Uncoordinated

Splits and Phases: 1: Midway Rd & I-95 SB off-ramp



HCM Signalized Intersection Capacity Analysis
1: Midway Rd & I-95 SB off-ramp

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↑↑	↑		↓	↑↑					↓	↓	↑	
Traffic Volume (vph)	0	161	182	3	731	293	0	0	0	0	509	0	42	
Future Volume (vph)	0	161	182	3	731	293	0	0	0	0	509	0	42	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		8.0	8.0		8.0	8.0					7.0	7.0	7.0	
Lane Util. Factor		0.95	1.00		1.00	0.95					0.95	0.95	1.00	
Frt		1.00	0.85		1.00	1.00					1.00	1.00	0.85	
Flt Protected		1.00	1.00		0.95	1.00					0.95	0.95	1.00	
Satd. Flow (prot)		3539	1583		1752	3505					1573	1573	1482	
Flt Permitted		1.00	1.00		0.95	1.00					0.95	0.95	1.00	
Satd. Flow (perm)		3539	1583		1752	3505					1573	1573	1482	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	175	198	3	795	318	0	0	0	0	553	0	46	
RTOR Reduction (vph)	0	0	179	0	0	0	0	0	0	0	0	0	37	
Lane Group Flow (vph)	0	175	19	0	798	318	0	0	0	0	276	277	9	
Heavy Vehicles (%)	2%	2%	2%	3%	3%	3%	3%	0%	0%	0%	9%	9%	9%	
Turn Type		NA	Perm	Prot	Prot	NA					Perm	NA	Perm	
Protected Phases		6		5	5	2						8		
Permitted Phases			6								8		8	
Actuated Green, G (s)		9.4	9.4		46.7	64.1					18.7	18.7	18.7	
Effective Green, g (s)		9.4	9.4		46.7	64.1					18.7	18.7	18.7	
Actuated g/C Ratio		0.10	0.10		0.48	0.66					0.19	0.19	0.19	
Clearance Time (s)		8.0	8.0		8.0	8.0					7.0	7.0	7.0	
Vehicle Extension (s)		3.0	3.0		5.0	3.0					3.0	3.0	3.0	
Lane Grp Cap (vph)		340	152		836	2297					300	300	283	
v/s Ratio Prot		c0.05			c0.46	0.09								
v/s Ratio Perm			0.01								0.18	0.18	0.01	
v/c Ratio		0.51	0.13		0.95	0.14					0.92	0.92	0.03	
Uniform Delay, d1		42.0	40.4		24.5	6.4					38.8	38.8	32.2	
Progression Factor		1.00	1.00		1.00	1.00					1.00	1.00	1.00	
Incremental Delay, d2		1.3	0.4		21.1	0.0					31.9	32.5	0.0	
Delay (s)		43.3	40.8		45.7	6.4					70.7	71.3	32.2	
Level of Service		D	D		D	A					E	E	C	
Approach Delay (s)		42.0				34.5			0.0			68.0		
Approach LOS		D				C			A			E		
Intersection Summary														
HCM 2000 Control Delay			45.4		HCM 2000 Level of Service							D		
HCM 2000 Volume to Capacity ratio			0.89											
Actuated Cycle Length (s)			97.8		Sum of lost time (s)						23.0			
Intersection Capacity Utilization			85.2%		ICU Level of Service							E		
Analysis Period (min)			15											
c Critical Lane Group														

Lanes, Volumes, Timings
2: I-95 NB off-ramp & Midway Rd

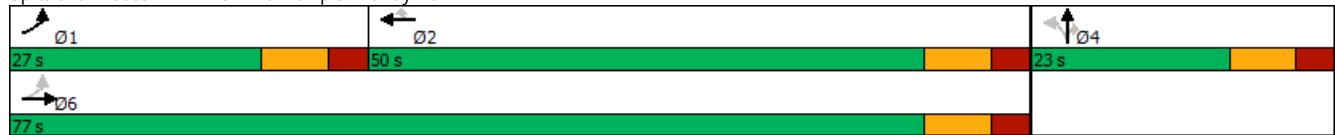
Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑			↑↑	↗	↘	↖	↗			
Traffic Volume (vph)	13	622	0	0	895	549	167	0	685	0	0	0
Future Volume (vph)	13	622	0	0	895	549	167	0	685	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	0		350	300		0	0		0
Storage Lanes	1		0	0		1	1		1	0		0
Taper Length (ft)	25			25			25			25		
Right Turn on Red			Yes			Yes			Yes			Yes
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1562			2097			847			779	
Travel Time (s)		23.7			31.8			12.8			11.8	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	6%	6%	6%	7%	7%	7%	7%	7%	7%	0%	0%	0%
Shared Lane Traffic (%)												
Turn Type	pm+pt	NA			NA	Perm	Perm	NA	Perm			
Protected Phases	1	6			2			4				
Permitted Phases	6					2	4		4			
Detector Phase	1	6			2	2	4	4	4			
Switch Phase												
Minimum Initial (s)	7.0	15.0			15.0	15.0	7.0	7.0	7.0			
Minimum Split (s)	15.0	23.0			23.0	23.0	15.0	15.0	15.0			
Total Split (s)	27.0	77.0			50.0	50.0	23.0	23.0	23.0			
Total Split (%)	27.0%	77.0%			50.0%	50.0%	23.0%	23.0%	23.0%			
Yellow Time (s)	5.0	5.0			5.0	5.0	5.0	5.0	5.0			
All-Red Time (s)	3.0	3.0			3.0	3.0	3.0	3.0	3.0			
Lost Time Adjust (s)	0.0	0.0			0.0	0.0	0.0	0.0	0.0			
Total Lost Time (s)	8.0	8.0			8.0	8.0	8.0	8.0	8.0			
Lead/Lag	Lead				Lag	Lag						
Lead-Lag Optimize?	Yes				Yes	Yes						
Recall Mode	None	Min			Min	Min	None	None	None			

Intersection Summary	
Area Type:	Other
Cycle Length:	100
Actuated Cycle Length:	61.9
Natural Cycle:	90
Control Type:	Actuated-Uncoordinated

Splits and Phases: 2: I-95 NB off-ramp & Midway Rd



HCM 6th Signalized Intersection Summary
2: I-95 NB off-ramp & Midway Rd

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗			↖	↗	↖	↗	↗			
Traffic Volume (veh/h)	13	622	0	0	895	549	167	0	685	0	0	0
Future Volume (veh/h)	13	622	0	0	895	549	167	0	685	0	0	0
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0			
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00			
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Work Zone On Approach		No			No			No				
Adj Sat Flow, veh/h/ln	1811	1811	0	0	1796	1796	1796	1796	1796			
Adj Flow Rate, veh/h	14	655	0	0	942	0	176	0	0			
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95			
Percent Heavy Veh, %	6	6	0	0	7	7	7	7	7			
Cap, veh/h	275	1973	0	0	1355		420	0				
Arrive On Green	0.02	0.57	0.00	0.00	0.40	0.00	0.12	0.00	0.00			
Sat Flow, veh/h	1725	3532	0	0	3503	1522	3421	0	1522			
Grp Volume(v), veh/h	14	655	0	0	942	0	176	0	0			
Grp Sat Flow(s),veh/h/ln	1725	1721	0	0	1706	1522	1711	0	1522			
Q Serve(g_s), s	0.2	5.3	0.0	0.0	12.1	0.0	2.5	0.0	0.0			
Cycle Q Clear(g_c), s	0.2	5.3	0.0	0.0	12.1	0.0	2.5	0.0	0.0			
Prop In Lane	1.00		0.00	0.00		1.00	1.00		1.00			
Lane Grp Cap(c), veh/h	275	1973	0	0	1355		420	0				
V/C Ratio(X)	0.05	0.33	0.00	0.00	0.70		0.42	0.00				
Avail Cap(c_a), veh/h	855	4507	0	0	2721		974	0				
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Upstream Filter(I)	1.00	1.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00			
Uniform Delay (d), s/veh	9.7	5.9	0.0	0.0	13.2	0.0	21.4	0.0	0.0			
Incr Delay (d2), s/veh	0.1	0.1	0.0	0.0	0.7	0.0	0.7	0.0	0.0			
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(95%),veh/ln	0.1	2.0	0.0	0.0	6.4	0.0	1.6	0.0	0.0			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	9.8	6.0	0.0	0.0	13.9	0.0	22.0	0.0	0.0			
LnGrp LOS	A	A	A	A	B		C	A				
Approach Vol, veh/h		669			942	A		176	A			
Approach Delay, s/veh		6.1			13.9			22.0				
Approach LOS		A			B			C				
Timer - Assigned Phs	1	2		4		6						
Phs Duration (G+Y+Rc), s	9.3	28.9		14.5		38.2						
Change Period (Y+Rc), s	8.0	8.0		8.0		8.0						
Max Green Setting (Gmax), s	19.0	42.0		15.0		69.0						
Max Q Clear Time (g_c+I1), s	2.2	14.1		4.5		7.3						
Green Ext Time (p_c), s	0.0	6.8		0.4		4.6						

Intersection Summary

HCM 6th Ctrl Delay	11.8
HCM 6th LOS	B

Notes

User approved volume balancing among the lanes for turning movement.
Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lanes, Volumes, Timings
3: LTC Pkwy & Midway Rd

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Lane Group	EBT	EBR	WBU	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↓	↑↑	↑↑	↑
Traffic Volume (vph)	941	276	4	265	909	534	318
Future Volume (vph)	941	276	4	265	909	534	318
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		550		400		0	175
Storage Lanes		1		1		2	1
Taper Length (ft)				25		25	
Right Turn on Red		Yes					Yes
Link Speed (mph)	35				45	30	
Link Distance (ft)	2097				1746	243	
Travel Time (s)	40.9				26.5	5.5	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	8%	8%	7%	7%	7%	7%	7%
Shared Lane Traffic (%)							
Turn Type	NA	Perm	pm+pt	pm+pt	NA	Prot	Perm
Protected Phases	2		1	1	6	4	
Permitted Phases		2	6	6			4
Detector Phase	2	2	1	1	6	4	4
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	25.2	25.2	11.9	11.9	25.2	11.4	11.4
Total Split (s)	55.0	55.0	20.0	20.0	75.0	45.0	45.0
Total Split (%)	45.8%	45.8%	16.7%	16.7%	62.5%	37.5%	37.5%
Yellow Time (s)	4.8	4.8	4.8	4.8	4.8	3.7	3.7
All-Red Time (s)	2.4	2.4	2.1	2.1	2.4	2.7	2.7
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	7.2	7.2		6.9	7.2	6.4	6.4
Lead/Lag	Lag	Lag	Lead	Lead			
Lead-Lag Optimize?	Yes	Yes	Yes	Yes			
Recall Mode	Min	Min	None	None	Min	None	None

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 93

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Splits and Phases: 3: LTC Pkwy & Midway Rd



HCM Signalized Intersection Capacity Analysis
3: LTC Pkwy & Midway Rd

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Movement	EBT	EBR	WBU	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↑		↓	↑↑	↑↓	↑
Traffic Volume (vph)	941	276	4	265	909	534	318
Future Volume (vph)	941	276	4	265	909	534	318
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	7.2	7.2		6.9	7.2	6.4	6.4
Lane Util. Factor	0.95	1.00		1.00	0.95	0.97	1.00
Frt	1.00	0.85		1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00		0.95	1.00	0.95	1.00
Satd. Flow (prot)	3343	1495		1687	3374	3273	1509
Flt Permitted	1.00	1.00		0.14	1.00	0.95	1.00
Satd. Flow (perm)	3343	1495		249	3374	3273	1509
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	980	288	4	276	947	556	331
RTOR Reduction (vph)	0	176	0	0	0	0	196
Lane Group Flow (vph)	980	112	0	280	947	556	135
Heavy Vehicles (%)	8%	8%	7%	7%	7%	7%	7%
Turn Type	NA	Perm	pm+pt	pm+pt	NA	Prot	Perm
Protected Phases	2		1	1	6	4	
Permitted Phases		2	6	6			4
Actuated Green, G (s)	36.2	36.2		56.5	56.5	22.6	22.6
Effective Green, g (s)	36.2	36.2		56.5	56.5	22.6	22.6
Actuated g/C Ratio	0.39	0.39		0.61	0.61	0.24	0.24
Clearance Time (s)	7.2	7.2		6.9	7.2	6.4	6.4
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	1305	583		359	2056	797	367
v/s Ratio Prot	0.29			c0.11	0.28	c0.17	
v/s Ratio Perm		0.08		c0.36			0.09
v/c Ratio	0.75	0.19		0.78	0.46	0.70	0.37
Uniform Delay, d1	24.4	18.6		16.4	9.8	31.9	29.1
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00
Incremental Delay, d2	2.5	0.2		10.3	0.2	2.7	0.6
Delay (s)	26.8	18.8		26.7	10.0	34.6	29.7
Level of Service	C	B		C	A	C	C
Approach Delay (s)	25.0				13.8	32.8	
Approach LOS	C				B	C	
Intersection Summary							
HCM 2000 Control Delay			23.0		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio			0.79				
Actuated Cycle Length (s)			92.7		Sum of lost time (s)		20.5
Intersection Capacity Utilization			77.7%		ICU Level of Service		D
Analysis Period (min)			15				
c Critical Lane Group							

Lanes, Volumes, Timings
5: Glades Cut-Off Rd & Midway Rd

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Lane Configurations		↔↔	↕↕		↔	↕↕		↔	↕	↕↕		↔↔	↕	↕↕
Traffic Volume (vph)	6	205	999	15	46	862	119	42	69	102	3	194	48	255
Future Volume (vph)	6	205	999	15	46	862	119	42	69	102	3	194	48	255
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		625		0	400		0	325		325		400		275
Storage Lanes		2		0	1		0	1		1		1		1
Taper Length (ft)		25			25			25				25		
Right Turn on Red				Yes			Yes			Yes				Yes
Link Speed (mph)			50			50			50				50	
Link Distance (ft)			894			1488			646				1361	
Travel Time (s)			12.2			20.3			8.8				18.6	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.92	0.94	0.94	0.94
Heavy Vehicles (%)	7%	7%	7%	7%	6%	6%	6%	6%	6%	6%	2%	9%	9%	9%
Shared Lane Traffic (%)														
Turn Type	Prot	Prot	NA		Prot	NA		pm+pt	NA	Perm	pm+pt	pm+pt	NA	Perm
Protected Phases	3	3	8		7	4		1	6		5	5	2	
Permitted Phases								6		6	2	2		2
Detector Phase	3	3	8		7	4		1	6	6	5	5	2	2
Switch Phase														
Minimum Initial (s)	15.0	15.0	7.0		15.0	7.0		7.0	10.0	10.0	7.0	7.0	10.0	10.0
Minimum Split (s)	22.0	22.0	25.0		22.0	14.0		15.0	18.0	18.0	18.0	18.0	18.0	18.0
Total Split (s)	25.0	25.0	60.0		25.0	60.0		15.0	25.0	25.0	20.0	20.0	30.0	30.0
Total Split (%)	19.2%	19.2%	46.2%		19.2%	46.2%		11.5%	19.2%	19.2%	15.4%	15.4%	23.1%	23.1%
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lost Time Adjust (s)		0.0	0.0		0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0
Lead/Lag	Lead	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	Min		None	Min		None	None	None	None	None	None	None

Intersection Summary

Area Type: Other

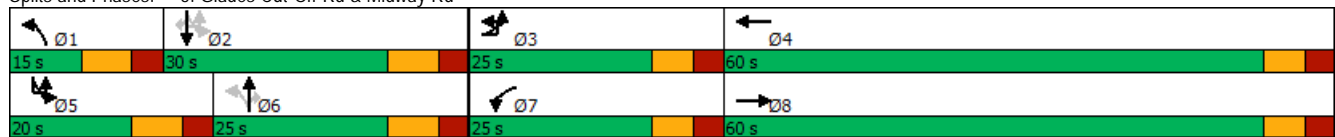
Cycle Length: 130

Actuated Cycle Length: 117.7

Natural Cycle: 85

























Control Type: Actuated-Uncoordinated

Splits and Phases: 5: Glades Cut-Off Rd & Midway Rd



HCM Signalized Intersection Capacity Analysis
5: Glades Cut-Off Rd & Midway Rd

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak

															
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBU	SBL	SBT	SBR	
Lane Configurations															
Traffic Volume (vph)	6	205	999	15	46	862	119	42	69	102	3	194	48	255	
Future Volume (vph)	6	205	999	15	46	862	119	42	69	102	3	194	48	255	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0	
Lane Util. Factor		0.97	0.95		1.00	0.95		1.00	1.00	1.00		1.00	1.00	1.00	
Frt		1.00	1.00		1.00	0.98		1.00	1.00	0.85		1.00	1.00	0.85	
Flt Protected		0.95	1.00		0.95	1.00		0.95	1.00	1.00		0.95	1.00	1.00	
Satd. Flow (prot)		3273	3366		1703	3344		1703	1792	1524		1657	1743	1482	
Flt Permitted		0.95	1.00		0.95	1.00		0.72	1.00	1.00		0.48	1.00	1.00	
Satd. Flow (perm)		3273	3366		1703	3344		1297	1792	1524		845	1743	1482	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.92	0.94	0.94	0.94	
Adj. Flow (vph)	6	218	1063	16	49	917	127	45	73	109	3	206	51	271	
RTOR Reduction (vph)	0	0	1	0	0	8	0	0	0	96	0	0	0	224	
Lane Group Flow (vph)	0	224	1078	0	49	1036	0	45	73	13	0	209	51	47	
Heavy Vehicles (%)	7%	7%	7%	7%	6%	6%	6%	6%	6%	6%	2%	9%	9%	9%	
Turn Type	Prot	Prot	NA		Prot	NA		pm+pt	NA	Perm	pm+pt	pm+pt	NA	Perm	
Protected Phases	3	3	8		7	4		1	6		5	5	2		
Permitted Phases								6		6	2	2		2	
Actuated Green, G (s)		16.2	52.8		11.7	48.3		19.8	14.4	14.4		33.2	21.1	21.1	
Effective Green, g (s)		16.2	52.8		11.7	48.3		19.8	14.4	14.4		33.2	21.1	21.1	
Actuated g/C Ratio		0.13	0.44		0.10	0.40		0.16	0.12	0.12		0.27	0.17	0.17	
Clearance Time (s)		7.0	7.0		7.0	7.0		8.0	8.0	8.0		8.0	8.0	8.0	
Vehicle Extension (s)		5.0	5.0		5.0	5.0		5.0	5.0	5.0		5.0	5.0	5.0	
Lane Grp Cap (vph)		438	1468		164	1334		230	213	181		313	303	258	
v/s Ratio Prot		c0.07	c0.32		0.03	c0.31		0.01	0.04			c0.07	0.03		
v/s Ratio Perm								0.02		0.01		c0.12		0.03	
v/c Ratio		0.51	0.73		0.30	0.78		0.20	0.34	0.07		0.67	0.17	0.18	
Uniform Delay, d1		48.7	28.3		50.8	31.6		43.5	49.0	47.4		36.9	42.5	42.6	
Progression Factor		1.00	1.00		1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Incremental Delay, d2		2.0	2.4		2.1	3.4		0.9	2.0	0.4		7.0	0.6	0.7	
Delay (s)		50.7	30.7		53.0	35.0		44.3	51.0	47.7		43.9	43.0	43.3	
Level of Service		D	C		D	D		D	D	D		D	D	D	
Approach Delay (s)			34.1			35.8			48.1					43.5	
Approach LOS			C			D			D					D	
Intersection Summary															
HCM 2000 Control Delay			37.3											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio			0.75												
Actuated Cycle Length (s)			121.0						30.0						
Intersection Capacity Utilization			86.7%											ICU Level of Service	E
Analysis Period (min)			15												
c Critical Lane Group															

Lanes, Volumes, Timings
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Lane Group	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↕	↗	↖	↕	↕	↖	↕		↖	↕	
Traffic Volume (vph)	1	1	884	465	396	853	1	215	0	320	0	1	6
Future Volume (vph)	1	1	884	465	396	853	1	215	0	320	0	1	6
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)		325		100	300		300	250		0	100		0
Storage Lanes		1		1	1		1	1		0	1		0
Taper Length (ft)		25			25			25			25		
Right Turn on Red				Yes			Yes			Yes			Yes
Link Speed (mph)			45			45			30			30	
Link Distance (ft)			1488			1388			510			298	
Travel Time (s)			22.5			21.0			11.6			6.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%
Shared Lane Traffic (%)													
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA	
Protected Phases	1	1	6		5	2		7	4		3	8	
Permitted Phases	6	6		6	2			4			8		
Detector Phase	1	1	6	6	5	2		7	4		3	8	
Switch Phase													
Minimum Initial (s)	7.0	7.0	7.0	7.0	7.0	7.0		7.0	7.0		7.0	7.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0		15.0	15.0		15.0	15.0	
Total Split (s)	15.0	15.0	70.0	70.0	40.0	95.0		25.0	25.0		15.0	15.0	
Total Split (%)	10.0%	10.0%	46.7%	46.7%	26.7%	63.3%		16.7%	16.7%		10.0%	10.0%	
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0		5.0	5.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0		8.0	8.0	
Lead/Lag	Lead	Lead	Lag	Lag	Lead	Lag		Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes		Yes	Yes	
Recall Mode	None	None	Min	Min	None	Min		None	None		None	None	

Intersection Summary

Area Type: Other

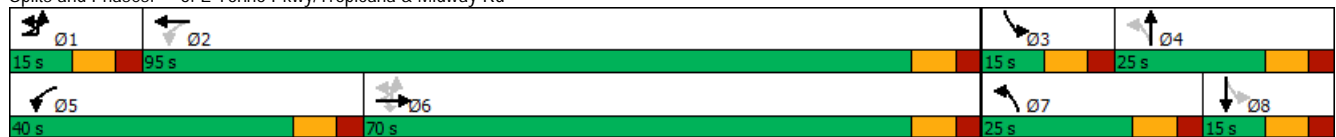
Cycle Length: 150

Actuated Cycle Length: 110.9

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Splits and Phases: 6: E Torino Pkwy/Tropicana & Midway Rd



HCM Signalized Intersection Capacity Analysis
6: E Torino Pkwy/Tropicana & Midway Rd

Glades Cut Off Road Industrial
Buildout Conditions, PM Peak



Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↔	↑↑	↗	↖	↕		↖	↕		↗	↕		
Traffic Volume (vph)	1	1	884	465	396	853	1	215	0	320	0	1	6	
Future Volume (vph)	1	1	884	465	396	853	1	215	0	320	0	1	6	
Ideal Flow (vphpf)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0		
Lane Util. Factor		1.00	0.95	1.00	1.00	0.95		1.00	1.00			1.00		
Frt		1.00	1.00	0.85	1.00	1.00		1.00	0.85			0.87		
Flt Protected		0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00		
Satd. Flow (prot)		1752	3505	1568	1736	3471		1770	1583			1415		
Flt Permitted		0.32	1.00	1.00	0.14	1.00		0.44	1.00			1.00		
Satd. Flow (perm)		590	3505	1568	258	3471		819	1583			1415		
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	
Adj. Flow (vph)	1	1	921	484	412	889	1	224	0	333	0	1	6	
RTOR Reduction (vph)	0	0	0	171	0	0	0	0	264	0	0	6	0	
Lane Group Flow (vph)	0	2	921	313	413	890	0	224	69	0	0	1	0	
Heavy Vehicles (%)	3%	3%	3%	3%	4%	4%	4%	2%	2%	2%	17%	17%	17%	
Turn Type	pm+pt	pm+pt	NA	Perm	pm+pt	NA		pm+pt	NA		pm+pt	NA		
Protected Phases	1	1	6		5	2		7	4		3	8		
Permitted Phases	6	6		6	2			4			8			
Actuated Green, G (s)		46.4	45.3	45.3	82.4	73.3		25.6	25.6			1.1		
Effective Green, g (s)		46.4	45.3	45.3	82.4	73.3		25.6	25.6			1.1		
Actuated g/C Ratio		0.37	0.37	0.37	0.66	0.59		0.21	0.21			0.01		
Clearance Time (s)		8.0	8.0	8.0	8.0	8.0		8.0	8.0			8.0		
Vehicle Extension (s)		3.0	3.0	3.0	3.0	3.0		3.0	3.0			3.0		
Lane Grp Cap (vph)		231	1280	572	518	2051		295	326			12		
v/s Ratio Prot		0.00	0.26		c0.19	0.26		c0.10	0.04			0.00		
v/s Ratio Perm		0.00		0.20	c0.34			c0.06						
v/c Ratio		0.01	0.72	0.55	0.80	0.43		0.76	0.21			0.09		
Uniform Delay, d1		24.3	33.9	31.2	26.7	13.9		44.8	40.8			61.0		
Progression Factor		1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00		
Incremental Delay, d2		0.0	2.0	1.1	8.3	0.1		10.7	0.3			3.2		
Delay (s)		24.3	35.8	32.3	35.0	14.1		55.5	41.1			64.1		
Level of Service		C	D	C	C	B		E	D			E		
Approach Delay (s)			34.6			20.7			46.9			64.1		
Approach LOS			C			C			D			E		
Intersection Summary														
HCM 2000 Control Delay			31.2										HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio			0.86											
Actuated Cycle Length (s)			124.0										Sum of lost time (s)	32.0
Intersection Capacity Utilization			86.2%										ICU Level of Service	E
Analysis Period (min)			15											
c Critical Lane Group														

Intersection														
Int Delay, s/veh	0.3													
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↕↔				↕↕				↕	↕		↕
Traffic Vol, veh/h	1	3	1227	6	6	0	1238	0	0	0	7	2	0	13
Future Vol, veh/h	1	3	1227	6	6	0	1238	0	0	0	7	2	0	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	-	None	-	-	-	None	-	-	None	-	-	None
Storage Length	-	0	-	-	-	-	-	-	-	-	0	0	-	0
Veh in Median Storage, #	-	-	0	-	-	-	0	-	-	0	-	-	0	-
Grade, %	-	-	0	-	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	3	1334	7	7	0	1346	0	0	0	8	2	0	14
Major/Minor	Major1			Major2			Minor1			Minor2				
Conflicting Flow All	1346	1346	0	0	1340	-	-	0	-	-	671	2035	-	673
Stage 1	-	-	-	-	-	-	-	-	-	-	-	1360	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	675	-	-
Critical Hdwy	6.44	4.14	-	-	6.44	-	-	-	-	-	6.94	7.54	-	6.94
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	-	-	6.54	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	-	-	-	6.54	-	-
Follow-up Hdwy	2.52	2.22	-	-	2.52	-	-	-	-	-	3.32	3.52	-	3.32
Pot Cap-1 Maneuver	199	508	-	-	200	0	-	0	0	0	399	33	0	398
Stage 1	-	-	-	-	-	0	-	0	0	0	-	156	0	-
Stage 2	-	-	-	-	-	0	-	0	0	0	-	410	0	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	361	361	-	-	196	-	-	-	-	-	399	29	-	398
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	-	-	-	29	-	-
Stage 1	-	-	-	-	-	-	-	-	-	-	-	154	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	398	-	-
Approach	EB			WB			NB			SB				
HCM Control Delay, s	0			0.1			14.2			31				
HCM LOS							B			D				
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBT	SBLn1	SBLn2							
Capacity (veh/h)	399	361	-	-	-	29	398							
HCM Lane V/C Ratio	0.019	0.012	-	-	-	0.075	0.036							
HCM Control Delay (s)	14.2	15.1	-	-	-	139	14.4							
HCM Lane LOS	B	C	-	-	-	F	B							
HCM 95th %tile Q(veh)	0.1	0	-	-	-	0.2	0.1							

Intersection						
Int Delay, s/veh	0.1					
Movement	SEL	SER	NEL	NET	SWT	SWR
Lane Configurations	W			↑		↓
Traffic Vol, veh/h	2	1	1	211	106	3
Future Vol, veh/h	2	1	1	211	106	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	15	15	2	2	2	2
Mvmt Flow	2	1	1	229	115	3

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	348	117	118	0	-	0
Stage 1	117	-	-	-	-	-
Stage 2	231	-	-	-	-	-
Critical Hdwy	6.55	6.35	4.12	-	-	-
Critical Hdwy Stg 1	5.55	-	-	-	-	-
Critical Hdwy Stg 2	5.55	-	-	-	-	-
Follow-up Hdwy	3.635	3.435	2.218	-	-	-
Pot Cap-1 Maneuver	624	901	1470	-	-	-
Stage 1	877	-	-	-	-	-
Stage 2	778	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	623	901	1470	-	-	-
Mov Cap-2 Maneuver	623	-	-	-	-	-
Stage 1	876	-	-	-	-	-
Stage 2	778	-	-	-	-	-

Approach	SE	NE	SW
HCM Control Delay, s	10.2	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NEL	NET	SELn1	SWT	SWR
Capacity (veh/h)	1470	-	694	-	-
HCM Lane V/C Ratio	0.001	-	0.005	-	-
HCM Control Delay (s)	7.5	0	10.2	-	-
HCM Lane LOS	A	A	B	-	-
HCM 95th %tile Q(veh)	0	-	0	-	-

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		↑	↑↑			↑↑
Traffic Vol, veh/h	0	18	835	0	0	542
Future Vol, veh/h	0	18	835	0	0	542
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	15	15	7	7	7	7
Mvmt Flow	0	20	908	0	0	589

Major/Minor	Minor1	Major1	Major2	Major2	Major2
Conflicting Flow All	-	454	0	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.2	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.45	-	-	-
Pot Cap-1 Maneuver	0	519	-	0	-
Stage 1	0	-	-	0	-
Stage 2	0	-	-	0	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	519	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12.2	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBT
Capacity (veh/h)	-	-	519	-
HCM Lane V/C Ratio	-	-	0.038	-
HCM Control Delay (s)	-	-	12.2	-
HCM Lane LOS	-	-	B	-
HCM 95th %tile Q(veh)	-	-	0.1	-

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	240	0	4	1	0	20	7	575	0	8	435	0
Future Vol, veh/h	240	0	4	1	0	20	7	575	0	8	435	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	1	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	7	7	7	15	15	15	7	7	7	7	7	7
Mvmt Flow	261	0	4	1	0	22	8	625	0	9	473	0
Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	820	1132	237	896	1132	313	473	0	0	625	0	0
Stage 1	491	491	-	641	641	-	-	-	-	-	-	-
Stage 2	329	641	-	255	491	-	-	-	-	-	-	-
Critical Hdwy	7.64	6.64	7.04	7.8	6.8	7.2	4.24	-	-	4.24	-	-
Critical Hdwy Stg 1	6.64	5.64	-	6.8	5.8	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.64	5.64	-	6.8	5.8	-	-	-	-	-	-	-
Follow-up Hdwy	3.57	4.07	3.37	3.65	4.15	3.45	2.27	-	-	2.27	-	-
Pot Cap-1 Maneuver	~ 259	194	749	215	183	646	1051	-	-	919	-	-
Stage 1	515	534	-	400	437	-	-	-	-	-	-	-
Stage 2	644	455	-	691	515	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	~ 246	189	749	210	178	646	1051	-	-	919	-	-
Mov Cap-2 Maneuver	363	304	-	210	178	-	-	-	-	-	-	-
Stage 1	509	527	-	395	432	-	-	-	-	-	-	-
Stage 2	615	450	-	678	508	-	-	-	-	-	-	-
Approach	EB		WB			NB			SB			
HCM Control Delay, s	36.8		11.4			0.1			0.3			
HCM LOS	E		B									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	1051	-	-	366	588	919	-	-				
HCM Lane V/C Ratio	0.007	-	-	0.725	0.039	0.009	-	-				
HCM Control Delay (s)	8.4	0	-	36.8	11.4	9	0.1	-				
HCM Lane LOS	A	A	-	E	B	A	A	-				
HCM 95th %tile Q(veh)	0	-	-	5.5	0.1	0	-	-				
Notes												
-: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon												

Intersection						
Int Delay, s/veh	0					
Movement	NWL	NWR	NET	NER	SWL	SWT
Lane Configurations		↑	↑↑			↑↑
Traffic Vol, veh/h	0	2	580	1	0	440
Future Vol, veh/h	0	2	580	1	0	440
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	15	15	7	7	7	7
Mvmt Flow	0	2	630	1	0	478

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	-	316	0	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	7.2	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.45	-	-	-
Pot Cap-1 Maneuver	0	643	-	0	-
Stage 1	0	-	-	0	-
Stage 2	0	-	-	0	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	-	643	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	NW	NE	SW
HCM Control Delay, s	10.6	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NET	NER	NWLn1	SWT
Capacity (veh/h)	-	-	643	-
HCM Lane V/C Ratio	-	-	0.003	-
HCM Control Delay (s)	-	-	10.6	-
HCM Lane LOS	-	-	B	-
HCM 95th %tile Q(veh)	-	-	0	-

**APPENDIX H:
Excerpts from Project Midway Traffic
Study**

Table 14: Future (2022) Buildout Conditions Turn Lane Analysis

Intersection/Turn Lane	Existing Length (Ft)	Posted Speed	Required Deceleration (Ft) ¹	50th Percentile Queue Length (Ft) ²	95th Percentile Queue Length (Ft) ²	Existing Turn Lane Sufficient (Y/N) ³	Turn Lane Extension (Ft)
I-95 & SB Ramps Westbound Left-turn Lane	470	45	185	525	700	N	240
I-95 & NB Ramps Eastbound Left-turn Lane	410	45	185	25	25	Y	-
Midway Rd & LTC Pkwy Eastbound Right-Turn Lane	550	45	185	175	275	Y	-
Westbound Left-turn Lane	470	45	185	150	225	Y	-
Northbound Right-turn Lane ⁴	200	30	145	225	350	N	170
Midway Rd & Glades Cut-Off Rd Eastbound Left-Turn Lane	650	50	240	75	150	Y	-
Westbound Left-Turn Lane	395	50	240	50	75	Y	-
Northbound Left-Turn Lane	375	50	240	25	50	Y	-
Northbound Right-Turn Lane	350	50	240	75	150	Y	-
Southbound Left-Turn Lane	440	50	240	125	225	Y	-
Southbound Right-Turn Lane ⁴	300	50	240	250	375	N	190
Midway Rd & E Torino Pkwy Eastbound Left-Turn Lane	325	45	185	0	25	Y	-
Eastbound Right-Turn Lane	N/A ⁵	45	185	350	500	-	500
Westbound Left-Turn Lane ⁴	300	45	185	250	400	N	135
Northbound Left-Turn Lane ⁴	250	40	155	175	275	N	80

Notes:

1. Required deceleration length based on the 2020 FDOT Design Manual, Exhibit 212-1.
2. Anticipated queue lengths are based on the 50th and 95th percentile queues during the future (2022) buildout PM peak hour as reported by Synchro analysis.
3. Existing storage lengths were determined to be sufficient if the turn lane could accommodate the higher of (1) the sum of the required deceleration length and 50th percentile queue length and (2) the 95th percentile queue length.
4. Noted turn lanes have insufficient capacity to accommodate forecasted turning movement volumes under future (2022) background conditions.
5. Eastbound right-turn lane is recommended improvement under future (2022) background conditions, before the addition of project traffic.

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & I-95 SB Ramps
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.91
PM PEAK HOUR FACTOR: 0.92

"AM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
AM Raw Turning Movements		0	93	114		363	190	0		0	0	0		213	1	14		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
AM EXISTING CONDITIONS																		
		0	114	140		446	234	0		0	0	0		262	1	17		
			7%				11%				0%				16%			
"PM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
PM Raw Turning Movements		0	112	132		397	207	0		0	0	0		254	0	30		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
PM EXISTING CONDITIONS																		
		0	138	162		488	255	0		0	0	0		312	0	37		
			2%				3%				0%				9%			
"AM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway			2			59	2							60				
TOTAL "VESTED" TRAFFIC		0	2	0		59	2	0		0	0	0		60	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
AM BACKGROUND TRAFFIC GROWTH		0	12	15		48	25	0		0	0	0		28	0	2		
AM NON-PROJECT TRAFFIC																		
		0	128	155		553	261	0		0	0	0		350	1	19		
"PM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway			1			47	1							47				
TOTAL "VESTED" TRAFFIC		0	1	0		47	1	0		0	0	0		47	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
PM BACKGROUND TRAFFIC GROWTH		0	15	18		53	28	0		0	0	0		34	0	4		
PM NON-PROJECT TRAFFIC																		
		0	154	180		588	284	0		0	0	0		393	0	41		
"AM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering				1.0%											27.0%		
	Exiting						27.0%	1.0%										
"PM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering				1.0%											27.0%		
	Exiting						27.0%	1.0%										
"AM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New				5			1	0						122			
AM TOTAL PROJECT TRAFFIC				0	5	0		1	0	0		0	0	0	122	0	0	
AM TOTAL TRAFFIC																		
		0	133	155		554	261	0		0	0	0		472	1	19		
"PM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New				4			130	5						106			
PM TOTAL PROJECT TRAFFIC				0	4	0		130	5	0		0	0	0	106	0	0	
PM TOTAL TRAFFIC																		
		0	158	180		718	289	0		0	0	0		499	0	41		

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & I-95 NB Ramps
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.89
PM PEAK HOUR FACTOR: 0.95

"AM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
AM Raw Turning Movements		7	298	0		0	450	327		104	2	388		0	0	0		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
AM EXISTING CONDITIONS																		
		9	367	0		0	554	402		128	2	477		0	0	0		
		HV %		13%			13%				7%			0%				
"PM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
PM Raw Turning Movements		9	331	0		0	508	255		121	0	384		0	0	0		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
PM EXISTING CONDITIONS																		
		11	407	0		0	625	314		149	0	472		0	0	0		
		HV %		6%			7%				7%			0%				
"AM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway			61				61	59				60						
TOTAL "VESTED" TRAFFIC		0	61	0		0	61	59		0	0	60		0	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
AM BACKGROUND TRAFFIC GROWTH		1	40	0		0	60	44		14	0	52		0	0	0		
AM NON-PROJECT TRAFFIC																		
		10	468	0		0	675	505		142	2	589		0	0	0		
"PM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway			49				49	47				47						
TOTAL "VESTED" TRAFFIC		0	49	0		0	49	47		0	0	47		0	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
PM BACKGROUND TRAFFIC GROWTH		1	44	0		0	68	34		16	0	51		0	0	0		
PM NON-PROJECT TRAFFIC																		
		12	500	0		0	742	395		165	0	570		0	0	0		
"AM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering				28.0%									27.0%				
	Exiting								28.0%	27.0%								
"PM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering				28.0%									27.0%				
	Exiting								28.0%	27.0%								
"AM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New				127				1	1				122				
AM TOTAL PROJECT TRAFFIC				0	127	0		0	1	1		0	0	122		0	0	0
AM TOTAL TRAFFIC																		
		10	595	0		0	676	506		142	2	711		0	0	0		
"PM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New				109				134	130				106				
PM TOTAL PROJECT TRAFFIC				0	109	0		0	134	130		0	0	106		0	0	0
PM TOTAL TRAFFIC																		
		12	609	0		0	876	525		165	0	676		0	0	0		

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & LTC Parkway
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.90
PM PEAK HOUR FACTOR: 0.96

"AM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
AM Raw Turning Movements		0	585	103		24	707	0		62	0	7		0	0	0		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
AM EXISTING CONDITIONS		0	720	127		30	870	0		76	0	9		0	0	0		
HV %			10%				13%				22%					0%		
"PM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
PM Raw Turning Movements		0	679	40		7	693	0		71	0	14		0	0	0		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
PM EXISTING CONDITIONS		0	835	49		9	852	0		87	0	17		0	0	0		
HV %			8%				7%				7%					0%		
"AM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway						121	-45			164		120						
TOTAL "VESTED" TRAFFIC		0	0	0		121	-45	0		164	0	120		0	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
AM BACKGROUND TRAFFIC GROWTH		0	78	14		3	95	0		8	0	1		0	0	0		
AM NON-PROJECT TRAFFIC		0	798	141		154	920	0		248	0	130		0	0	0		
"PM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway						102	-45			140		100						
TOTAL "VESTED" TRAFFIC		0	0	0		102	-45	0		140	0	100		0	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
PM BACKGROUND TRAFFIC GROWTH		0	91	5		1	93	0		9	0	2		0	0	0		
PM NON-PROJECT TRAFFIC		0	926	54		112	900	0		236	0	119		0	0	0		
"AM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering					55.0%		40.0%										
	Exiting											55.0%		40.0%				
"PM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering					55.0%		40.0%										
	Exiting											55.0%		40.0%				
"AM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New					249		181				3		2				
AM TOTAL PROJECT TRAFFIC		0	0	249		181	0	0		3	0	2		0	0	0	0	
AM TOTAL TRAFFIC		0	798	390		335	920	0		251	0	132		0	0	0		
"PM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New					215		156				264		192				
PM TOTAL PROJECT TRAFFIC		0	0	215		156	0	0		264	0	192		0	0	0	0	
PM TOTAL TRAFFIC		0	926	269		268	900	0		500	0	311		0	0	0		

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & Glades Cut-Off Road
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.93
PM PEAK HOUR FACTOR: 0.94

"AM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
AM Raw Turning Movements		119	435	17		87	612	81		32	26	41		83	54	102		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
AM EXISTING CONDITIONS																		
		146	535	21		107	753	100		39	32	50		102	66	125		
HV %			11%				9%				13%					29%		
"PM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
PM Raw Turning Movements		124	564	11		32	494	86		26	50	72		143	34	161		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
PM EXISTING CONDITIONS																		
		153	694	14		39	608	106		32	62	89		176	42	198		
HV %			7%				6%				6%					9%		
"AM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway		12	65				65									11		
TOTAL "VESTED" TRAFFIC		12	65	0		0	65	0		0	0	0		0	0	11		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
AM BACKGROUND TRAFFIC GROWTH		16	58	2		12	82	11		4	3	5		11	7	14		
AM NON-PROJECT TRAFFIC																		
		174	658	23		119	900	111		43	35	55		113	73	150		
"PM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway		9	48				48									9		
TOTAL "VESTED" TRAFFIC		9	48	0		0	48	0		0	0	0		0	0	9		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
PM BACKGROUND TRAFFIC GROWTH		17	75	2		4	66	12		3	7	10		19	5	22		
PM NON-PROJECT TRAFFIC																		
		179	817	16		43	722	118		35	69	99		195	47	229		
"AM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering								34.0%									6.0%
	Exiting			6.0%	34.0%													
"PM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering								34.0%									6.0%
	Exiting			6.0%	34.0%													
"AM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New			0	2				154									27
AM TOTAL PROJECT TRAFFIC			0	2	0		0	154	0		0	0	0		0	0	27	
AM TOTAL TRAFFIC																		
		174	660	23		119	1,054	111		43	35	55		113	73	177		
"PM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New			29	163				133								23	
PM TOTAL PROJECT TRAFFIC			29	163	0		0	133	0		0	0	0		0	0	23	
PM TOTAL TRAFFIC																		
		208	980	16		43	855	118		35	69	99		195	47	252		

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & E Torino Parkway
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.93
PM PEAK HOUR FACTOR: 0.96

"AM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
AM Raw Turning Movements		5	439	122		139	507	5		271	1	252		4	2	5
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23

AM EXISTING CONDITIONS		6	540	150		171	624	6		333	1	310		5	2	6
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"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
PM Raw Turning Movements		2	498	318		288	501	1		140	0	233		0	1	5
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23

PM EXISTING CONDITIONS		2	613	391		354	616	1		172	0	287		0	1	6
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"AM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway			58	8			57			8						
TOTAL "VESTED" TRAFFIC		0	58	8		0	57	0		8	0	0		0	0	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
AM BACKGROUND TRAFFIC GROWTH		1	59	16		19	68	1		36	0	34		1	0	1

AM NON-PROJECT TRAFFIC		7	657	174		190	749	7		377	1	344		6	2	7
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"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway			43	6			43			6						
TOTAL "VESTED" TRAFFIC		0	43	6		0	43	0		6	0	0		0	0	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH		0	67	43		38	67	0		19	0	31		0	0	1

PM NON-PROJECT TRAFFIC		2	723	440		392	726	1		197	0	318		0	1	7
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"AM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering							30.0%			4.0%						
	Exiting			30.0%	4.0%												

"PM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering							30.0%			4.0%						
	Exiting			30.0%	4.0%												

"AM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New			2	0			136			18						
AM TOTAL PROJECT TRAFFIC			0	2	0		0	136	0		18	0	0		0	0	0

AM TOTAL TRAFFIC		7	659	174		190	885	7		395	1	344		6	2	7
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"PM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New			144	19			117			16						
PM TOTAL PROJECT TRAFFIC			0	144	19		0	117	0		16	0	0		0	0	0

PM TOTAL TRAFFIC		2	867	459		392	843	1		213	0	318		0	1	7
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: Midway Road & Project Driveway 1
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.89
PM PEAK HOUR FACTOR: 0.95

"AM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
AM Raw Turning Movements		0	687	0		0	773	0		0	0	0		0	0	0		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
AM EXISTING CONDITIONS																		
		0	845	0		0	951	0		0	0	0		0	0	0		
"PM EXISTING TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
PM Raw Turning Movements		0	717	0		0	764	0		0	0	0		0	0	0		
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23		
PM EXISTING CONDITIONS																		
		0	882	0		0	939	0		0	0	0		0	0	0		
"AM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway			-45	165			119											
TOTAL "VESTED" TRAFFIC		0	-45	165		0	119	0		0	0	0		0	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
AM BACKGROUND TRAFFIC GROWTH		0	92	0		0	103	0		0	0	0		0	0	0		
AM NON-PROJECT TRAFFIC																		
		0	892	165		0	1,173	0		0	0	0		0	0	0		
"PM BACKGROUND TRAFFIC"																		
	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR		
Proposed Speedway			-45	140			95											
TOTAL "VESTED" TRAFFIC		0	-45	140		0	95	0		0	0	0		0	0	0		
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3		
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%		
PM BACKGROUND TRAFFIC GROWTH		0	96	0		0	102	0		0	0	0		0	0	0		
PM NON-PROJECT TRAFFIC																		
		0	933	140		0	1,136	0		0	0	0		0	0	0		
"AM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering				55.0%	0.0%												
	Exiting								55.0%									
"PM PROJECT DISTRIBUTION"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Net New Distribution	Entering				55.0%	0.0%												
	Exiting								55.0%									
"AM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New				249	2			3									
AM TOTAL PROJECT TRAFFIC				0	249	2		0	3	0		0	0	0		0	0	
AM TOTAL TRAFFIC																		
		0	1,141	167		0	1,176	0		0	0	0		0	0	0		
"PM PROJECT TRAFFIC"																		
LAND USE	TYPE		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Project	Net New				215	3			264									
PM TOTAL PROJECT TRAFFIC				0	215	3		0	264	0		0	0	0		0	0	
PM TOTAL TRAFFIC																		
		0	1,148	143		0	1,400	0		0	0	0		0	0	0		

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: LTC Parkway & Project Driveway 2
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.90
PM PEAK HOUR FACTOR: 0.96

"AM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
AM Raw Turning Movements		0	0	0		0	0	0		0	69	0		0	127	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23

AM EXISTING CONDITIONS		0	0	0		0	0	0		0	85	0		0	156	0
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"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
PM Raw Turning Movements		0	0	0		0	0	0		0	85	0		0	47	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23

PM EXISTING CONDITIONS		0	0	0		0	0	0		0	105	0		0	58	0
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"AM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											10				10	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	10	0		0	10	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
AM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	9	0		0	17	0

AM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	104	0		0	183	0
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"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											7				7	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	7	0		0	7	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	11	0		0	6	0

PM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	123	0		0	71	0
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"AM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										0.0%					95.0%	0.0%
	Exiting											95.0%					

"PM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										0.0%					95.0%	0.0%
	Exiting											95.0%					

"AM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		3								0	5				429	2
AM TOTAL PROJECT TRAFFIC			3	0	0		0	0	0		0	5	0		0	429	2

AM TOTAL TRAFFIC		3	0	0		0	0	0		0	109	0		0	612	2
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"PM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		4								0	456				371	2
PM TOTAL PROJECT TRAFFIC			4	0	0		0	0	0		0	456	0		0	371	2

PM TOTAL TRAFFIC		4	0	0		0	0	0		0	579	0		0	442	2
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: LTC Parkway & Project Driveway 3
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.9
PM PEAK HOUR FACTOR: 0.96

"AM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
AM Raw Turning Movements		0	0	0		0	0	0		0	69	0		0	127	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
AM EXISTING CONDITIONS		0	0	0		0	0	0		0	85	0		0	156	0

"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
PM Raw Turning Movements		0	0	0		0	0	0		0	85	0		0	47	0
Peak Season Correction Factor	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23	1.23
PM EXISTING CONDITIONS		0	0	0		0	0	0		0	105	0		0	58	0

"AM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											10				10	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	10	0		0	10	0
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
AM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	9	0		0	17	0
AM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	104	0		0	183	0

"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											7				7	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	7	0		0	7	0
Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	11	0		0	6	0
PM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	123	0		0	71	0

"AM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										2.0%					35.0%	60.0%
	Exiting		45.0%		2.0%							50.0%					

"PM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										2.0%					35.0%	60.0%
	Exiting		45.0%		2.0%							50.0%					

"AM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		2		0						9	3				158	271
AM TOTAL PROJECT TRAFFIC			2	0	0		0	0	0		9	3	0		0	158	271
AM TOTAL TRAFFIC			2	0	0		0	0	0		9	107	0		0	341	271

"PM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		216		10						8	240				137	235
PM TOTAL PROJECT TRAFFIC			216	0	10		0	0	0		8	240	0		0	137	235
PM TOTAL TRAFFIC			216	0	10		0	0	0		8	363	0		0	208	235

TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: LTC Parkway & Project Driveway 4
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.9
PM PEAK HOUR FACTOR: 0.96

"AM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
AM Raw Turning Movements		0	0	0		0	0	0		0	69	0		0	127	0
Peak Season Correction Factor	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230

AM EXISTING CONDITIONS		0	0	0		0	0	0		0	85	0		0	156	0
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"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
PM Raw Turning Movements		0	0	0		0	0	0		0	85	0		0	47	0
Peak Season Correction Factor	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230

PM EXISTING CONDITIONS		0	0	0		0	0	0		0	105	0		0	58	0
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"AM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											10				10	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	10	0		0	10	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
AM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	9	0		0	17	0

AM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	104	0		0	183	0
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"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											7				7	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	7	0		0	7	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	11	0		0	6	0

PM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	123	0		0	71	0
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"AM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										2.0%	2.0%				10.0%	25.0%
	Exiting		35.0%		2.0%							15.0%				2.0%	

"PM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										2.0%	2.0%				10.0%	25.0%
	Exiting		35.0%		2.0%							15.0%				2.0%	

"AM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		2		0						9	10				45	113
AM TOTAL PROJECT TRAFFIC			2	0	0		0	0	0		9	10	0		0	45	113

AM TOTAL TRAFFIC		2	0	0		0	0	0		9	114	0		0	228	113
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"PM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		168		10						8	80				49	98
PM TOTAL PROJECT TRAFFIC			168	0	10		0	0	0		8	80	0		0	49	98

PM TOTAL TRAFFIC		168	0	10		0	0	0		8	203	0		0	120	98
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TRAFFIC VOLUMES AT STUDY INTERSECTIONS

INTERSECTION: LTC Parkway & Project Driveway 5
COUNT DATE: June 25, 2020
AM PEAK HOUR FACTOR: 0.9
PM PEAK HOUR FACTOR: 0.96

"AM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
AM Raw Turning Movements		0	0	0		0	0	0		0	69	0		0	127	0
Peak Season Correction Factor	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230

AM EXISTING CONDITIONS		0	0	0		0	0	0		0	85	0		0	156	0
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"PM EXISTING TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
PM Raw Turning Movements		0	0	0		0	0	0		0	85	0		0	47	0
Peak Season Correction Factor	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230	1.230

PM EXISTING CONDITIONS		0	0	0		0	0	0		0	105	0		0	58	0
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"AM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											10				10	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	10	0		0	10	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
AM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	9	0		0	17	0

AM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	104	0		0	183	0
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"PM BACKGROUND TRAFFIC"	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
Proposed Speedway											7				7	
TOTAL "VESTED" TRAFFIC		0	0	0		0	0	0		0	7	0		0	7	0

Years To Buildout	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Yearly Growth Rate	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%	3.5%
PM BACKGROUND TRAFFIC GROWTH		0	0	0		0	0	0		0	11	0		0	6	0

PM NON-PROJECT TRAFFIC		0	0	0		0	0	0		0	123	0		0	71	0
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"AM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										1.0%	4.0%					10.0%
	Exiting		15.0%		1.0%											4.0%	

"PM PROJECT DISTRIBUTION"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Net New Distribution	Entering										1.0%	4.0%					10.0%
	Exiting		15.0%		1.0%											4.0%	

"AM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		1		0						5	18				0	45
AM TOTAL PROJECT TRAFFIC			1	0	0		0	0	0		5	18	0		0	0	45

AM TOTAL TRAFFIC		1	0	0		0	0	0		5	122	0		0	183	45
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"PM PROJECT TRAFFIC"		EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR	SBU	SBL	SBT	SBR
LAND USE	TYPE																
Project	Net New		72		5						4	16			19	39	
PM TOTAL PROJECT TRAFFIC			72	0	5		0	0	0		4	16	0		19	39	

PM TOTAL TRAFFIC		72	0	5		0	0	0		4	139	0		0	90	39
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**APPENDIX I:
Committed Improvements Information**



E-Updates | FL511 | Mobile | Site Map

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Web Application

Office of Work Program and Budget Lisa Saliba - Director

Five Year Work Program

Selection Criteria	
District 04 (Updated: 2/17/2016-21.15.02) Item Number:436646-1	2016-2021 G1 St Lucie County Description Contains:MIDWAY

Transportation System Description	District	Length	Type of Work	Item		
				2019	2020	2021
INTRASTATE INTERSTATE	District 04 - St Lucie County	7.894	BRIDGE REHABILITATION	436646-1		
SR-9/I-95 OVER GATLIN BLVD. & SR-9/I-95 OVER CR-712 MIDWAY RD.						SIS
Highways /Preliminary Engineering (<i>On-Going</i>)		\$140,799				
Highways /Construction		\$12,180,640				
Highways /Contract Incentives			\$300,000			
Highways /Environmental (<i>On-Going</i>)						

This site is maintained by the Office of Work Program and Budget, located at 605 Suwannee Street, MS 21, Tallahassee, Florida 32399. For additional information please e-mail questions or comments to:
(Lisa Saliba: Lisa.Saliba@dot.state.fl.us or call 850-414-4622)
[View Contact Information for Office of Work Program and Budget](#)

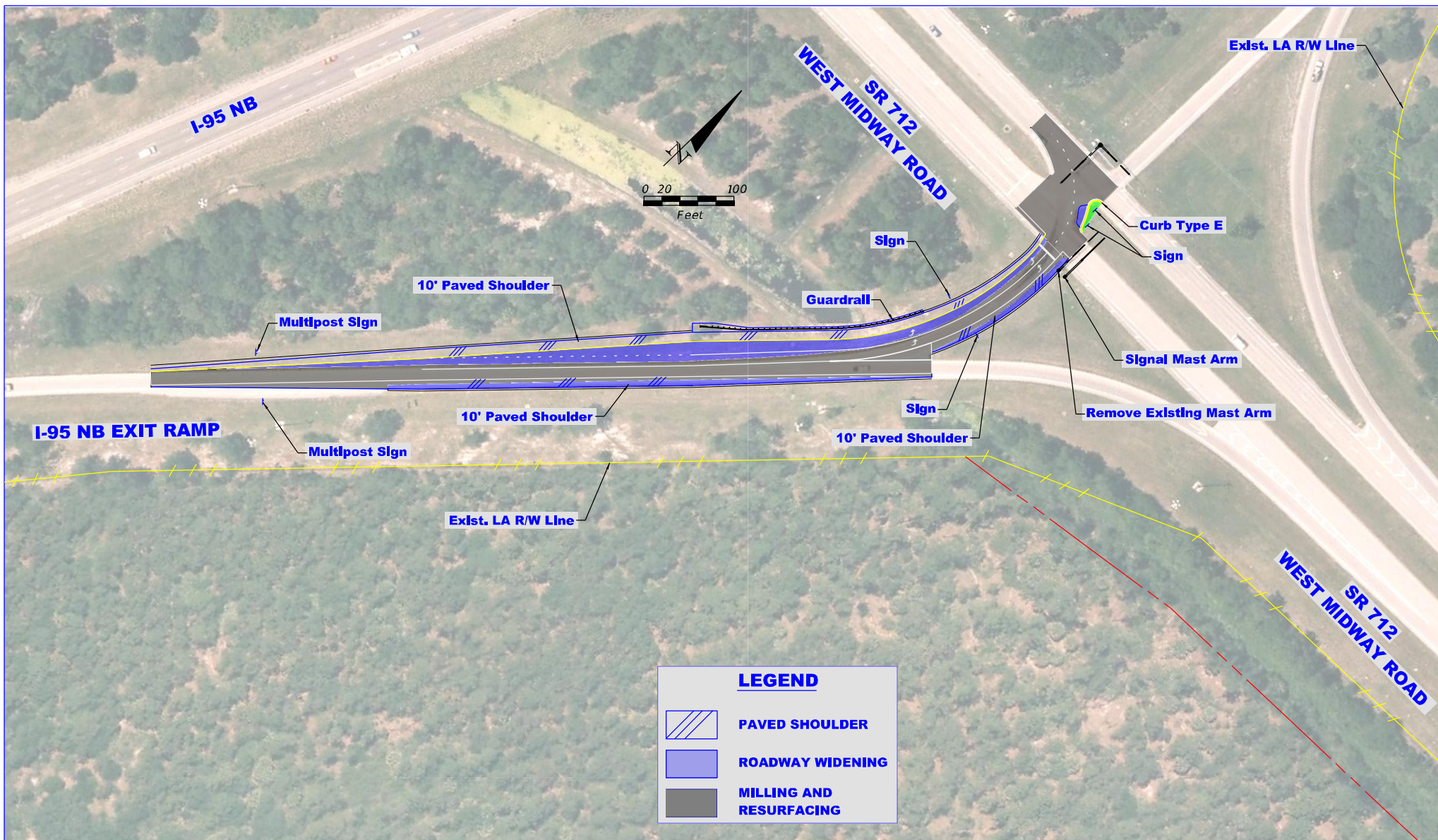
Application Home: [Work Program](#)
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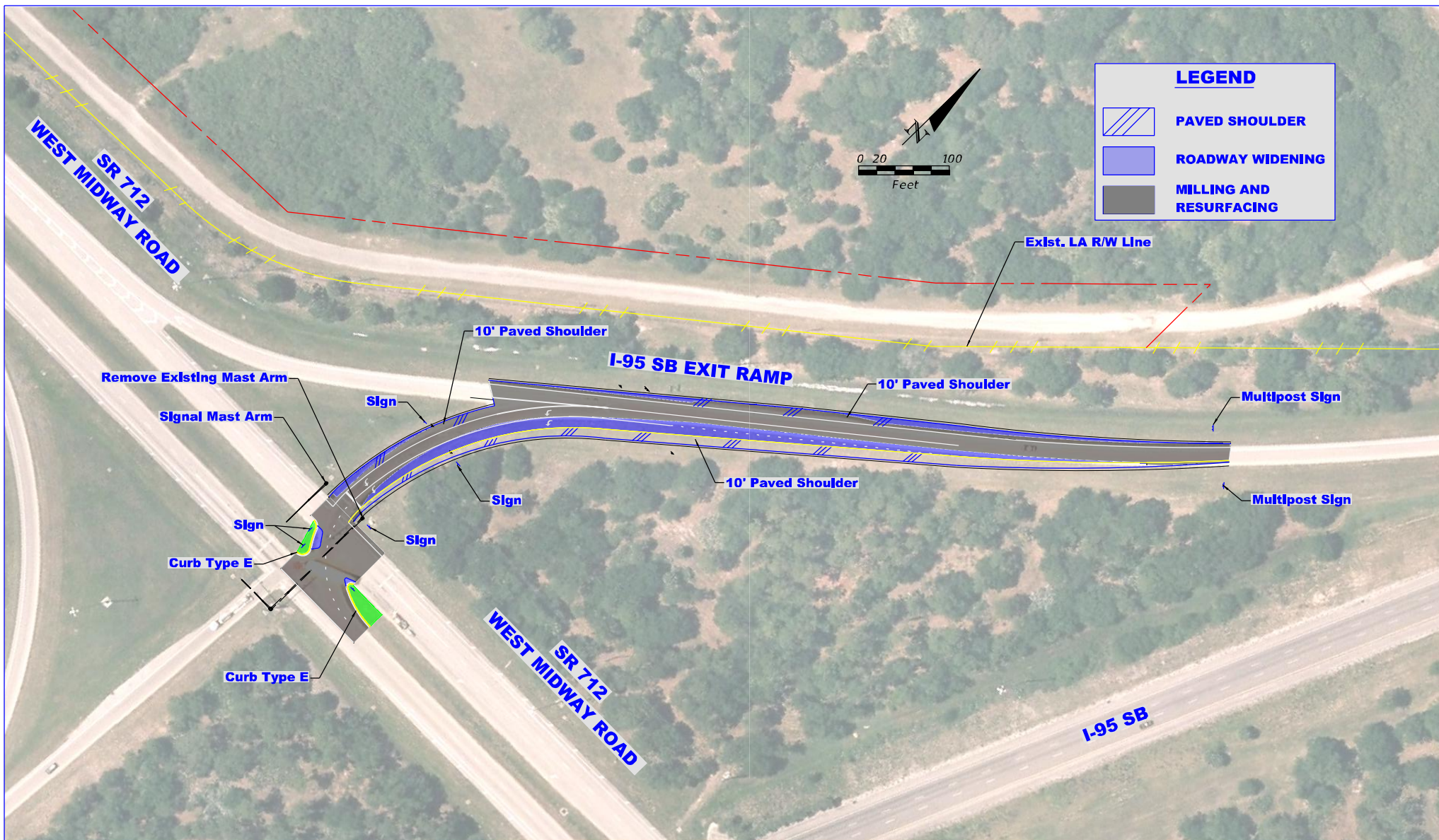


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Florida Department of Transportation
Consistent, Predictable, Repeatable



FDOT I-95 NB RAMP AT SR 712 WEST MIDWAY ROAD DRAFT CONCEPT PLAN



I-95 SB RAMP AT SR 712 WEST MIDWAY ROAD DRAFT CONCEPT PLAN

**APPENDIX J:
NCHRP Report 457 Worksheets**

Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

INPUT

Roadway geometry:	2-lane roadway
Variable	Value
Major-road speed, mph:	50
Major-road volume (one direction), veh/h:	108
Right-turn volume, veh/h:	3

OUTPUT

Variable	Value
Limiting right-turn volume, veh/h:	156
Guidance for determining the need for a major-road right-turn bay for a 2-lane roadway:	
Do NOT add right-turn bay.	

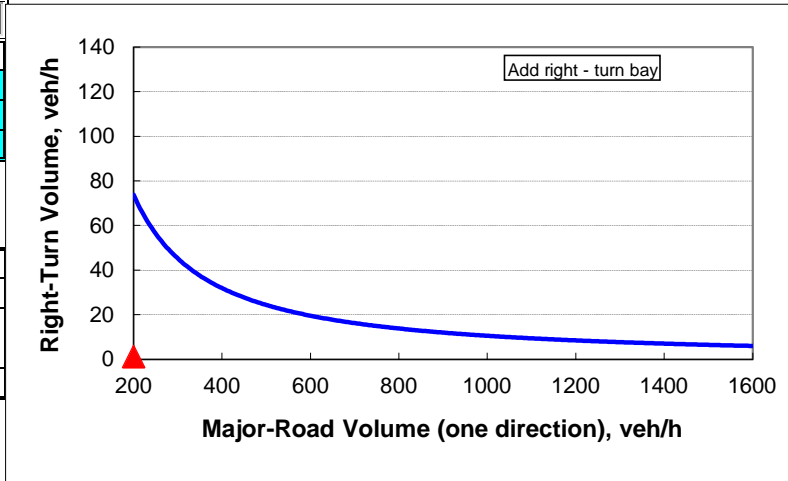


Figure 2 - 5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.

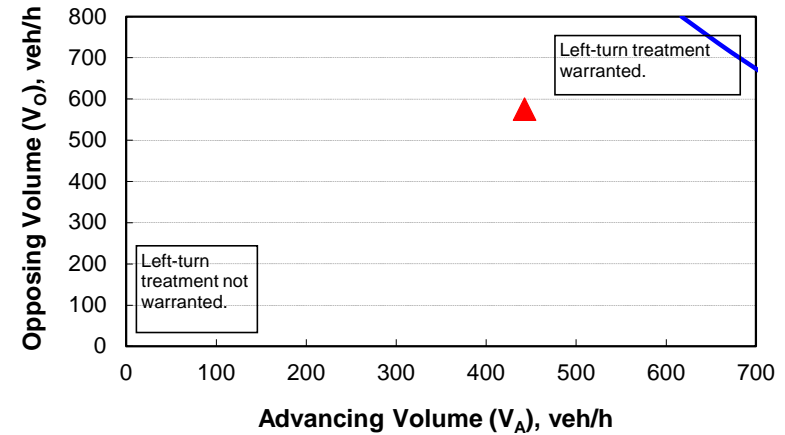
2-lane roadway (English)

INPUT

Variable	Value
85 th percentile speed, mph:	30
Percent of left-turns in advancing volume (V_A), %:	2%
Advancing volume (V_A), veh/h:	443
Opposing volume (V_O), veh/h:	575

OUTPUT

Variable	Value
Limiting advancing volume (V_A), veh/h:	773
Guidance for determining the need for a major-road left-turn bay:	
Left-turn treatment NOT warranted.	



CALIBRATION CONSTANTS

Variable	Value
Average time for making left-turn, s:	3.0
Critical headway, s:	5.0
Average time for left-turn vehicle to clear the advancing lane, s:	1.9

Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

INPUT

Roadway geometry:	4-lane roadway
Variable	Value
Major-road speed, mph:	35
Major-road volume (one direction), veh/h:	582
Right-turn volume, veh/h:	0

OUTPUT

Variable	Value
Limiting right-turn volume, veh/h:	295
Guidance for determining the need for a major-road right-turn bay for a 4-lane roadway:	
Do NOT add right-turn bay.	

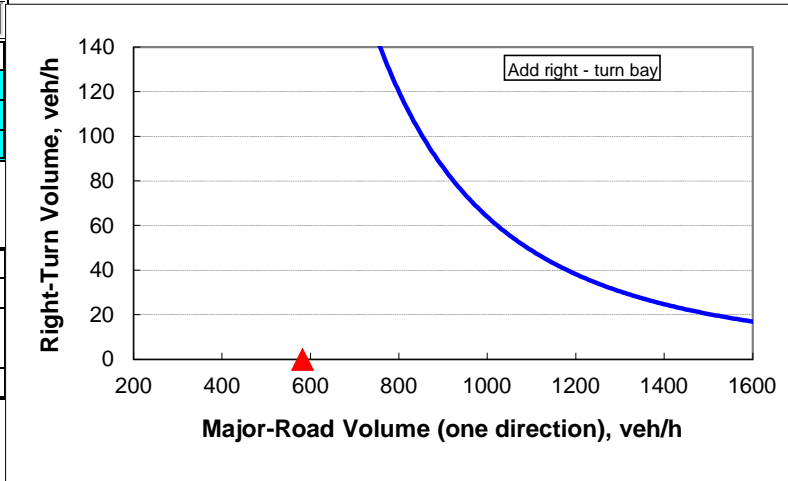


Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

INPUT

Roadway geometry:	4-lane roadway
Variable	Value
Major-road speed, mph:	45
Major-road volume (one direction), veh/h:	1238
Right-turn volume, veh/h:	6

OUTPUT

Variable	Value
Limiting right-turn volume, veh/h:	21
Guidance for determining the need for a major-road right-turn bay for a 4-lane roadway:	
Do NOT add right-turn bay.	

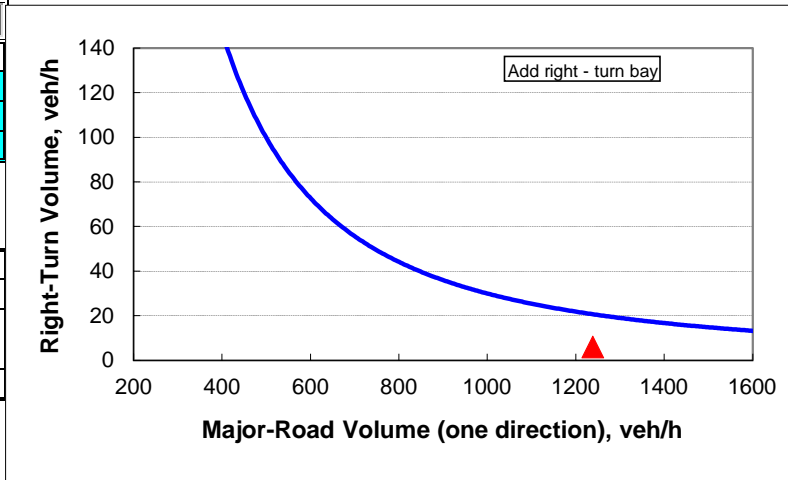


Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

INPUT

Roadway geometry:	4-lane roadway
Variable	Value
Major-road speed, mph:	35
Major-road volume (one direction), veh/h:	581
Right-turn volume, veh/h:	1

OUTPUT

Variable	Value
Limiting right-turn volume, veh/h:	297
Guidance for determining the need for a major-road right-turn bay for a 4-lane roadway:	
Do NOT add right-turn bay.	

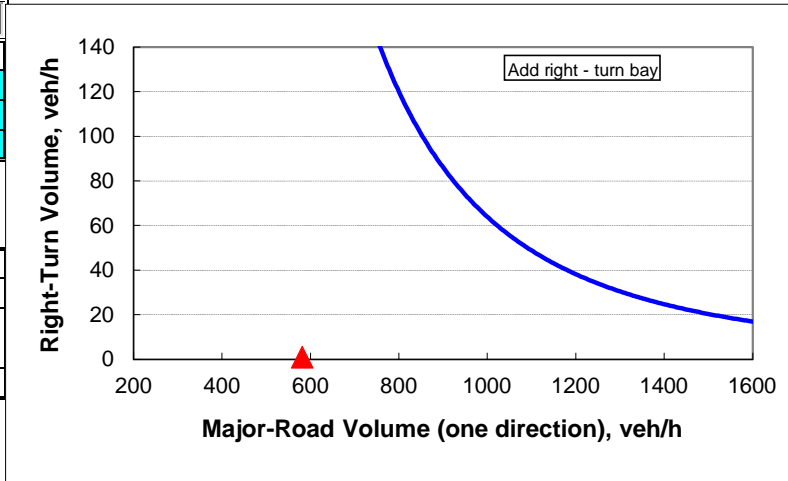


Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

INPUT

Roadway geometry:	4-lane roadway
Variable	Value
Major-road speed, mph:	35
Major-road volume (one direction), veh/h:	835
Right-turn volume, veh/h:	0

OUTPUT

Variable	Value
Limiting right-turn volume, veh/h:	106
Guidance for determining the need for a major-road right-turn bay for a 4-lane roadway:	
Do NOT add right-turn bay.	

